

Richmond Wastewater Treatment Facility 20 Year Evaluation *90% Submittal*

January 29, 2024

Prepared for: Town of Richmond, Vermont







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- Appendix C Town of Richmond Fiscal Year 2024 Water and Sewer Budget
- Appendix D Biological Treatment Process Model
- Appendix E Collection System Evaluation Manhole Inspection Reports
- Appendix F Engineer's Opinion of Probable Cost
- Appendix G Life Cycle Costs
- Appendix H Proposed Site Plan
- Appendix I Proposed Hydraulic Profile

Executive Summary

For more than 50-years, the Richmond Wastewater Treatment Facility has served the Town of Richmond (Town), protecting public health and maintaining water quality in the Winooski River through solids removal, biological/chemical treatment, and disinfection of municipal wastewater. The Richmond WWTF also serves the wider Chittenden County community as a septage acceptance facility, historically accepting up to 4.8 million gallons per year.

The Richmond WWTF was substantially upgraded in 2005 to provide advanced treatment and septage receiving facilities, and much of the existing equipment has reached the end of its useful life. Hoyle, Tanner and Associates (Hoyle Tanner) has prepared a 20-Year Engineering Evaluation of the existing wastewater treatment facility and collection system to document existing conditions and make recommendations for refurbishment. The purpose of this 20-Year Evaluation serves as the facility's preliminary engineering report (PER) to further develop alternatives for refurbishment of major unit processes and document recommended upgrades to ensure treatment capacity is maintained through the 20-year planning horizon.

This report also includes a comprehensive analysis of historical and projected flows and loads to determine the basis of design for each unit process. The existing facility has an average daily flow (ADF) design capacity of 0.222 million gallons per day (MGD), and a peak hydraulic design capacity of 1.152 MGD. The analysis concluded that while the Richmond WWTF is currently operating at 0.073 MGD (approximately 33% of its permitted design flow), it has experienced peak instantaneous flows during flooding events that exceed the design peak hydraulic capacity of 1.152 MGD, and it is recommended that critical infrastructure be designed for a peak hydraulic flow of 2.3 MGD.

To address age-related rehabilitation and hydraulic capacity requirements, the Town selected to advance the following replacement and refurbishment projects at the Richmond WWTF and Bridge Street Pump Station:

- Age-related replacements of influent pumps
- New Screening Building with ¼" multi-rake screen & wash press
- Modifications to existing Headworks Building with new grit removal unit
- New compressed gas mixing in anoxic selectors & sludge holding tanks
- Biological treatment improvements including new aeration tank diffusers & blowers
- New coagulant chemical feed pumps for redundancy and additional secondary containment
- Replacement of existing waste activated sludge (WAS) pump and return activated sludge (RAS) electrically actuated pinch valve & flow meter
- Structural assessment of filter units and replacement of cloth media, pumps & other parts
- New ultraviolet (UV) disinfection system to accommodate peak hydraulic flow
- Reconfigured 90° v-notch weir and new radar level sensor for effluent flow measurement
- New effluent pump station with wet well, effluent pumps, level control, and a backflow preventer to prevent flooding in facility during flooding events

- Age-related replacement of septage receiving unit
- Structural modifications to septage receiving area to address classification code concerns
- Expansion and modifications to Dewatering Building with new 4-channel rotary press, sludge transfer & feed pumps, conveyor system, polymer feed system, and chemical feed system to accommodate increased septage acceptance
- Modifications to Operations Building including new boiler, HVAC, and hatch in the floor for influent pump access
- Site improvements including exterior flood protection, new generator, new plant water system, & SCADA programming
- Cleaning of 15 manholes and maintenance of 6 manholes identified
- Age-related replacement of Bridge Street Pump Station with submersible-type pump station & new force main under bridge

The engineer's opinion of probable total project costs for the recommended Richmond WWTF upgrade project are estimated to be \$17,938,000 for an April 2027 bid date.

1. PROJECT PLANNING

1.1 Background

The Richmond Wastewater Treatment Facility (WWTF) operates under NPDES Permit No. 3-1173 effective December 21, 2020. The Richmond Wastewater Treatment Facility is a 0.222 MGD activated sludge treatment facility that incorporates the use of an anoxic selector. In general, the treatment processes at the facility involve the use of screening, grit removal, anoxic selectors, aeration basins, secondary clarification, filtration, and disinfection. The solids train includes septage receiving, aerated sludge holding, and dewatering. A process flow diagram is located below in Figure 1-1.



Figure 1-1. Process Flow Diagram

The following is a history of the facility:

- 1972: Original Facility Constructed
- 2005: Major Facility Upgrade (New Headworks Building w/ Grit Removal, Anoxic Selectors, Aeration Tank Diffusers, New Process Building w/ Filtration & UV Disinfection, New Aerated Septage Holding Tank)
- 2023: One influent pump replaced with a used emergency pump

This 20-Year Evaluation will assess the existing facility and collection system to identify needs, develop alternatives to address the needs, and select a recommended alternative.

1.2 Scope of Services

Hoyle, Tanner's scope of services for this study is summarized in the following:

The 20-Year Evaluation will be prepared to incorporate the following information. The report will follow the State Water Investment Division (WID) format.

• Project Planning (Section 1)

Richmond Wastewater Treatment Facility 20-Year Evaluation Section 1 – Project Planning

- Existing Wastewater Facilities (Section 2)
- Existing Collection System (Section 3)
- Need for Project (Section 4)
- Alternatives Considered (Section 5)
- Selection of Alternative (Section 6)
- Proposed Project (Recommended Alternative) (Section 7)
 - Proposed Hydraulic Profile
 - Proposed Process Flow Diagram
 - Proposed Site Plan
 - o Equipment selection details including design criteria
 - Opinion of Probable Construction Cost
 - Opinion of Probable Total Project Cost
 - Project phasing defined in a Sequence of Work
 - List of permits/approvals needed for State agencies
 - Proposed project schedule
 - Proposed next steps

1.3 Location

The Richmond WWTF is located on 281 Esplanade Street in Richmond, Vermont. The Richmond WWTF receives wastewater from residential, commercial, and industrial sources and discharges to the adjacent Winooski River through an outfall. An overall location map is provided in Figure A-1 in Appendix A.

1.4 Environmental Resources

The proposed project will not increase the hydraulic capacity of the Main WWTF as the improvements will be addressing age-related needs. All proposed work will occur within the Town property at the Main WWTF in previously disturbed areas.

1.4.1. Winooski River

The Richmond WWTF discharges through Outfall S/N 001 to a waste management zone in the Winooski River, a Class B (2) water, and a designated Cold Water Fish Habitat.

Class B waters are suitable for swimming and other primary contact recreation; irrigation and agricultural uses; aquatic biota and aquatic habitat; good aesthetic value; boating, fishing, and other recreational uses; and suitable for public water source with filtration and disinfection or other required treatment. A waste management zone is a specific reach of waters designated by a permit to accept the discharge of properly treated wastes that prior to treatment contained organisms pathogenic to human beings.

1.4.2. Floodplain

The record drawings from 2004 identify the 100-year flood elevation as 308.50 feet. In 2014, the FEMA Flood Insurance Study No. 50007CV002B for Chittenden County dated August 4, 2014, provided updated flood profiles for the Winooski River. Based on the 2014 Winooski River flood profiles and the facility's approximate distance of 1,300 feet from cross-section "BO" on Flood Profile 65P, the **100-year** and **500-year** flood elevations at the location of the Richmond WWTF are **310.50 feet** and **313.77 feet**, respectively. New flood elevation determination resulted in a 2-foot increase in the 100-year flood

elevation from 308.50 feet to 310.50 feet. See Figure A-2 in Appendix A for the river profile at the Richmond WWTF site and Figure A-3 for the FEMA FIRMette Map. The Richmond WWTF is not located within a regulatory floodway of the Winooski River, however is located within a Special Flood Hazard Area as shown in the FEMA FIRMette Map Figure A-3 in Appendix A, .

The ground elevation around the Process Building, which houses the chemical storage, UV disinfection, and filter units, is 313.50 feet. While the Process Building will be protected from a 100-year flood event, there is risk of flooding during a 500-year flood event.

During Tropical Storm Irene on August 28, 2011, the lower level of the Process Building flooded due to the Winooski River backing up the outfall pipe and overtopping the UV channel which has a top of wall elevation of 309.54 ft. After this flooding event, a check valve was installed on the outfall and the operators installed a pump in the sump downstream of the effluent weir to discharge effluent flows to the outside grade during an emergency to prevent future flooding inside the WWTF building.

Inside the Process Building, the elevations of critical process components are as follows:

- Effluent wet well top of wall elevation = 313.30 ft
- Top of UV channel = 309.54 ft
- Top of filter tank wall = 313.22 ft

All of these critical component elevations are below the 500-year flood elevation and the UV channel walls are below the 100-year flood elevation. In the recent July 2023 flooding event, emergency use of a sump pump to lift flow to an external discharge point was used to prevent flooding of the Filter/UV Room.

The ground elevation and the finished floor elevation of the Operations Building is at approximately 314.00 feet and is protected from the 100-year and 500-year flood events.

Richmond is part of a County-wide regional All Hazards Mitigation Plan which discusses flooding risk to the Town. A link to the plan is provided: <u>https://www.ccrpcvt.org/wp-content/uploads/2024/01/Annex-11-Town-of-Richmond_Adopted.pdf</u>

1.4.3. Wetlands

There are no classified wetlands located on the property as shown in Figure A-4 in Appendix A. A Wetlands Advisory Layer borders the north-west side of the property. The State of Vermont defines the Wetlands Advisory Layer to be wetland locations which have not been formally assessed for significance.

1.4.4. Rare and Endangered Species

No portions of the WWTF property are located in an area designated with the element occurrence of a rare or endangered animal or plant as shown in Figure A-5 of Appendix A.

1.4.5. Archeological Resources

Pending

1.4.6. Historical Preservation

Pending

1.5 Population Trends

The United States Census Bureau population data for the Town of Richmond from 1990, 2000, 2010, and 2020 were 3,729, 4,090, 4,081, and 4,167 respectively. Population data for the Town of Richmond is shown below in Table 1-1.

Census Year	Population	Previous 10-Year Growth (+/-)
1990	3,729	-
2000	4,090	9.68%
2010	4,081	-0.22%
2020	4,167	2.11%

Table 1-1 United	States Census	Information	Richmond	Vermont
Table T-T Olliced	Julies Cellous	mornation	, incrinitiona,	vermont

1. From US Census Data

Historical census data shows positive growth in the Town of Richmond from 1990 to 2020. The Town of Richmond's 2018 Town Plan states "population predictions show a relatively stable population over the next 10-15 years (ranged from a decline of about 180 people to an increase of about 35 people)". The Town of Richmond is well suited for population growth due to its proximity to the greater Burlington area and location along the I-89 corridor. Along with this steady population growth, the Town is looking to extend the municipal wastewater service area to zoned growth areas of the Town that are growth-limited by on-site subsurface disposal systems.

1.6 Community Engagement

1.6.1. Public Participation

The Town of Richmond actively engages the community and promotes public participation through the following:

- Public Meetings
- Local Newspaper Advertisements
- Front Porch Forum Postings
- Direct Mailings

1.6.2. Environmental Justice

The US Environmental Protection Agency (EPA) Environmental Justice Screening and Mapping Tool (EJScreen, Version 2020) was consulted to assess environmental justice and demographic indicators for the Richmond WWTF project site. Per the EPA, EJScreen is used to "screen for areas that may be candidates for additional consideration, analysis or outreach as EPA develops programs, policies, and activities that may affect communities." Figure 1-2 presents the demographic indicator results of the Richmond WWTF project screening





Figure 1-2 environmental Justice Demographic Indicators for Richmond WWTF (Source: https://ejscreen.epa.gov/mapper)

The results indicate that within a one-mile radius of the Richmond WWTF, the following statements are true:

- 34% of the State has a higher population of people of color while 81% of the US has a higher population of people of color
- The State and US population percentiles for low income population, unemployed population, and less than High School education are similar
- 93% of the State has a higher low income population
- 63% of the State has a higher unemployed population
- 73% of the State has a higher population with less than High School education
- 43% of the State has a greater population under the age of 5 while 53% of the US has a greater population under the age of 5
- 65% of the State has a greater population over the age of 64 while 43% of the US has a greater population over the age of 64

2. EXISTING FACILITIES

2.1 Location Map

A location map is shown in Figure A-1 in Appendix A.

2.2 History

The Town of Richmond owns and operates the Richmond Wastewater Treatment Facility and associated 4.14 miles (21,880 linear ft) of sewer mains and the Bridge Street pump station that make up the collection system within the service area. The history of the facility is as follows:

- 1972: Original Facility Constructed
- 2005: Major Facility Upgrade (New Headworks Building w/ Grit Removal, Anoxic Selectors, Aeration Tank Diffusers, New Process Building w/ Filtration & UV Disinfection, New Aerated Septage Holding Tank)
- 2023: One influent pump replaced with a used emergency pump

Although some specific equipment upgrades and replacements have occurred in the past, the majority of the facility has not been upgraded since the major upgrade in 2005. The existing WWTF site plan and hydraulic profile are provided in Figures A-6 and A-7 respectively in Appendix A.

2.3 Existing Discharge Permit

The Richmond WWTF is permitted under a National Pollutant Discharge Elimination System (NPDES) Permit No. 3-1173, effective January 1, 2021, to discharge treated effluent from outfall S/N 001 to the Winooski River.

Table 2-1 summarizes the WWTF's existing discharge permit flow and effluent quality requirements. The current version of the NPDES permit and fact sheet are publicly available at:

https://dec.vermont.gov/watershed/wastewater/discharge-permits

Effluent Parameter	Annual Avg	Annual Total	Monthly Average	Weekly Average	Daily Maximum	Instantaneous Maximum
Flow	0.222 MGD					
BOD ₅			30 mg/L 55.5 lbs/day	45 mg/L 83.3 lbs/day	50 mg/L 	
Total Phosphorus (TP)		 134 lbs/yr	0.8 mg/L 			
Total Nitrogen (TN)					Monitor Only	
Total Kjeldahl Nitrogen (TKN)					Monitor Only	
Nitrate/Nitrite Nitrogen (NOx)					Monitor Only	
Settleable Solids						1 ml/L
Total Suspended Solids (TSS)			30 mg/L 55.5 lbs/day	45 mg/L 83.3 lbs/day	50 mg/L 	
E. Coli						77 col/100 mL
рН			Between	6.5 and 8.5 Star	ndard Units	

 Table 2-1 Richmond WWTF Current NPDES Discharge Permit

2.4 Original Design Criteria

Table 2-2 summarizes the original WWTF design criteria, as well as current loadings.

Table 2-2 Existing Influent Design	Criteria and Current Loadings
------------------------------------	-------------------------------

Parameter	Design Criteria ¹	Current Loadings ²	
Average Daily Flow	0.222 MGD	0.073 MGD	
Peak Hourly Flow	1.152 MGD ³	0.660 MGD ⁴	
Rischamical Oxygon Domand (ROD)	324 mg/L	670.2 mg/L⁵	
Biochemical Oxygen Demand (BOD)	600 lbs/day	461.7 lbs/day	
Total Suspended Solids (TSS)	272.5 mg/L	932.0 mg/L⁵	
Total Suspended Solids (155)	500 lbs/day	680.5 lbs/day	
Total Phasehorus (TD)	10 mg/L	19.5 mg/L⁵	
Total Phosphorus (TP)	18.5 lbs/day	14.0 lbs/day	

Notes:

- 1. Basis of Final Design, 2003.
- 2. Based on historic operating data from January 2018 to February 2023.
- 3. Noted in 2003 Basis of Final Design as "based on influent pumping capacity."
- 4. Additional data is needed to verify current peak hourly flow.
- 5. Influent sampling is taken at the influent channel of the wet well, upstream of RAS and pressate side streams. Samples can, at times, include return activated sludge (RAS) & pressate, when the wet well is used for flow equalization and an isolated influent sample is not possible.

The original design criteria from the 2003 Basis of Final Design was established as a daily organic load (lb/day). These loads were converted to a concentration (mg/L) based on the design average daily flow of 0.222 MGD.

Historical influent data was analyzed and revealed that the current influent TSS load of 680.5 lb/day TSS exceeds the original design criteria of 500 lb/day TSS. While historical influent BOD and TP loads do not exceed the original design criteria, the historical influent concentrations are greater than the equivalent original design concentrations. Currently, Richmond is operating at an average daily flow of 0.073 MGD, which is approximately 33% of the design flow. Given that the current TSS load exceeds original design criteria, and that current BOD and TP concentrations are greater than the equivalent design criteria concentrations, a closer look needs to be taken into sources of these high influent concentrations and evaluate ways to reduce influent loading to the WWTF. If current influent concentrations were to remain constant at the design flow of 0.222 MGD, the original design criteria would be severely exceeded.

2.5 Historical Operating Data

Historical operating data was reviewed from January 2018 through February 2023.

2.5.1. Flow

The Richmond WWTF records effluent flow from the v-notch weir located on the effluent channel. The average monthly effluent flow from January 2018 to February 2023 ranged from 0.05 to 0.11 MGD with an average of 0.073 MGD which is 31.5% of the design influent average daily flow of 0.222 MGD.

The peak day effluent flow from January 2018 to February 2023 ranged from 0.07 to 0.59 MGD with an average of 0.14 MGD. The 2003 basis of design did not include design criteria for peak day flow.

The peak instantaneous effluent flow from January 2018 to February 2023 ranged from 0.27 to 0.66 MGD with an average of 0.54 MGD. The effluent flow measurement has a maximum range of 0.66 MGD that likely has been exceeded. The 2003 basis of design peak hour flow design criteria was 1.152 MGD based on influent pumping capacity. The historic maximum peak instantaneous flow of 0.66 MGD is approximately 57.3% of the existing peak hour flow design flow, although it is suspected that this has been exceeded. Further investigation or additional flow monitoring is necessary to determine the facility's peak instantaneous flow to establish a reliable peaking factor. Determining an accurate peak hour flow is important for influent pump selection to be sure the pumps can convey peak hour flows throughout the facility.

Historical operating data suggested that the effluent flow meter maxes out at 0.66 MGD, which would indicate a calibration issue or a malfunctioning flow meter. To solve this issue, the effluent flow meter was corrected and recalibrated in early October 2023. Flow data was monitored after October 2023 to monitor peak flows. During the rain event on December 18, 2023, the flow meter recorded a peak instantaneous flow of 2.3 MGD.



Figure 2-1 Historical Effluent Flow Data

While flow data includes the flow of pressate, there is no flow meter located on the dewatering pressate line that returns to the influent wet well. Therefore, it is difficult to understand the volume of pressate flow entering the biological process and the nutrient loads associated with pressate separate from municipal influent.

2.5.2. Biochemical Oxygen Demand (BOD)

Influent sample collection occurs at the influent wet well at the Richmond WWTF. Sample collection typically consists of combining grab samples over an 8-hour period to create a composite sample. The influent wet well receives flow from the collection system, return activated sludge (RAS), tertiary filter backwash, and pressate from the rotary press, however Richmond WWTF operators collect samples at the influent to the wet well, upstream of RAS and on days when dewatering is not running and pressate is not entering the wet well. While influent sampling does not necessarily isolate collection system quality from the RAS and pressate; historically, samples contain mostly municipal influent.

Influent BOD concentrations from January 2018 to February 2023 ranged from 170 to 1,700 mg/L with an average concentration of 670 mg/L, which is well above the 2003 influent BOD loading design criteria equivalent concentration of 324 mg/L. The variability in BOD concentration may be impacted by the volume of septage received and subsequently dewatered and the timing of when the grab sample is collected, however it is suspected that there may be significant industrial users on the collection system that are contributing to the high BOD concentration seen in the influent samples. The average historical BOD load associated with the influent concentration is 462 lbs/day, which is below the 2003 influent BOD loading design criteria of 600 lbs/day.

Influent BOD concentrations have been decreasing over this time period. The average influent BOD concentration from January 2018 through December 2020 was 754 compared to 554 mg/L from January 2021 to February 2023.



Effluent BOD concentrations ranged from 1 to 17 mg/L with an average of 2.86 mg/L. The average effluent BOD load was 1.96 lb/day which is well below the permitted monthly average load of 55.5 lb/day. Based on this data, the existing biological process provides conditions that support effective BOD removal at current loadings. Over this period of time, BOD removal ranged from 98.53-99.90+% with an average of 99.60% removal.



Figure 2-3 Historical Effluent BOD Data

2.5.3. Total Suspended Solids (TSS)

Influent TSS concentrations from January 2018 to February 2023 ranged from 271 to 4,120 mg/L with an average of 932 mg/L, which is well above the 2003 influent TSS loading design criteria concentration of 272.5 mg/L. The variability in TSS concentration may be impacted by the volume of septage received and subsequently dewatered when the grab sample is collected, however it is suspected that there may be significant industrial users on the collection system that are contributing to the high TSS concentration seen in the influent samples. The average TSS load was 680.5 lb/day TSS which is 1.36 times the 2003 design influent TSS load. Note, the influent TSS loading design criteria does not include side streams included in the influent sampling (RAS, pressate).



Figure 2-4 Historical Influent TSS Data

The Richmond WWTF has been performing well and removes between 98.15-99.95+% of TSS. From January 2018 to February 2023, the effluent TSS concentration ranged from 1.00 to 6.00 mg/L, with an average of 2.65 mg/L, which is well below the permitted monthly effluent TSS concentration of 30 mg/L. The average effluent TSS load from January 2018 to February 2023 was 1.86 lb/day which is also well below the permitted monthly effluent TSS load of 55.5 lb/day. Data indicates the WWTF has the capacity to treat the current TSS load.

Richmond Wastewater Treatment Facility 20-Year Evaluation Section 2 – Existing Facilities



Figure 2-5 Historical Effluent TSS Data

2.5.4. Total Nitrogen (TN)

Historically, influent nitrogen samples have not been collected at the Richmond WWTF. However, a 24-hour sampling event took place from 9/20/2022 to 9/21/2022 which analyzed ammonia concentrations from composite samples taken at several different locations throughout the facility. Ammonia concentrations from the 24-hour sampling event are displayed in Table 2-3.

Sampling Location	Ammonia Concentration (mg/L)	
Influent Wet Well	30	
Septage	74	
Pressate	44.5	

Table 2-3 Sampling	Event Ammonia Data
--------------------	--------------------

Richmond WWTF's permit requires quarterly reporting of effluent total nitrogen (TN), total Kjeldahl nitrogen (TKN), and nitrite and nitrate (NOx). In addition to these monitoring requirements, effluent ammonia is tested periodically. Data available for nitrogenous species monitoring is from January 2021 through February 2023.



Figure 2-6 Historical Effluent Nitrogen

Effluent TN concentrations ranged from 9.49 to 28.60 mg/L with an average of 19.66 mg/L indicating that the system has some potential for denitrification. Effluent NOx concentrations ranged from 8.10 to 27.20 mg/L with an average of 17.52 mg/L. Effluent TKN concentrations ranged from 0.15 to 7.60 mg/L with an average of 2.35 mg/L indicating the system maintains effective nitrification.

2.5.5. Phosphorus

Influent total phosphorus (TP) concentrations from January 2018 to February 2023 ranged from 4.8 to 73 mg/L with an average of 19.45 mg/L. Pressate total phosphorus concentrations can be high strength, which can contribute to the total phosphorus concentration in the wet well if samples were collected on days when the press was running, however it is suspected that there may be significant industrial users on the collection system that are contributing to high TP concentrations seen in the influent samples. The historical influent TP concentration of 19.45 mg/L exceeds the existing influent TP design criteria is 10 mg/L.



Figure 2-7 Historical Influent Phosphorus Data

Effluent TP concentrations from January 2018 to February 2023 ranged from 0.03 to 0.39 mg/L with an average of 0.10 mg/L which is well below the permitted average monthly effluent TP concentration of 0.80 mg/L. Monthly average effluent TP load ranged from 0.01 to 0.40 lbs/day with an average of 0.07 lbs/day. In addition to the permitted monthly average effluent TP limit, the facility has an annual permitted effluent TP load of 134 lbs/year, equivalent to approximately 0.37 lbs/day. The monthly average effluent TP load of 0.07 lbs/day is equivalent to 25.6 lbs/year. Over this time period, phosphorus removal ranged from 97.10-99.88+% with an average removal of 99.36%. Data indicates that the Richmond WWTF has the capacity to treat the current total phosphorus load. Overall, the facility achieves excellent TP removal.



Figure 2-8 Historical Effluent Phosphorus Data

2.5.6. E. Coli

The Richmond WWTF had no exceedances of the permitted instantaneous maximum e. coli limit of 77 counts per 100 mL as displayed in Figure 2-9. From January 2018 to February 2023, maximum effluent e. coli ranged from <0.10 to 16 counts per 100 mL.

Richmond Wastewater Treatment Facility 20-Year Evaluation Section 2 – Existing Facilities



Figure 2-9 Effluent E. Coli

2.5.7. Septage Received

The monthly septage received from January 2018 to February 2023 ranged from 27,300 to 1,200,500 gallons per month with an average of 610,995 gallons per month. The Richmond WWTF was designed to accept 2,000,000 gallons of septage per year, or approximately 167,000 gallons per month. In 2021 and 2022, the Richmond WWTF received a total of 9,087,100 and 7,626,000 gallons per year, respectively. The facility is currently accepting significantly more septage than the original design, which is possible as significant available loading at the facility was freed up when the cheese production factory closed in 1999. Effluent quality continues to meet permit limits.



Figure 2-10 Historical Septage Receiving

Septage receiving is fed through a septage receiving unit prior to mixing with waste activated sludge (WAS). The combination of septage and WAS is then dewatered and pressate is directed to the influent wet well where it combines with municipal influent. Due to the lack of a flow meter and historical sampling of pressate, it can be difficult to draw conclusions on the impact of septage receiving and pressate return on the process. However, on 9/20/2022 through 9/21/2022, a sampling event took place at the Richmond WWTF as part of the plant's Phosphorus Optimization Plan study. Data was collected on both the raw septage and the pressate. Sampling results are presented below in Table 2-4.

Location	BOD [mg/L]	TSS [mg/L]	VSS [% dry wt]	Orthophosphate [mg/L PO ₄ -P]
Septage	1,400	1,760	85.8	7.83
Pressate	110	458	77.5	4.50

Table 2-4 Septage & Pressate Sampling Results

2.5.8. Industrial Users

Stone Corral Brewery, which discharges to the Richmond WWTF is considered a significant industrial user and is permitted under Pretreatment Discharge Permit No. 3-1560. The Richmond WWTF can receive a monthly average flow of up to 1,000 gpd from Stone Corral Brewery. The Brewery has a monthly average discharge effluent limit of 100 lb/day of BOD. The Brewery is required to test for BOD in their discharge 1x/week, TSS 2x/month, and TP 1x/month.

The Town of Richmond should consider evaluating local commercial and industrial users that may be discharging a significant load as one way to understand the influent nutrient load that is historically experienced at the WWTF.

2.6 Condition of Existing Wastewater Treatment Facility

Hoyle Tanner conducted a site visit on May 24, 2023, to the Richmond WWTF to assess the physical condition of WWTF process components and the site. The following section presents the findings of that assessment.

2.6.1. Influent Pumping

Raw wastewater from the collection system enters the Richmond WWTF at the influent wet well, located next to the Operations Building, where it is mixed with RAS, pressate, and filter backwash. The roof drains for the Operations Building also drain to the wet well. The wet well is divided into two cells and the operators indicated that only one side of the wet well is being used due to problems with rags, bricks and stones that make their way through the collection system. A level transmitter in the wet well controls the pumping speed of the influent pumps which are located inside the Operations Building basement. The concrete of the wet well structure was not inspected during the field visit and the condition of the structure is unknown.



The influent pumping system consists of two influent pumps located in the lower level of the Operations Building. Influent pumping transfers wastewater from the wet well to the

Figure 2-11 Influent Wet Well



Figure 2-12 Influent Pumping System

Headworks Building where it flows by gravity through the remaining treatment process. An in-line grinder that was part of the original design has been disconnected. There is a magnetic flow meter on the pump discharge header to measure flows. The operators indicated that they do not believe it accurately measures flow. Influent Pump #1 is driven by a 40 HP motor and was installed in 2023 as a temporary replacement for the existing Influent Pump #1 which had failed. Influent Pump #2 is a vertical mounted, flooded suction centrifugal pump driven by a 25 HP motor. Currently, Influent Pump #2 is not in use as it has a leaking seal and sprays the room when operating. The operators indicated that they have replaced both pump discharge check valves recently.

Both pumps are equipped with a variable frequency drive (VFD) which is controlled by the level transmitter in the influent wet well. The operators indicate that they let the wet well back up to the first landing during high flow events and use it for flow equalization. They indicated that they can use the wet well up to this level and back up the influent sewer to within 1 foot of the rim of the upstream manholes in the park, which are elevated for flood proofing.

Access to the pumps is problematic with no overhead hatch to hoist pumps through. The operators noted that they have to bring equipment down three flights of stairs if maintenance is required.

The facility experiences significant large solids carried to the wet well through to the influent pumps including large rocks/bricks, large pieces of wood, rag balls greater in diameter than the pumps can pass. These materials often lead to pump clogging, damage to the pump impellers and emergency maintenance of the pumps. The operators indicated that the Town cleaned out the influent collection sewer up to Bridge Street in 2022 to try to mitigate the amount of debris that enters the wet well and pump inlet.

Description	Existing Design
Influent Wet Well	
Dimensions	26'8" L x 11'6" W x 2' D
Operating Levels	Normal: 2'
	Max: approx. 18'
Operating Volume	1,032 gal (at normal operating level)
Influent Pump #1	
Manufacturer/Model	Cornell Pump Co. GNHTA-CSV 40-4
Motor	40 HP
Nameplate RPM	1775 rpm
Influent Pump #2	
Туре	Centrifugal, Vertical Mounted, Flood Suction
Motor	25 HP, VFD Driven
Capacity	800 gpm @ 65' TDH (each)
Flow Meter	
Type/Manufacturer	Magnetic/Siemens/ MAG6000
Max Flow Read	800 gpm

Table 2-5 Influent Wet Well & Pump Existing Design Information

Design Standards

• Convey peak design flow with largest pump out-of-service.

Assessment

The assessment of the major components for the influent wet well, influent grinder, and influent pump station are summarized in Table 2-6, and the major needs are described as follows:

Findings:

- Influent Pump #1 failed and was replaced with a used emergency pump in 2023. The pump is old and only a temporary solution.
- Influent Pump #2 is original to facility. The pump is in poor condition with a leaking seal. This pump is at risk of imminent failure leaving the facility with no pumping redundancy.
- Influent pumps routinely experience problems such as ragging and bricks/rocks causing damage.

- Influent grinder has been removed.
- Hatch not available for pump removal
- Roof drains are connected to the wet well.
- The concrete of wet well structure was not evaluated. It is recommended that a concrete assessment be conducted.
- Additional flow data is required to select influent pump replacements.

Item		Ra	nk of Ex Condit	kisting ion	5	Year	Projected	Notos
	Poor		Fair Go		od	Installed	Life (vears)	NOLES
	1	2	3	4	5		Life (years)	
Wot Wall			~			1072	20+	Concrete should be
vvet vven			^			1972	20+	inspected.
								Used emergency
Influent Pump #1	х					2023 (used)	0-2	pump, not permanent
								solution
Influent Duran #2	v					1072	0.2	Leaking seal,
innuent Pump #2	^					1972	0-2	imminent failure
Pump VFDs			х			2005	2-5	
Level Control			×			2005	2 5	
System			×			2005	2-5	
Influent Grinder	Х					2005	-	Has been removed

Table 2-6 Influent Wet Well and Pump Station Assessment

2.6.2. Headworks Assessment

Screening and grit removal consists of a package unit located in the Headworks Building. Screening and grit removal is accomplished using a combination unit that uses a rotary-type mechanical fine screen and an aerated grit removal chamber with a grit dewatering screw to screen inorganics and remove grit from the process stream. Wastewater is pumped to the unit via the influent pumps. Wastewater flows into the unit's screening basket where solids are retained. Screened material is removed from the screening basket and is spray washed to return organics to the process stream. The screened material is then transported up the unit's central screw conveyor, compacted, dewatered, and discharged to storage containers. The wastewater that flows through the screening basket passes directly into a grit removal chamber. Grit settles to the floor of the grit chamber and a grit transport screw moves the settled grit to a lateral sump. A grit dewatering screw transports the settled grit out of the lateral sump and dewaters the grit before it is discharged into storage containers. Air for the aerated grit chamber is fed from a new blower located in the one-room structure located over the aerobic sludge digester tank. Flow from the grit chamber is discharged directly into the first anoxic



Figure 2-13 Grit Removal

selector. The headworks can be bypassed by pumping directly to the aeration basins.

The rotary-type fine screen shaft has significant wear and while the screen does catch some rags, the operators indicate that it doesn't work well. The grit auger has significant wear and has been patched over the years. The operators indicated that grit that is captured is not being properly dewatered resulting in a product with a mud-like consistency. The operators have also noted a leak in the stainless steel package unit tank, which they have been patching regularly.

The Headworks Building's gas detection system is inoperable, and the ventilation system is activated by the light switch, indicating occupancy. It could not be determined if the existing ventilation meets the required air changes for this Class I Division 1 Hazardous space.

The existing design information for the screening and grit removal equipment is presented in Table 2-7 on the following page.

Description	Existing Design							
Screening & Grit Removal Package								
Quantity	1							
Manufacturer	Lakeside Equipment Corp.							
Type/Model	Complete Plant/SO 03-191							
Rotary Screen								
Туре	Mechanical, Fine Screen							
Bar Spacings	3/8"							

Table 2-7 Screening and Grit Removal Design Information

Motor Power Draw	2 HP
Capacity	2.97 MGD
Screen Openings Spacing	1/2"
Screen Basket Diameter	31"
Grit Chamber	
Number of Units	1
Dimensions	26' x 3.5'
SWD	9.17′
Capacity (Maximum Flow Rate)	2.97 MGD
Grit Transfer Screw	
Diameter	8"
Motor Drive	1 HP
Diameter	8"
Motor Drive	2 HP
Grit Blower	
Quantity	1
Туре	Rotary Positive Displacement Blower
Power Draw	2 HP
Design Capacity	26 scfm

Design Standards

- <u>Bypass Screens</u>: Installations using mechanically cleaned screens or comminution devices should include multiple units or a manually cleaned bypass screen. (TR-16 Standards)
- <u>Manually Cleaned Screens</u>: Unobstructed openings between bars should be 1–2 inches (2.5–5 cm) wide. Manually cleaned screens should be placed on a slope of 30–45 degrees with the horizontal. (TR-16 Standards)
- <u>Mechanically Cleaned Screens</u>: Unobstructed openings between bars are generally 0.25–1.5 inches (0.6–3.8 cm) wide. (TR-16 Standards)
- <u>Velocities</u>: Screen chambers should provide good velocity distribution across and through the screen. Approach velocities in screen channels should be at least 1.3 feet per second at minimum flows (2.0 ft/sec is preferred if possible), or 2.5 ft/sec during diurnal peak flow periods. Approach velocities in screen channels serving combined systems should be at least 3 ft/sec during storm flows. Velocities through openings of mechanically cleaned screens should be 2–4 ft/sec. Velocities through manually cleaned screens should be limited to 1–2 ft/sec. (TR-16 Standards)
- <u>Grit Removal</u>: Grit can be removed in grit chambers or by centrifugal separation of primary sludge. Acceptable grit chambers include aerated, vortex (including induced vortex and multi-tray vortex units), detritus, and horizontal flow (velocity control tanks) units. A single, manually or mechanically cleaned grit chamber with bypass is acceptable for small plants serving sanitary sewer systems. (TR-16 Standards)
- <u>Fire Protection</u>: For coarse and fine screen facilities, grit removal tanks, and pre-aeration tanks that are continuously ventilated at 12 air changes per hour, the entire enclosed space is classified as Class I, Division 1. This space requires a portable fire extinguisher, combustible gas detection system, and hydrant protection.

Assessment

The assessment of the major components for the Headworks is summarized in Table 2-8 on the following page, and the major needs are described as follows:

Findings:

- The stainless tank of the package screening and grit unit is leaking.
- The effectiveness of screening and grit removal is poor.
- Grit auger is worn and has been repeatably patched.
- Grit is not properly dewatering, creating a mud-like consistency.
- Gas detection in the building is inoperable.
- Existing ventilation may not provide adequate air changes.
- No drain in the sump where package unit sits. Operators must use a trash pump to drain if level gets too high.

Item		Ra	nk of Ex Condit	kisting ion	5	Veer lestelled	Projected	Natas				
	Poor		Fair Go		od	rear installeu	Kemaining	Notes				
	1	2	3	4	5		Life (years)					
Rotary Screen		х				2005	2-5					
Grit Chamber		х				2005	10+					
Grit Blower		х				2005	2-5					
Grit Transfer		x	x	x	v	V				2005	2 5	
Screw								2005	2-5			
Grit Dewatering		x	x				2005	2 5				
Screw							2005	2-5				
Headworks				X			2005	10				
Building			x	X	~			2005	10+			
Ventilation		х				2005	2-5					
Gas Detection	х					2005	0	inoperable				

Table 2-8 Headworks Assessment

2.6.3. Biological Process

The biological treatment system at the Richmond WWTF consists of anoxic selectors followed by aeration tanks. The first part of the process consists of three anoxic selectors located inside the Headworks Building. The first two anoxic selectors each have a capacity of 3,000 gallons while the third anoxic selector has a 6,000 gallon capacity. Mixing throughout the selectors is achieved with submersible mixers. Upon the time of inspection, the submersible mixer in the first anoxic selector is inoperable and only one of the mixers has a Class I, Division 1 explosionproof motor. Since the site visit, the submersible mixer in the first anoxic selector has been repaired.

Flow from the third anoxic selector is designed to split between two parallel aeration tanks, each having a volume of 150,000 gallons. Typical operation consists of one aeration tank in operation at a time. The aeration tanks have a fine bubble diffused air system and are fed air from two (2) 25 HP positive displacement blowers, each with a capacity of 450



Figure 2-14 Biological Aeration Tank

scfm, located in the Process Building upper level. A single dissolved oxygen (D.O.) probe is provided but is not connected to SCADA for blower operation. The operators indicated they would like one D.O. probe for each tank. The outdoor intake for the aeration blowers has been disconnected due to noise complaints, and air for the blowers is drawn from the septage holding tank in the Operations Building Garage. This air has significant hydrogen sulfide concentration for the air space above a non-aerated sludge holding tank. Blower control is cycled on/off from SCADA; however, D.O. concentration in the

aerated basin is not being used to inform the aeration schedule.

The aeration tanks are original to the Richmond WWTF and were constructed in 1972. The concrete of the tank structures was not evaluated during the site visit. The diffusers were not inspected during the site visit, however a uniform bubble pattern in the operating tank suggests that the diffusers are functioning as intended. The operators noted that Aeration Tank #1 was drained and cleaned out recently, however, Aeration Tank #2 has not been cleaned since the upgrade. The operators indicated that it is difficult to balance air in the air header between the tanks using manually operated butterfly valves and would like to see dedicated air lines from each blower to each aeration tank.

Aeration tank effluent is discharged from each tank over a fixed weir into an effluent channel where coagulant for chemical phosphorus removal is applied.

The existing design information for the biological system is presented in Table 2-9.

Description	Existing Design
Anoxic Selector 1 & 2	
Quantity	2
Capacity	3,000 gal (each)
Dimensions	8.25′ x 6.0′
SWD	8.10'
MLSS Target	Pending
Anoxic Selector 3	
Quantity	1
Capacity	6,000 gal
Dimensions	16.50' x 6.0'
SWD	8.10'
MLSS Target	4,000 mg/L
Submersible Mixers	
Quantity	3 (1/anoxic tank)
Туре	Submersible Mixers
Manufacturer	Flygt
Power Draw	1.21 HP (each)
Aeration Basins	
Quantity	2
Dimensions	38' x 38'
SWD	14'
Volume	150,000 gal (each)
Aeration System	
Type/Manufacturer	Fine Bubble Flexible Membrane Diffuser/Sanitaire Aeration System
Quantity	162/tank
Discharge Pressure	7 psig

Table 2-9 Biological Treatment System Existing Design

Blowers	
Quantity	2
Manufacturer	Aerzen USA
Motor	25 HP (each)
Capacity	220 to 450 scfm
Control Strategy	DO Control (not operable)
VFD	Yes

Design Standards

- Liquid depths should not be less than 10 feet or more than 25 feet. (TR-16)
- Aeration systems should be sized for the maximum daily oxygen requirements (considering facility side streams, and seasonal variations in temperature and humidity) while maintaining an aeration basin DO concentration of 2 milligrams per liter. (TR-16)
- Oxygen supply should be designed based on 0.85–1.2 pounds of oxygen per pound of BOD removed plus 4.2 pounds of oxygen per pound of ammonia nitrogen oxidized at maximum daily loading conditions. (TR-16)
- Blower capacity must be based on the air volume required during summer temperature and humidity conditions. The size of motors for centrifugal compressors must be based on summer air flow rates and the coldest expected winter temperature (or other means provided to control mass air flow rate and prevent motor overload). (TR-16)
- Blower controls should be incorporated into the system, providing sufficient ability to meet oxygen demand in the various tanks in service through multiple blowers, variable blower output, dissolved oxygen monitoring, air flow measurement, and automated control valves. (TR-16)
- The size of air piping should be based on maximum expected summer temperatures and in-line velocities of 2,000–2,500 feet per minute. (TR-16)
- Fine bubble, full-floor coverage: 0.12 scfm per square foot of tank area. (TR-16)

Assessment

The assessment of the major components for biological process is summarized in Table 2-10 on the following page, and the major needs are described as follows:

Findings:

- Only mixers in anoxic selectors #2 and #3 are operable, and only one mixer has an explosion-proof motor.
- Aeration Tank #2 has not been cleaned out since the 2005 upgrade.
- Blowers cycle on/off from SCADA but are not controlled by the D.O. probe in the active aeration basin. New D.O. probes are needed.
- Blower intake is from a sealed aerated solids holding tank and shows significant signs of corrosion in the garage, outdoor air intake was disconnected due to noise complaints.
- Air balance to the aeration tanks is challenging due to manually operated butterfly valves.
- The concrete tank structures of the anoxic tanks nor aeration tanks were not evaluated. It is recommended that a concrete assessment be conducted.
- Data on mixed liquor suspended solids (MLSS) is not collected at the facility.

Item		Ra	nk of Ex Condit	kisting ion	5	Veer Instelled	Projected	Notes
	Poor		Fair	Go	od	real installeu	Life (veers)	Notes
	1	2	3	4	5		Life (years)	
Anoxic Tanks			х			2005	20+	Concrete should be inspected
Submersible Mixers		x				2005	2-5	Mixer #1 is inoperable. Do not have ex-proof motors on all mixers
Aeration Tank concrete			х			1972	20+	Concrete should be inspected
Aeration Tank walkway/railings				х		2005	20+	
Diffusers			х			2005	5-10	
Blowers			x			2005	5-10	Blower intake from septage holding tank is severely corroded.
VFDs			Х			2005	2-5	
DO Probes		Х				2005	0-2	New probes need

Table 2-10 Biological Treatment System Assessment

2.6.4. Coagulant Chemical Feed and Storage

There is one (1) 1,500 gallon sodium aluminate storage tank and two (2) positive displacement diaphragm type feed pumps located in the basement of the Process Building, though only one (1) feed pump is in use and the other is in storage. Secondary containment is provided in the form of a 3-ft high concrete retaining wall around the coagulant storage tank. An emergency eyewash station is located next to the coagulant storage tank; however, there is no emergency shower. The original design allowed for coagulant to be pumped to the aeration tank effluent channel and/or into both clarifier effluent pipes prior to the filter units. Typical operation is consistently dripping sodium aluminate through a 2" line into the aeration tank effluent channel and is based on an operator feed rate that is determined by effluent pH levels.



Figure 2-15 Sodium Aluminate Storage
Description	Existing Design								
Coagulant Storage									
Coagulant	Sodium Aluminate (Alum)								
Storage Tank Capacity	1,500 gallons								
Containment Volume Required	1,875 gallons								
Containment Volume Provided	1,544 gallons								
Coagulant Feed – Process Building									
Feed Pump Type	Positive Displacement, Diaphragm Type								
Number of Pumps	2 (1 duty, 1 in storage)								
Flow Rates	0 – 48 gpd								
Application Points	Aeration Tank Effluent Launder								

Table 2-11 Chemical Storage & Feed Existing Design Information

Design Standards

- <u>Redundancy:</u> A minimum of two feed pumps, one duty and one standby, should be provided. (TR-16)
- <u>Location</u>: Chemical feed equipment should be located in a separate, dedicated room to reduce potential hazards and exposure. (TR-16)
- <u>Storage:</u> Space should be provided for at least 30-days of chemical storage under average design conditions. Tanks should have a liquid level indicator, overflow and receiving basin, and secondary containment. Secondary containment should be no less than 125% of the storage tank volume. (TR-16)
- <u>Eye-Wash Fountains and Emergency Showers:</u> Should be provided no more than 25 feet from points of hazardous chemical exposure and supply tempered water at 30-50 gpm and 20-50 psi for 15-30 minutes. (TR-16)

Assessment

The assessment of the major components of the chemical feed and storage are summarized in Table 2-12 on the following page, and the major needs are described as follows:

Findings:

- No emergency shower provided in chemical area.
- Only 1,544 gallons of containment provided, which does not meet design standards. The secondary containment volume required is 1,875 gallons.
- No redundant chemical feed pump installed on standby.

lt e us		Rar	nk of Ex Conditi	istir on	ng	Year	Projected	Netes
item	Poor Fa		Fair	Fair Good		Installed	Remaining	Notes
	1 2 3 4 5		Life (years)					
Coagulant Pumps			x			2005	2-5	One duty, one on shelf (should be installed as stand-by)
Coagulant Storage Tank				х		2005	20+	
Secondary Containment				х		2005	20+	Inadequate volume

Table 2-12 Chemical Feed and Storage Assessment

2.6.5. Secondary Clarification

Flow from the aeration tank effluent channel can be split between the two rectangular secondary clarifiers by a splitter box with stop gates. Typically, only one (1) clarifier is in operation at a time. Each rectangular secondary clarifier is 50-ft long and 11-ft wide. An access bridge extends the length of the clarifiers. The drives for the chain and flight scrapers are located on the walkway above each clarifier. Effluent troughs have v-notched weirs to control discharge from the clarifiers. Clarified effluent is discharged through a 12" pipe to filtration.

Flight scrapers scrape sludge along the bottom of the secondary clarifiers toward the influent end of each clarifier into a sump. A waste activated sludge (WAS) pump, located in the basement of the Operations Building, removes sludge from the clarifiers and transports it to the septage holding tanks. Return activated sludge (RAS) is drawn from the same



Figure 2-16 Secondary Clarifier

sump and conveyed back to the influent wet well via a gravity line. RAS flow is controlled by an electrically actuated pinch valve and flow is measured with a magnetic flow meter located on the RAS line.

The operators noted that they recently had drained both clarifiers, cleaned, and inspected the flight scrapers and both are in good working condition. Operators also noted that there was no grease skimmer on the clarifiers and that grease routinely passes through to the filters causing fouling of the cloth media.

Design information for the existing secondary clarifiers is presented in Table 2-13 on the following page.

Description	Existing Design	Notes
Secondary Clarifiers		
Number of Units	2	
Dimensions	50' x 11'	
SWD	10'	
Effective Weir Length	108'	
Surface Overflow Rate	202 gpd/sf @ ADF (0.222 MGD)	Meets design standards listed
(w/ 2 units in service)	1,047 gpd/sf @ PHF (1.152 MGD)	below
Weir Overflow Rate	2,056 gpd/ft @ ADF (0.222 MGD)	Meets design standards listed
(w/ 2 units in service)	10,667 gpd/ft @ PHF (1.152 MGD)	below
Sludge Collector		
Manufacturer/Model	FMC Corp. / Link-Belt Environmental	
	Equipment/ EE5057-G	
WAS Pumps		
Quantity	1	
Туре	Double Disc, Positive Displacement Pump	
Manufacturer	Penn Valley Pump Co. Inc.	
Motor	7.5 HP	
Rating	150 gpm @ 18' TDH	
Equipment	VFD	
RAS Valve		
Quantity	1	
Туре	Pinch Valve	
Manufacturer	Red Valve	
RAS Flow Meter		
Type/Manufacturer	Magnetic/Siemens	
Model	SITRANS FM MAG 6000	
Size/Length	4-inch	
Typical RAS Rate	90% of effluent	

Table 2-13 Secondary Clarifier Existing Design Information

1. Average RAS Rate = 91.4% of Effluent Flow (January 2018 – February 2023)

Design Standards

- Surface Overflow Rate @ PHF
 - Extended Aeration 1,000 gpd/sf, 1,200 gpd/sf Contact Stabilization based on influent only (10 State Standards)
 - 1,140 gpd/sf (TR-16 @ SVI = 150 mL/g, MLSS = 3,000 mg/L)
 - Facility does not have MLSS data, SOR for MLSS = 3,000 mg/L used
- Weir Overflow Rate
 - Maximum weir loading rate of 20,000 gpd/ft at PHF for plants with an average capacity equal to or less than 1 MGD. (10 State Standards)
- Peak Solids Loading Rate @ PDF + Peak RAS Flow
 - Extended Aeration 35 lbs/d/sf, Contact Stabilization 40 lbs/d/sf (10 State Standards)
- Minimum of 12 ft side water depth. (10 State Standards)

Assessment

The assessment of the major components for the secondary clarifiers is summarized in Table 2-14, and the major needs are described as follows:

Findings:

- Clarifiers are in good working condition.
- Concrete tank structures were not evaluated. It is recommended that a concrete assessment be conducted.
- The surface overflow rate @ PHF meets the TR-16 design standard, assuming MLSS = 3,000 mg/L. If MLSS is greater than 3,000 mg/L, then the SOR standard may be exceeded.
- Lack of MLSS data does not allow for accurate solids loading rate analysis on the secondary clarifiers.

ltow		Rai	nk of Ex Condit	kisting ion	5	Projected		Notos
item	Ро	or	Fair	Go	bod	Year Installed	Remaining	Notes
	1	2	3	4	5		Life (years)	
Clarifier #1 Drive				х		2005	10-15	
Clarifier #2 Drive				х		2005	10-15	
Internal Mechanisms				х		2005	10-15	
Launders, weirs				х		2005	10-15	
Tankage				х		1972	20+	Concrete repaired in 2005 upgrade.
Walkway/Railings				х		2005	20+	

Table 2-14 Secondary Clarifier Assessment

2.6.6. Return and Waste Activated Sludge Pump System

There is one (1) waste activated sludge (WAS) pump located in the Pump Room in the lower level of the Operations Building. The WAS pump is a 7.5 HP Penn Valley Pump double disc, positive displacement pump rated for 150 gpm at 18 ft TDH. The WAS pump moves sludge to the aerated holding basins. The original design also allowed for the WAS to be pumped to the Aerobic Sludge Holding Tank. Operators indicated that many of the valves on the discharge piping are frozen, although they are systematically rebuilding them one at a time. WAS is wasted two (2) times per day for 1-hour intervals and is controlled by a timer. Operators indicate that the pump model is obsolete, and it is hard to get parts.

RAS flows by gravity back to the influent wet well. RAS flow is controlled by an electrically actuated pinch valve and flow is measured with a Siemens magnetic flow meter. Operators indicated that the RAS flow rate is set at 110 gpm. Historical data from January 2018 through February 2023 indicates that RAS is approximately 90% of forward flow.



Figure 2-17 WAS Pump

Description	Existing Design
WAS Pump	
Quantity	1
Туре	Double Disc, Positive Displacement Pump
Manufacturer	Penn Valley Pump Co. Inc.
Motor	7.5 HP
Rating	150 gpm @ 18' TDH
Equipment	VFD
RAS Valve	
Quantity	1
Туре	Electrically Actuated Pinch Valve
Manufacturer	Red Valve
RAS Flow Meter	
Type/Manufacturer	Magnetic/Siemens SITRANS FM MAG 6000
Size/Length	4-inch

Table 2-15 RAS and WAS Existing Design

Design Standards

• At facilities with an average design flow of 10 MGD or less, waste sludge pumping facilities should normally be designed with a maximum capacity of 25 percent of the average design flow and should provide a minimum flow rate of approximately 80 gallons per minute (to allow velocity of 2 feet per second in a 4-inch diameter pipe) (10 State Standards).

• Suitable devices for observing, sampling, and controlling return activated sludge flow from each settling tank hopper shall be provided (10 State Standards).

Assessment

The assessment of the major components for sludge pumping is summarized in 2-16, and the major needs are described as follows:

Findings:

- The WAS pump is obsolete, and parts are hard to obtain.
- The RAS pinch valve and flow meter operate well, however should be inspected for wear.
- There is only one (1) WAS pump with no redundancy provided.
- Multiple WAS plug valves are frozen.

	Ranki	ng of E	xisting	; Cond	ition	Vear	Projected	
Item	Poor		Fair Good		od	Installed	Remaining	Notes
	1 2 3 4 5 Installed		Instaneu	Useful Life				
WAS Pump			x			2005	2-5	Pump model is obsolete and no redundancy is provided for
WAS plug valves		Х				2005	2-5	Many frozen valves
RAS Pinch Valve			х			2005	2-5	
RAS Flow Meter			Х			2005	2-5	

Table 2-16 Sludge Pumping Condition Assessment

2.6.7. Filtration

The tertiary process is located in the Filter/UV Room on the upper level of the Process Building and consists of two (2) steel package tanks, each containing two (2) cloth media disks with 5-micron fabric. Flow is split between the two units by way of a common manifold, though typical operation consists of one (1) unit in operation at a time. Flow can be throttled to each filter by means of a manual butterfly valve on the influent pipe to each filter unit.

Flow entering the filtration tank passes through the cloth membrane by gravity. Filtered water then enters the internal portion of the disk where it flows through the center-tube to an effluent box. Solids collected from the bottom of the tank are pumped via a 3 HP sludge removal pump to a drain line that drains back to the influent sewer. Filtered effluent flows by gravity via a 12" pipe to the UV disinfection system.



Figure 2-18 Filtration Tank

When a backwash cycle is initiated, the filter drive activates which rotates the filter disk at 1 RPM. The first backwash valve then opens, and the waste pump starts. Backwash is recycled to the influent wet well. Both backwash pumps were replaced in the Summer of 2023.

Operators indicated that the filter units have been going into continuous backwash, causing the operators to have to manually waste sludge from the bottom of the filter tank by opening a drain valve. One suspected reason is polymer carryover from dewatering clogging the cloth media surface. Another is grease and solids carryover from the secondary clarifiers fouling the cloth fabric. All cloth media was replaced within the past few years. The operators noted that the filters are not controlled by the SCADA system, and they have to manually turn on the second filter unit during high flows.

During the site visit, corrosion of the steel tanks was observed.

The influent weir for the filters is at 311.39 ft. and the top of the filter tank wall is at 313.22 ft, which are both below the 500-year flood elevation of 313.8 ft.

Design information for the existing filtration equipment is presented in Table 2-17.

Description	Existing Design					
Filter Tanks						
Number of Tanks	2					
Manufacturer/ Model Number	Aqua-Aerobic Systems, Inc. Cloth Media Filter ADFP-54X2E-PC					
Number of Disks, Total	4 (2, 2-Disk Units)					
Dimensions	8' x 9'-2"					
SWD	7.84'					
Max Water Level	9.61′					
	53.8 sf/disk					
Filter Area Provided	107.6 sf/unit					
	215.2 sf total					
	1.43 gpm/sf @ ADF (0.222 MGD) – one filter unit (2 disks)					
Hydraulic Loading Data	7.43 gpm/sf @ PHF (1.152 MGD) – one filter unit (2 disks)					
Hydraulic Loading Kate	0.72 gpm/sf @ ADF (0.222 MGD) – two filter units (4 disks)					
	3.72 gpm/sf @ PHF (1.152 MGD) – two filter units (4 disks)					
	0.52 lbs TSS/sf/day @ ADF (0.222 MGD) – one filter unit (2 disks)					
Solids Loading Pato ¹	2.68 lbs TSS/sf/day @ PHF (1.152 MGD) – one filter unit (2 disks)					
Solids Loading Nate	0.26 lbs TSS/sf/day @ ADF (0.222 MGD) – two filter units (4 disks)					
	1.34 lbs TSS/sf/day @ PHF (1.152 MGD) – two filter units (4 disks)					
Sludge Removal Pump						
Quantity	2					
Power Draw	3 HP					
Motor Drives						
Quantity	2					
Power Draw	½ HP					

Table 2-17 Filtration Existing Design

1. Assuming tertiary influent concentration of 30 mg/L TSS.

Design Standards

- Filter systems should be designed to accommodate peak hourly flows with one unit in backwash mode and to accommodate filters operating at design maximum headloss through filter media. (TR-16)
- Filters should include provisions for automatic bypass in the event of filter media binding as well as provisions for positive flow distribution. (TR-16)
- Effluent filtration systems should include automatic control features to initiate backwash based on intervals of time or on high filter headloss. (TR-16)
- Filter systems should be provided with instrumentation to monitor headloss and turbidity of both filter influent and effluent, and to monitor for filter influent and backwash flows. (TR-16)
- Disc filters should be housed in heated and ventilated enclosures. (TR-16)
- Loading rates at peak hourly flow should not exceed 6.5 gpm/sf of filter surface area. (TR-16)
- A minimum of two filter units should be provided. (TR-16)
- A minimum of 100% of peak hourly design capacity with largest unit out of service. (VTDEC Design Guidance)

Assessment

The assessment of the major components associated with the filtration system are summarized in Table 2-18 on the following page, and the major needs are described as follows:

Findings:

- Filter units have been going into continuous backwash as cloth media is fouled. Operators need to manually waste solids using drain valves at bottom of tank.
- Polymer carry over from dewatering has negatively impacted cloth media performance.
- Significant corrosion on the steel tank, trough, and rusted filter drains was observed.
- Ventilation in the filter room is sealed/shut off and is not functional.
- Filters not connected to SCADA for automatic operation at high flows, must be started manually.
- Suction of settled solids is not functioning properly
- Filter tank wall elevation is below the 500-year flood elevation.

litere		Rar	nk of Ex Conditi	Existing lition		Projected	Notos	
Item	Pc	or	Fair	Go	od	Year Installed	Kemaining	Notes
	1	2	3	4	5		Life (years)	
Filter Tanks			х			2005	5-10	Rust needs to be addressed
Filter cloth		х				2022	0-2	Cloth is routinely fouled.
Sludge Pumps		х				2005	0-2	Sludge pump is not working properly
Backwash Pumps			x			2023	10+	Both backwash pump replaced in Summer 2023
Motor Drives			Х			2005	2-5	
Backwash Valves			х			2005	2-5	

Table 2-18 Filtration System Assessment

2.6.8. Ultraviolet Disinfection

Ultraviolet (UV) disinfection is located in the Filter/UV Room on the upper level of the Process Building. The UV disinfection system consists of a package unit with two (2) banks of UV lamps in series with ten (10) modules in each bank and four (4) lamps in each module. The banks are housed in a stainless-steel channel with transition boxes for flanged pipe attachment at both inlet and outlet ends. A fixed serpentine weir located downstream of the second bank controls the surface water level and directs flow into an outlet transition box. Each bank provides full disinfection treatment at peak flows, providing full redundancy.

The operators indicated that the UV intensity meter was not working, however they have ordered a replacement and will install it soon. The operators also indicated that they have never had a disinfection violation.

It should be noted that the weir in the UV unit is at 308.95 ft and the top of UV channel is at 309.54 ft. These elevations are below the 100-year flood elevation of 310.5 ft and the 500-year flood elevation of 313.8 ft.

Design information for the existing ultraviolet disinfection system is presented in Table 2-19 on the following page.

Item Description	Existing							
UV Disinfection Design Conditions								
Average Daily Flow	0.222 MGD							
Peak Hourly Flow	1.0 MGD							
TSS	10 mg/L							
UV Dose @ UVT 60% and Peak Flow	36,724 uWs/cm ²							
Effluent Standards	77/100 ML E. coli							
UV Disinfection System								
Manufacturer / Model	Trojan System / UV3000 PTP							
Number of UV Banks	2 (1 duty, 1-stand-by)							
Number of UV Modules (Total)	20 (10/bank)							
Number of Lamps per Module	4							
Total Number of UV Lamps	80							
Liquid Depth	12.6″							
Width	30"							
Length	23"-2"							

Table 2-19 UV Disinfection Existing Design Information

Design Standards

- <u>Dosage Monitoring</u>: Each UV module should be equipped with a UV intensity meter responding only to light between 2,525 and 2,550 angstroms. The sensing device for this meter should be fixed at the area of minimum expected intensity. The sensor should be installed within a quartz sleeve. (TR-16)
- <u>Contact Period</u>: Sufficient contact time is required in a UV reactor to provide the established design dose at the delivered UV intensity under peak flow conditions.
- <u>Control Equipment:</u> Each UV module should activate a local and remote alarm signal when the UV intensity drops to 80 percent of original output. A spare PLC processor with a current program should be available.
- <u>UV Dose</u>: The system will provide a minimum UV dose of at least 30,000 microwatt-seconds per square centimeter at peak flow. (VT UV Disinfection Standard)
- <u>Open Channel Units</u>: For open channel units, at least two banks of lamps shall be provided, which operate in series. The multiple open channel units shall cumulatively provide at least the minimum required dosage at the facility's peak flow rate. (VT UV Disinfection Standard)

Assessment

The assessment of the major components for the disinfection system is summarized in Table 2-20 on the following page and the major needs are described as follows:

Findings:

• The UV intensity meter is not working, however a replacement has been ordered and will be installed in the near future.

• UV top of channel wall elevation is below the 100-year and 500-year flood elevations.

ltow	I	Ranl C	c of Exis ondition	ting า		Year	Projected	Nata
Item	Ро	or	Fair	Go	od	Installed	Remaining	Notes
	1	2	3	4	5	LI	Life (years)	
UV Disinfection System			х			2005	5-10	May need new controls.
Stainless Steel UV Channel				х		2005	20+	Top of channel is below the 100-yr flood elevation

Table 2-20 UV Disinfection System Assessment

2.6.9. Effluent Flow Measurement

Flow leaving the UV disinfection system flows from the UV outlet transition box through a 12-inch pipe into the effluent wet well. Effluent flow measurement is achieved by the use of a 90° vnotch weir and an ultrasonic level detector in the wet well. The 90° v-notch weir can accurately measure up to 1.616 MGD with 1.0 feet of head above the bottom of the v-notch according to Isco Open Channel Flow Measurement Handbook, 3rd Edition. Historical operating data suggested that the open channel Sigma 980 effluent flow meter seems to max out at 0.66 MGD, which would indicate a calibration issue or a malfunctioning flow meter. To solve this issue, the effluent flow meter was corrected and recalibrated. Additional effluent flow data collection beginning in October 2023 will help inform design criteria.

The TR-16 Standard is to accurately measure peak flows at the 25-year flood elevation and protect against the 100-year flood. As discussed in Section



Figure 2-20 Effluent Wet Well & V-Notch

1.4.2, the 100-year and 500-year flood elevations at the Richmond WWTF site are EL. 310.50 ft and EL. 313.77 ft, respectively. The invert of the v-notch weir is EL. 306.14 ft, which is below the updated 100-year flood elevation. A check valve is located on the effluent line to prevent water from hydraulically backing up the outfall during a 100-year or 500-year flood event. During the recent July 2023 flooding event, emergency use of a sump pump to lift flows to an external discharge point was used to prevent flooding of the Filter/UV Room. While the sump pump was able to keep up with flows during the most recent flooding event, it has not been properly sized to handle peak flows and provide redundancy.

A plant water system is not provided, and operators noted the WWTF uses significant potable water for screening and dewatering.

Effluent sampling is accomplished by an auto sampler located adjacent to the effluent channel that takes 24-hour composite samples from the effluent sump.

Table 2-21 Effluent Flow Measurement Existing Design Information

Item Description	Existing
Control Device	90° V-Notch Weir
Bottom of V-Notch Weir Elevation	306.13-ft
Measurement	Ultrasonic Flow Meter
Manufacturer/Model	Sigma 980
	Min: 0.03 MGD at 0.2' above v-notch
Flow Meter Capacity	Max: 1.616 MGD at 1' above v-notch
	Recorded Max: 0.66 MGD ¹

Notes:

1. Max effluent flow from historical operating data January 2018 to February 2023

Assessment

The assessment of the major components for the effluent flow measurement system is summarized in Table 2-22 on the following page, and the major needs are described as follows:

Findings:

- The effluent flow meter did not appear to be accurate based on recorded historical flow data. Since then, the meter has been corrected and recalibrated.
- The effluent weir elevation is at 306.14 feet which is below the 100-year flood elevation of 310.50 feet and will impact flow measurement accuracy during a flood event.
- The effluent wet well top of wall elevation is at 313.30 ft, which is below the 500-year flood elevation of 313.8 feet.
- Effluent check valve and sump pump downstream of weir is an emergency fix and not sized for peak flows or redundancy.
- No plant water system is provided.

ltom	Ranking of Existing Condition		Projected Year Remaining		Notos			
nem	Po	or	Fair	Fair Good		Installed	Life	Notes
	1	2	3	4	5		(years)	
90° V-Notch Weir				х		2005	20+	
Ultrasonic Flow	×					2005	0-2	Flow meter was recently
Meter	^					2005	02	corrected and recalibrated.
Emergency effluent sump		х				2007	2-5	Not sized for peak flows. No redundancy.
Effluent Sampler			v			2005	F 10	
Emuent Sampler			X			2005	5-10	

Table 2-22 Effluent Flow Measurement Assessment

2.6.10. Outfall

Disinfected effluent leaves the effluent wet well by means of a 12" pipe which flows to Manhole No. 13 prior to being discharged to the Winooski River through an 18" reinforced concrete outfall pipe. A check valve was added to the outfall after Tropical Storm Irene. This and the temporary provision of using a sump pump in the effluent well to discharge effluent flows to a higher elevation of the Winooski River prevented flooding of the lower level of the WWTF during the July 11, 2023 flood event.

The outfall was not observed during the site visit and therefore the condition could not be assessed.

Assessment

The assessment of the major components for the outfall is summarized in Table 2-23, and the major needs are described as follows:

Findings:

• Condition assessment of the existing outfall was not performed.

Table 2-23 Outfall Assessment

ltom	R	anki (ing of E Conditio	xisti on	ing	Year	Projected Remaining	Notes	
nem	Рс	or	Fair	G	bod	Installed	Life		
	1	2	3	4	5		(years)		
18" Outfall to Winooski River						1972	unknown	A condition assessment of the outfall was not performed.	

2.6.11. Septage Receiving Facilities

The Richmond WWTF has the ability to accept up to 2,000,000 gallons of septage per year. Septage receiving facilities consist of a septage receiving unit located on the upper level of the Processing Building with two (2) aerated holding basins below. The septage receiving unit consists of a cylindrical bar screen complete with screen basket, rotating rake, cleaning comp, screw conveyor, dewatering screw, screenings press with drive unit mounted in a stainless-steel tank, tank spray wash system, motorized inlet valve, and liquid level sensing system. Septage haulers manually record the volumes discharged. The Town has indicated that they would prefer to have a flow meter on the discharge to allow for more accurate record keeping.

Septage, along with WAS, discharges directly to either of the two aerated holding basins, each with a capacity of 23,000 gallons, located below the floor of the upper level of the Processing Building. Each tank is equipped with a diffused fine bubble aeration system and fed air



Figure 2-21 Septage Receiving Unit

from a 10 HP, 200 scfm positive displacement blower with a VFD located in the basement of the Process Building. Alternatively, the diffused aeration system can also be fed from the aeration tank blowers.

A single sludge transfer pump is provided to transfer sludge from either of the two tanks to the aerobic sludge holding tank or between the two tanks. The operators indicated that the pump runs well, however the model is obsolete, and it is hard to get replacement parts. The aerated holding basins were designed to be decanted to increase the solids concentration using manually operated decant valves and ports on each basin that drain to the influent sewer; however, the operators indicated that decant has never been used and has since been disconnected.

The inside of the holding tanks was not accessed and therefore not evaluated. During the time of the site visit, operators indicated that there is about 4 feet of solids accumulated in the tanks and they have been experiencing issues with the diffusers in the aerated holding basins. They indicated that as the tanks are considered a confined space, they will need to contract with an outside firm to do the cleaning. Since the visit, solids have been removed from both holding tanks.

The ventilation system within the septage receiving area in the upper Process Building is inoperable. Additionally, vents have been cut into the electrical room housing the motor control center (MCC) in the septage receiving garage, allowing heat from the electrical room to vent into the septage receiving area. The septage receiving area is classified as Class I, Division 1, hazardous space per NFPA 820. Equipment located in this space and shared air space are required to have explosion proof motors.

The design information for the existing septage receiving facilities is presented in Table 2-24 on the following page.

Description	Existing Design				
Septage Receiving Unit					
Manufacturer	Lakeside Equipment Corp.				
Hydraulic Capacity	400 gpm (up to 3% solids)				
Aerated Holding Basins					
Quantity	2				
Dimensions	21.58' x 14.58'				
SWD	10'				
Capacity	23,000 gal (each)				
Aerated Holding Basin Tank Blower					
Number of Blowers	1				
Location	Process Building Basement				
Blower Type	Positive Displacement				
Power Draw	10 HP				
Capacity	200 scfm @ 5 psig				
VFD	Yes				
Aerated Holding Basin Aeration System					
Diffusers	Fine Bubble, Membrane Type				
Manufacturer	Sanitaire Aeration System				
Sludge Transfer Pump #1					
Quantity	1				
Location	Process Building Basement				
Туре	Double Disc, Positive Displacement				
Power	7.5 HP				
Capacity	150 gpm @ 18 ft TDH				
VFD	Yes				

Table 2-24 Septage Receiving Facilities Existing Design Information

Design Standards

- Without pretreatment or wastewater process modifications, septage addition should not exceed 2-5 percent of actual wastewater flow on any day and must be slowly metered into the wastewater stream during periods of the day with higher flow. (TR-16)
- The receiving station area must collect and contain any septage spilled during unloading. Equipment and space for washdown must be provided, including water with ample pressure, hose, and spray nozzle. (TR-16)
- Receiving facilities should provide for the containment, collection, and treatment of odors. (TR-16)
- A sludge storage system should be equipped with mixing devices to prevent separation of solids and to provide a more uniform feed-to-dewatering device. Aeration may be required if the sludge is unstabilized. (TR-16)
- A minimum mixing and oxygen requirement of 15-20 cfm per 1,000 cf of tank volume is recommended for WAS with the largest blower out of service. If diffusers are used, the nonclog type is recommended, and they should permit continuity of service. If mechanical aerators are used, a minimum of 1.0 HP per 1,000 cf should be provided. (TR-16)

- Pumps for handling the septage should be non-clogging and capable of passing 3-inch diameter solids. (10 State Standards)
- Sludge withdrawal piping should have a minimum diameter of 8 inches for gravity withdrawal and 6 inches for pump suction and discharge lines. For dilute sludges, the available head should provide a velocity of at least 3 feet per second at the design flow. (TR-16)
- Class I Division 1 sludge storage wet wells, pits, and holding tank spaces require combustible gas detection system, portable fire extinguisher, and hydrant protection. (NFPA 80)

Assessment

The assessment of the major components for the septage receiving facilities is summarized in Table 2-25, and the major needs are described as follows:

Findings:

- The septage receiving unit is reaching the end of its anticipated useful life and was found to be in poor condition, the bearings are gone, and the auger has significant wear.
- Septage receiving unit must be manually cleaned out by operators.
- The Town is interested in a key card system with a flow meter for recording septage hauler discharges.
- There is significant solids accumulation in each aerated holding basin, causing clogging issues with the diffusers.
- Aerated holding basins are a confined space.
- The aerated holding basins decant system has been disconnected.
- Plug valves on septage holding tanks are not operable.
- There is no redundancy provided for the sludge transfer pump. The pump model is obsolete making parts hard to obtain.
- The ventilation system is not operable in the garage where the septage receiving unit is located. Vents have been cut into the electrical room providing airflow between the two spaces.
- The septage receiving room is not compliant with NFPA 820 hazardous classifications.

		k of l	Existing	g Cond	ition	Veer	Projected	
Item	Po	or	Fair	Go	od	rear	Remaining	Notes
	1	2	3	4	5	installeu	Life (years)	
Septage Receiving Unit			Х			2005	2-5	
Aerated Holding Basins				х		2005	20+	Significant solids accumulation
Septage Receiving Diffusers	х					2005	0-2	Diffusers are clogged by accumulated solids
Tank Decant	х							Decant has been disconnected.
Septage Receiving Blower			Х			2005	5-10	
Sludge Transfer Pump #1			х			2005	2-5	No redundancy provided. Model is obsolete.

Table 2-25 Septage Receiving Facility Assessment

2.6.12. Aerobic Sludge Holding Tank

Sludge from the two aerated holding basins is pumped by the sludge transfer pump to the aerobic sludge holding tank. The aerobic sludge holding tank can hold 34,000 gallons and is equipped with coarse bubble diffusers. Air to the diffuser system is supplied by a 10 HP, 170 scfm blower located in the one-room structure located over the aerobic sludge holding tank, which also houses the grit blower. The intake for the blower is internal to the building.

The aerobic sludge holding tank was not drained to evaluate the condition of the concrete tank, nor the existing diffusers. The operators noted that the tank needs to be cleaned as there is an accumulation of rocks, grit, and rags in the tank which are creating plugging issues with the downstream sludge transfer and feed pumps. The non-uniform bubble pattern observed on the surface of the tank contents during the evaluation indicates that some of the diffusers may be clogged and inoperable.



Figure 2-22 Aerobic Sludge Holding Tank

Description	Existing Design
Aerobic Sludge Holding Tank	
Dimensions	38' x 8.5'
SWD	14'
Capacity	34,000 gal (each)
Aerobic Sludge Holding Aeration System	
Diffusers	Coarse Bubble
Туре	unknown
Number	unknown
Air Required for Mixing	140 scfm
Blowers	
Number of Blowers	1
Manufacturer	Aerzen USA Corp.
Blower Type	Positive Displacement
Capacity	170 scfm at 7 psi
Motor	10 HP
VFD	Yes

Table 2-26 Aerobic Sludge Holding Tank Existing Information

Assessment

The assessment of the major components for the aerobic sludge holding tank is summarized in Table 2-27, and the major needs are described as follows:

Findings:

- The concrete tank structure was not able to be evaluated. It is recommended that a concrete assessment be conducted.
- Operators noted that the tank needs to be cleaned as there is an accumulation of rocks, grit, and rags.
- Observance of non-uniform buddle pattern at surface indicates that some diffusers in the aerobic sludge holding tank are not operational and need to be changed.

ltom	R	anki (ing of E Conditio	xisti on	ing	Year	Projected Remaining	Neter	
item	Poor		Fair	Fair Goo		Installed	Life	Notes	
	1	2	3	4	5		(years)		
Aerobic Sludge Holding Tank			х			1972	20+	Concrete assessment needed. Tank requires cleaning due to accumulation of rags/rocks/grit.	
Diffusers Coarse Bubble	х					1972	0-2	Some diffusers appear clogged. Need to be replaced.	
Blower			Х			2005	5-10		

Table 2-27 Aerated Sludge Holding Assessment

2.6.13. Dewatering Facilities

A separate Dewatering Building is located at the far side of the site and houses sludge pumping equipment, a sludge day tank, rotary press, polymer feed system, dewatered sludge cake conveyors, and a sludge trailer. A site visit by the manufacturer of the dewatering equipment, Fournier, was made on September 30, 2023, and the summary report of the technician's findings is presented in Appendix B.

When the dewatering equipment is in operation, sludge is removed from the aerobic sludge holding tank using the sludge transfer pump located in the Dewatering Building which originally discharged through a sludge grinder to the sludge day tank, also located in the Dewatering Building. The sludge grinder has since been removed by the operators.



Figure 2-23 Sludge Day Tank

The sludge day tank is a 3,000-gallon fiberglass tank which

supplies sludge to the dewatering process. The tank has an access hatch at the top and is equipped with a mixer, to keep the contents mixed, and a level detector that is integrated into the controls of the sludge transfer pump and the sludge feed pump. The operators have noted that the sludge day tank needs to be cleaned out as the sludge feed pump routinely gets plugged from rags passing through. The

operators also noted that there is no way to remove the mixer from the day tank as there is no headroom above the tank for the long shaft.



Figure 2-24 Rotary Press

The sludge feed pump pumps sludge directly to the dewatering equipment, specifically the flocculator, where sludge is mixed with polymer from the polymer feed system. This pump is directly controlled by the rotary press PLC. The inspection report noted that sludge gravity flows through the pump from the day tank, either indicating that new pump interior parts are needed, or sludge valves need replacing.

A two-channel rotary press, located in the Sludge Dewatering Building, provides sludge dewatering using rotating perforated screens. Sludge is fed from the sludge day tank into the flocculator where polymer is injected into the sludge via a polymer feed

pump. Conditioned sludge then enters a manifold and is divided into a series of circular channels. In the channel, sludge is sandwiched between two (2) low speed rotating perforated screens. The operators noted that maintenance on the rotary press has lapsed, resulting in operational issues. It was noted that regular cleaning is necessary every six months at a minimum. Currently, only one (1) channel in the two (2) channel rotary press is operational. The manufacturer's report indicates that the one inoperable channel has significant damage to it's screen, however all four screens are very warn.

The polymer feed system is located in the Sludge Dewatering Building and polymer injects to the sludge dewatering flocculator. The polymer feed system consists of a package system that provides for the mixing of polymer with water in a mixing chamber and then uses a chemical metering pump to transfer dilute polymer to a batch tank, where the polymer can age. A progressive cavity pump equipped with a VFD then feeds the dilute polymer to the Rotary Press flocculator. The polymer feed system is automatically controlled by the rotary press control panel.



Figure 2-25 Rotary Press Conveyor System

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A conveyor system is used to transfer dewatered sludge cake from the rotary press to a roll-off



Figure 2-26 Inclined Conveyor

During the site visit, it was noted that the Dewatering Building does not have proper ventilation. The operators have noted there are significant odors issues and a build-up of ammonia gases, especially in the winter season when garage doors are kept closed. At the time of the site visit, operators noted that the only control for the overhead door is inside the building and across the room from the main entrance. Since then, a switch has been installed on the exterior of the building to allow operators the ability to open the overhead door and vent gases without having to enter the building. A fair amount of corrosion of metal components in the Dewatering Building was also observed during the site visit.

Additionally, the floor of the dewatering building does not have a rear floor drain, causing difficulties managing drainage from the sludge trailer. Operators expressed interest in a water meter on the water line to

container where it is taken off-site for final disposal. The conveyor consists of three parts: a discharge conveyor, the inclined conveyor, and the distributing conveyor. All three are hollow flight screw type conveyors. The discharge conveyor runs horizontally for 9 feet from the discharge chutes of both channels of the rotary press to the inclined conveyor. The inclined conveyor is 20 feet in length with an approximate slope of 37 degrees. This conveyor brings dewatered cake up to the middle of the distributing conveyor. The distributing conveyor is approximately 32 feet in length and is elevated over the parked roll-off container. The distributing conveyor has openings at either end, and two pneumatically operated slide gates to distribute dried cake evenly within the container. The hollow flight conveyor within the distributing conveyor has a reversing motor to allow cake to be discharged to either end. The operators noted that both the auger and screw were replaced in the Spring of 2022, and that the screw broke again in December of 2022. During the repair, staff noted that the liner needed to be replaced.



Figure 2-27 Dewatering Building

the dewatering building to monitor the cost of potable water used during dewatering, as well as a dedicated electrical meter for the building to assess power consumption.

Description	Existing Design
Sludge Transfer Pump (From Digester to Day Tan	k)
Quantity	1
Туре	Double Disc, Positive Displacement
Manufacturer	Penn Valley Pump Co. Inc.
Capacity	50 gpm
Max TDH	25.5 ft
Min TDH	11.5 ft
Max Suction Lift	19 ft
Min Suction Lift	5 ft
Motor	5 HP
VED	Yes
Sludge Day Tank	100
Quantity	1
	3,000 gal
	Fiberglass Tank
Mixer	Shaft impeller
Sludge Feed Pump (From Day Tank to Rotary Pre	ss)
Quantity	1
Туре	Double Disc. Positive Displacement
Manufacturer	Penn Valley Pump Co. Inc.
Flow Rate	0-60 gpm
Max TDH	20 ft
Min TDH	0 ft
Motor	3 HP
VFD	Yes
Control	Botary Press PLC
Sludge Dewatering	
Solids Feed ¹	Volume Pressed: 26 445 gpd
Solids Concentration ²	WAS: 9 300 mg TSS/I
	Septage: 1 760 mg TSS/I
Average Solids Feed % ³	0.26%
Sludge Disposal	98 wet tons/month ¹
	$28.6 \text{dry tons/month}^3$
Manufacturer/Model	Eournier Industries Inc. 2-900/2000CV
Type	Rotary Press with Elocculator
Number of Units	1
Channels	2
Channel Diameter	26″
Motor	7 5 HD
Through-put ⁴	100 lbs/br/channel
Dewatered Sludge Cake Average Solids %5	200 105/11/chamiei
Dewatered Sludge Cake Average Solids %	29.2%

Table 2-28 Sludge Dewatering Existing Design Information

Description	Existing Design		
Flow Meter			
Type/Manufacturer	Magnetic/Endress and Hauser		
Size	3"		
Conveyor System			
Туре	Hollow flight screw conveyor		
Discharge Conveyor Length	9'		
Inclined Conveyor Length	20' @ 37 degrees		
Distributing Conveyor Length	32'		
Slide Gates	Pneumatically operated		
Polymer Feed System			
Тура	Liquid polymer activation, dilution, and feed		
Туре	system including mixing chamber and feed pump		
Storage	55-gallon drums		
Chemical Feed Pump Type	Positive Displacement, Diaphragm Type		
Number of Pumps	1		
Metering Pump Flow Range	0.4 – 8.0 gph		
Dilution Water Flow Range	120 to 1,200 gph		
Application Points	Sludge Dewatering Flocculator		

Notes

- 1. Average is from historical operating data from January 2018-February 2023.
- 2. Concentration is from 9/21/2022 sampling event.
- 3. Calculated from historical operating data and sampling event.
- 4. From Rotary Press Basis of Design
- 5. Cake solids is average of five dewatered sludge testing results from 2022.

Design Standards

- The operating period should not usually exceed 30 hours per week, which allows one-shift operation with time for chemical makeup, cleanup, and delays. (TR-16)
- Hydraulic loading rate of a single drive unit ranges from 5 to 250 gpm, with a maximum loading rate of 50 gpm per channel. (TR-16)
- Facilities should be provided for ventilation of the dewatering area. (TR-16)
- Floors should be pitched and drained for cleaning purposes and should be slip-proof. (TR-16)
- Volume of containment area be no less than 125% of the tank volume for hazardous or corrosive chemicals. (TR-16)

Assessment

The assessment of the major components of the dewatering system are summarized in Table 2-29, and the major needs are described as follows:

Findings:

• Significant wear and damage to the screens of the dewatering press screens was observed by the manufacturer's technician during the September 30th site visit. Significant repairs are warranted to keep dewatering operations functional.

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- The grinder has been removed from the sludge transfer line.
- No ventilation is provided in the sludge dewatering room resulting in accumulation of corrosive gases and odors within the building.
- No gas detection system in building.
- Day tank needs to be cleaned.
- Mixer in the sludge day tank has a long shaft and there is no way to remove.
- Lack of floor drain at rear of sludge trailer makes it difficult to manage drainage from trailer.
- Polymer system defaults to over-pumping when power is lost.
- Operators noted that the hollow flight augers of the conveyors are deteriorated and need replacement, as well as the liner.
- A check valve on the line between the aerobic sludge holding tank and day tank was added and is cleaned daily.
- The feed pump to the dewatering press plugs frequently.

ltom	Rank of Existing Condition Year Projecte		Projected	Notos					
item	Ро	or	Fair	Go	od	Installed	Life (years)	Notes	
	1	2	3	4	5		Life (years)		
Sludge									
Transfer			х			2005	2-5		
Pump									
Sludge Day			v			2005 2.5		Needs cleaning. No way to get	
Tank			X			2005 2-5		mixer shaft out of tank.	
Sludge		v				2005	0.2	Pluge frequently	
Feed Pump		X				2005	0-2	Plugs frequentiy.	
Rotary	~					2005	0.2	Maintenance and repairs required	
Press	х					2005	0-2	to remain operational	
Flocculator	х					2005	0-2	Leaking at seal shaft	
Flow Meter			х			2005	2-5		
Conveyor						2005 0.2		Screw auger is shot. Liner has been	
System	х					2005	0-2	replaced several times	
Dewatering						2005	2 5	Severe corrosion inside building	
Building		Х				2005	2-5	due to build-up of corrosive gases	

Table 2-29 Dewatering Facilities Assessment

2.6.14. Operations Building

The Operations Building was built in 1972 and has undergone modifications and renovations in the 2005 upgrade. The upper level houses the office and laboratory, bathroom, and break room, while the lower level houses the RAS and WAS pump systems, influent pumping, and mechanical room. The roof drains of the Operations Building are connected directly to the influent wet well. Currently, there is no access hatch to reach the influent pumps located underneath the laboratory, and the operators expressed a need for better access.

The operators indicated that the control on the boiler have all been replaced and they are continuously having to repair it. They also noted that HVAC systems throughout the facility need to be completely replaced.

As part of the 2005 upgrade, an extension to the existing Operations Building was constructed to include the garage that houses septage receiving, workshop space, and an electrical room containing the motor control center (MCC).

Observations noted during the May 2023 site visit include the following:

- Boiler needs to be replaced.
- HVAC systems need to be replaced
- Laboratory refrigerator is inoperable.
- Operations Building roof drains to the influent wetwell.
- Doors and windows are original to plant.
- No method to hoist influent pumps from basement.
- Washer/dryer discharges to the floor drain and vents to the building interior.

2.6.15. Site

As previously mentioned in Section 1.4.2, the new flood elevation determination by FEMA in 2014 resulted in a 2 foot increase in the 100-year flood elevation. The new **100-year** and **500-year** flood elevations at the location of the Richmond WWTF are **310.50 feet** and **313.77 feet**, respectively.

The ground elevation around the Process Building, which houses the chemical storage, UV disinfection, and filter units, is **313.50 feet**. While the Process Building will be protected from a 100-year flood event, there is risk of flooding during a 500-year flood event.

The ground elevation of the Operations Building is at approximately **314.00 feet** and is protected from the 100-year and 500-year flood events.

Other site observations made during the May 2023 site visit include the following:

Stand-by Generator:

There is a 150-kW diesel engine driven generator, rated 150 kW at a governed speed of 1,800 rpm providing 480 volt, 3-phase stand-by electrical service located outside of the Blower Building. The generator has an approximately 390-gallon skid-mounted No. 2 diesel fuel tank capable of providing 24 hours of operation. When a loss of utility power occurs, the amount of load placed on the stand-by generator is limited to loads deemed critical for maintaining operation, including: one (1) influent pump, screening & grit removal, anoxic selector submersible mixers, one (1) aeration tank blower, secondary clarifiers, RAS pinch valve, RAS flow meter, two (2) filter units, UV disinfection system, influent & effluent flow metering, automatic samplers, SCADA system & PLC, heating, and lighting.

While the operators indicated that the generator operates well, it is noted that there is a need for an electrical assessment for the Richmond WWTF as one does not currently exist.

Plant Water System:

The Richmond WWTF does not have a plant water system and Town water is used throughout the site. The operators have expressed a desire to implement a plant water system to reduce the Town water consumption and cost.

Former Sludge Drying Bed: The former sludge drying bed has been converted to a storage area. There is no electricity to this area and the operators indicated wanting to supply power and heat to the garage to allow for other opportunities for use.

<u>Site Fence</u>: The facility is surrounded by a security fence with access gates at the driveway entrance and behind the Storage Garage. Feedback from the trucking company that delivers the sludge trailer indicates the need for a wider gate opening at the main entrance.

Assessment

The assessment of the major components is summarized in Table 2-30 for the WWTF site.

Itom	R	anki C	ng of E Conditio	xistir on	ng	Year	Projected Remaining	Netes	
nem	Ро	or	Fair	Good		Installed	Life	Notes	
	1	2	3	4	5		(years)		
Process Building					х	2005	20+	Finished floor at 313.50 is below the 500-yr flood elevation	
Generator				х		2005	10-15	An electrical assessment of the generator is recommended.	
Storage Garage	x					?	0-2	Installation year unknown – between 1972 and 2005. No electricity.	
Security Fence & Entrance Gate			х			2005	5-10	Wider main entrance gate needed for sludge trailer	
Yard Hydrants & yard piping			х			1972/2005	2-5	Addition yard piping added in 2005	

Table 2-30 Site Assessment

2.6.16. WWTF Electrical System and Instrumentation

General

A detailed electrical and instrumentation review by an electrical engineer was not included in the scope of work. General observations are provided in the following section.

Applicable Codes and Standards

The electrical systems design for the refurbishment of the wastewater treatment facility must meet applicable State of Vermont and Fire, Electrical and Energy codes. The electrical systems design for the planned upgrades at the WWTF will consider the following codes and standards:

- Vermont Fire and Building Safety Code (2015)
- The National Electrical Code (NFPA 70) (2020)
- The National Fire Alarm and Signaling Code (NFPA 72) (2013)
- Vermont Access Rules (2012)
- Americans with Disabilities Act Accessibility Guidelines (ADAAG), July 26, 1991
- Vermont Commercial Building Energy Standards (CBES) (2020)
- NFPA 1 (2015), Fire Code
- NFPA 101 (2015), Life Safety Code
- IBC (2015), International Building Code
- NFPA 37 (2010), Standard for the Installation and Use of Stationary Combustion Engines and Gas Turbines
- NFPA 110 (2013), Standard for Emergency and Standby Power Systems
- NFPA 820 (2012), Standard for Fire Protection in Wastewater Treatment and Collection Facilities
- Technical Report #16 (TR-16) Guides for the Design of Wastewater Treatment Works prepared by the New England Interstate Water Pollution Control Commission.

2.6.17. Existing Conditions

Motor Controllers

The motor control center (MCC) is located in the Operations Building Garage. Outside ventilation has been dismantled and vents have been cut into the room connecting the air space with the Operations Garage that is classified as a Class I, Division I hazardous space as it contains the septage receiving unit.

PLC/SCADA System/Alarm Communications

The central plant PLC/SCADA system was installed in the 2000 upgrade.

LCS recently repaired the dialer from the alarm system to have the alarm send texts to staff cell phones until the alarm is cleared in SCADA. The Town intends to have LCS set up to alarm system to call the pager when there is an alarm condition and the cell service is not working.

2.7 Condition of Collection System

As part of the 20-Year Evaluation, a visual inspection of the Town of Richmond's sewer collection system was conducted, and conditions were documented.

Description

Description of Richmond's sewer collection system:

- 21,880 linear feet of sewer mains
- The system features 125 sewer manholes:
 - Two (2) manholes were not able to be opened
 - Of the 123 manholes inspected:
 - 118 are 48" precast concrete manhole
 - 3 are 60" precast concrete manhole
 - 1 is a precast concrete rectangular vault measuring approximately 106" x 80"
 - 1 is a 25" diameter circular brick manhole

Assessment

The assessment of each manhole was documented on individual manhole inspection reports, which are presented in Appendix E. A summary of the findings include the following:

- 121 manholes were found to be in good structural condition
 - 2 manholes were found to be in fair structural condition
 - Both manholes had the covers stuck in the ring which was broken from the frame
 - ES 01
 - TD 06
- 121 manholes were found to be in good operational condition
 - 2 manholes were found to be in fair or poor conditions
 - Both manholes had sewage and obstructions in the channel
 - NM 09
 - RR 02
- 15 manholes were identified for cleaning
 - o BK 01

- o BR 02
- o BR 06
- o CH 01
- CH 05
- ML 01
- o NM 05
- o NM 08
- o RR 02
- o SM 01
- o TD 06
- o TD 08
- o TH 06
- o VG 01
- WW 07
- 6 manholes were identified for maintenance:

Richmond Wastewater Treatment Facility 20-Year Evaluation Section 2 – Existing Facilities

- ES 01 Ring and frame replacement
- NM 04 Replace bricks in riser
- NM 05 Replace bricks in riser
- PL 02 Ring and frame replacement
- TD 06 Ring and frame replacement
- VG 02 Reset frame and cover

2.8 Condition of Pump Station

Bridge Street Pump Station

The Town of Richmond has only one (1) pump station located within the collection system. The pump station is located approximately 350 feet south of the Bridge Street bridge. The pump station collects sewage by gravity from served homes south of the Bridge Street bridge. The pump station pumps via a force main to the next manhole in the collection system, north of the bridge located at the south side of the Volunteers Green parking lot. The gravity sewer is then directed along the Volunteers Green access road then to Esplanade Street where it enters the Wastewater Treatment Facility. The Bridge Street Pump Station is located below the 100-year flood elevation and experiences flooding during wet weather events.

Design information for the existing Bridge Street Pump Station is presented in Table 2-31 on the following page.

Description	Description
Pump Station	
Hatch Dimensions	20" tall x 45" in diameter
Pump #1	
Туре	Unknown
Motor	Unknown
Motor Run Time (as of 07/07/2023)	13.8 Hours
Capacity	Unknown
Pump #2	
Туре	Unknown
Motor	Unknown
Motor Run Time (as of 07/07/2023)	51.1 Hours
Capacity	Unknown
Valves	
Quantity	2
Туре	Gate Valve
Communication System	
Manufacturer	Emerson
Model Number	HSP-121BT1RU
Model Name	Islatrol HSP
Voltage/Amps	120V/15Amp
Control Panel	
Manufacturer	Pratt & Smith, Electrical Contractors, Inc.
Manufacture Date	10/25/2011
Electrical Supply	208/120V 3 Phase 60 Hz
Input Overcurrent Protection	100 Amps
Input Full Load Amps	85.5 Amps
Largest Motor Full Load Amps	30.8 Amps
Field Wiring Diagram Number	BSP102511
SCCR	5kA RMS Symmetrical, 280V Max

Table 2-31 – Bridge Street Pump Station Inventory

Assessment

The assessment of the major components for the pump station is summarized in Table 2-32. As the pump station is considered a confined space, an assessment was performed from surface. The summary is as follows:

Findings:

- All systems seemed to be in good working order.
- The pump station is well beyond its estimated useful life as a structure and should be considered for replacement.
- The pump station force main that hangs underneath the bridge was damaged during the December 18, 2023 flooding event. The Town has since repaired it, however, would like to relocate the force main higher up underneath the bridge to prevent future damage during flooding events.

ltom		Rai	nk of Ex Condit	kisting ion	;	Year	Projected	Notos
item	Poor		Fair	Go	od	Installed	Life (vears)	NOLES
	1	2	3	4	5		Life (years)	
Wet Well			Х			1960	0	
Ladder			Х			1960	0	
Light			Х				0	
Pump #1			Х				2-5	
Pump #2			Х				2-5	
Communication				v			2 5	
System				^			2-5	
Control Panel					Х	2011	5-10	

Table 2-32 – Bridge Street Influent Wet Well and Pump Station Assessment

2.9 Financial Status of Any Existing Facilities

2.9.1. Wastewater Revenue

The Town of Richmond's Water and Wastewater Budget pays for wastewater treatment, pump station and force main conveyance operation, maintenance, and capital costs within the Town. A detailed Water and Wastewater Budget for Fiscal Year (FY) 2024 is presented in Appendix C. Table 2-33 summarizes budgeted wastewater revenue generated to support existing municipal wastewater facilities.

 Table 2-33 Richmond Wastewater Revenue

Description	FY 22 Budget	FY 22 Actual	FY 23 Budget	FY 24 Budget							
Wastewater Revenue											
Sewer User Receipts	\$357,337	\$373,213	\$361,326	\$292,874							
Hook-Up Fees - Sewer	\$1,000	\$3,213	\$1,000	\$1,000							
Net Interest on Checking Account	\$1,200	\$1,998	\$1,500	\$14,000							
Fund Balance Usage			\$48,394								
Septage Receipts	\$430,000	\$483,577	\$460,000	\$550,000							
Wastewater Revenue Subtotals	\$789,537	\$862,001	\$872,220	\$857,874							

2.9.2. User Rate Structure

Richmond's sewer user rates are summarized in Table 2-34.

Sewer User Type	User Rate		
Commencial	\$315.00/Annual Fee		
Commercial	\$13.85/1,000 gal treated		
Desidential	\$144.36/Annual Fee		
Residential	\$14.92/1,000 gal treated		

 Table 2-34 Richmond Wastewater User Rate Structure

2.9.3. Wastewater Expenses

Table 2-35 summarizes budgeted wastewater expenses for Richmond's municipal wastewater facilities.

Description	FY 22 Budget	FY 22 Actual	FY 23 Budget	FY 24 Budget
Wastewater Expenses				
Wastewater Administration Expenses	\$251,622	\$264,477	\$305,046	\$344,635
Wastewater Operating Expenses	\$329,900	\$393,699	\$359,900	\$416,800
Wastewater Capital Expenses	\$208,015	\$207,955	\$207,274	\$96,439
Wastewater Expenses Subtotals	\$789,537	\$866,131	\$872,220	\$857,874

 Table 2-35 Richmond Wastewater Expenses

2.10 Water/Energy/Waste Audits

There are no water, energy, and waste audits for the Richmond WWTF.

3. NEED FOR PROJECT

3.1 Health, Sanitation and Security

The reliable function of the wastewater treatment system is required to protect public health and sanitation by meeting the requirements of the Richmond WWTF NPDES discharge permit. The Richmond WWTF has effectively met its permit requirements over the past 5 years and has not had any exceedances during this time.

3.1.1. Public Health

The Richmond WWTF discharges effluent to the Winooski River which feeds into Lake Champlain. Multiple locations downstream of the location where the Richmond WWTF discharges are used as swimming locations by the community.

3.1.2. Water Quality

The Winooski River discharges to the Main Lake section of Lake Champlain which is impaired by phosphorus. A total maximum daily load (TMDL) was implemented for phosphorus for wastewater treatment facilities whose effluent reaches Lake Champlain. The TMDL wasteload allocation for the Richmond WWTF is .061 mt/yr. While Richmond WWTF effectively meets its permit requirements, a reliable wastewater treatment system is required to continue meeting the TMDL allocation as well as the NPDES permit requirements.

The Richmond WWTF NPDES permit is provided in Section 2.3. The recommended project will ensure that Richmond can continue meeting their existing discharge permit.

3.2 Aging Infrastructure

Age related needs were identified in the assessments completed in Section 2.5.8 for the Richmond WWTF. Some items requiring upgrade are original (1972) to the plant while others were implemented during the 2005 upgrade. Much of the equipment at the facility has reached the end of its useful life, and replacements are recommended to maintain reliable operation and treatment functions.

In addition, the Richmond WWTF serves the larger community through septage acceptance. The WWTF treats septage directly through its solid stream by mixing septage together with its own waste sludge and dewatering it using a rotary press. The WWTF would like to receive 50,000 gallons of septage per day (gpd), however due to aging septage receiving equipment and downstream unit treatment processes, i.e. dewatering equipment, septage holding aeration equipment, transfer pumps, etc., the WWTF has had to scale back the amount of septage they can receive to 25,000 gpd. The reliability of the downstream treatment equipment is vital to Richmond's ability to be a regional septage acceptance facility.

Table 3-1 summarizes the needs for the WWTF.

	Projected Date of Required Upgrade				
Item Description	<2 Voars	2 to 5	6 to 10	11 to 15	20+ Years
		Years	Years	Years	
Influent Pump #1	\checkmark				
Influent Pump #2	\checkmark				
Grit Removal		\checkmark			
Anoxic Tank Mixers & VFD's		\checkmark			
Aeration Tank Diffusers & Blowers			\checkmark		
Clarifier Drives, Internal Mechanisms,				×	
Launders, & Weirs, & Walkway/Railings				·	
Clarifier Tankage					\checkmark
WAS Pump		\checkmark			
RAS Flow Meter		\checkmark			
Filtration Backwash Pump, Motor Drives,		\checkmark			
& Backwash Valves		·			
Filtration Sludge Pumps & Filter Cloth	\checkmark				
Filter Tanks			\checkmark		
UV Disinfection System			\checkmark		
Stainless Steel UV Channel					\checkmark
Septage Receiving Unit		\checkmark			
Aerobic Sludge Holding Tank					\checkmark
Aerobic Sludge Diffusers	\checkmark				
Aerobic Sludge Blower		\checkmark			
Sludge Day Tank, Transfer Pump, Feed					
Pump & Flow Meter		Ŷ			
Rotary Press & Flocculator	\checkmark				
Conveyor System	\checkmark				
Dewatering Building		\checkmark			

Table 3-1 Summary of Major Deficiencies

In addition to the summary of the major deficiencies at the Richmond WWTF, an emphasize is placed on the following immediate needs:

- Influent Pumps: the influent pumps are at risk of imminent failure and immediate replacement is recommended.
- Septage receiving equipment is at the end of its life and not able to meet the demands of the volume of needed septage acceptance.

3.3 Codes & Standards

Specific components of the Richmond WWTF do not meet current codes and standards. In order to bring the plant up to modern code and standards, replacement of existing equipment is recommended. Table 3-2 displays specific areas of the existing WWTF that do not meet current codes and standards.

Process Area	Standard	Status	
Influent Pumps	Peak Hour Flow with Largest Pump Out of Service (TR-16)	Does NOT meet standard	
Biological Process - Blowers	Blowers sized for the maximum daily oxygen requirements (TR-16)	Does NOT meet standard	
Chemical Secondary Containment	Secondary containment of 125% of chemical storage volume provided (TR-16)	Does NOT meet standard	
Chemical Feed	Minimum of two chemical feed pumps, 1 duty & 1 stand-by	Does NOT meet standard	
Ultraviolet (UV) Disinfection	Provide disinfection for peak flow with one bank out of service (TR-16)	Does NOT meet standard	

Table 3-2. Existing Equipment Codes & Standards Violations

In addition to the wastewater treatment standards that are not met throughout the facility, multiple buildings on-site lack proper ventilation and are not up to codes for their building classification including the Headworks Building, the Septage Receiving Area, and the Dewatering Building. Upgrades are required to ensure the WWTF meets modern code and standards.

3.4 Environmental Resiliency & Sustainability

The Richmond WWTF is not located in a regulatory flood boundary, however, is within a Special Flood Hazard Area as shown on the FEMA FIRMette Map in Appendix A-3. The site is located on a raised island out of the 100-year floodplain; however, the lower level of the Process Building is not protected from a 500-year storm and is at risk of flooding. Certain process components are below the 500-year flood elevation including the top of the filter tank wall and the top of the effluent wet well. The top of the UV channel is below the 100-year flood elevation as well. The Bridge Street Pump Station is located below the 100-year flood elevation and routinely experiences flooding. The proposed project will mitigate these flooding impacts and provide the Richmond WWTF with resilient and sustainable solutions to combat environmental impacts.

3.5 Reasonable Growth

3.5.1. Current Residential & Non-Residential Flows

The Town of Richmond has a population of 4,167 as of 2020. Approximately 390 residential units are currently connected to the system. The West Main Street Sewer Extension Preliminary Engineering Report performed by Green Mountain Engineering in July, 2021 stated that the average annual residential flows as reported by the Town of Richmond are 32,000 gal/year (88 gpd) per living unit. Given the 390 residentials units, the total annual residential flow is approximately 12.5 MG/year. In 2022, the total yearly flow was 30.9 MG. Assuming that 12.5 MG is associated with residential flow, 18.4 MG is associated with non-residential flow.

3.5.2. Existing and Future Wastewater Flows

A proposed sewer service expansion area exists along the east side of West Main Street in Richmond, VT from Richmond Village to the I-89 interchange. Wastewater in the area is currently treated by individual on-site systems which is limiting growth potential. The expansion project is split up into three (3) phases, where Phase 1 and Phase 2 involve the expansion of the sewer collection system along the east side of West Main Street. Phases 1 and 2 are estimated to have a total future average daily wastewater flow of 8,420 gpd.

Phase 3 of the expansion project includes extending the sewer collection system to an existing mobile home park and commercial fuel company. Phase 3 of the future expansion area is considered separately from Phases 1 & 2 as implementation of Phase 3 is unknown at this time. Table 3-3 below depicts the future average daily wastewater flows associated with each phase of the sewer expansion area.

	Future Average Daily		
Expansion Phase	Wastewater Flow ¹		
	(gpd)		
Phase 1	2,945		
Phase 2	5,475		
Phases 1 & 2 Total	8,420		
Phase 3	36,420		
Phases 1, 2, & 3 Total	44,840		

Table 3-3 Future Sewer Expansion Area Wastewater Flows

Notes:

1. July 2021 West Main Sewer Extension Preliminary Engineering Report

In addition to the proposed expansion areas, the current unconnected committed allocations are 3,530 gallons per day between the residences and commercial users listed in Table 3-4.

Area	Unconnected Committed Sewer Allocated Flows ¹ (gpd)
Peaceable Kingdom (residential)	1,680
Whistle Stop Lane (residential)	680
112 E. Main Street (residential)	210
DS0022 (residential & commercial)	960
Total	3,530

Table 3-4 Unconnected Committed Sewer Allocated Flows

Notes:

1. July 2021 West Main Sewer Extension Preliminary Engineering Report

Combining the unconnected committed sewer allocated flows with the proposed expansion area Phase 1-3 flows, the total flow of proposed and committed unconnected sewer allocated flows is 48,370 gpd.

Assuming that the current unconnected committed sewer allocated flows and the total proposed expansion area are implemented during the next 20-years, Table 3-5 on the following page presents the future wastewater treatment capacity at the Richmond WWTF.

Description	Wastewater Flow (gpd)
WWTF Permitted Design Capacity	222,000
80% of WWTF Permitted Design Capacity	177,600
Historical Average Daily Flow ¹	73,000
Unconnected Committed Sewer Flows	3,530
Proposed Future Expansion Area Flows (Phases 1-3)	44,840
Total Remaining WWTF Treatment Capacity ²	56,230

Table 3-5 Future Wastewater Treatment Flow Capacity

Notes:

1. Historical Average Daily Flow from January 2018 – February 2023

2. Assuming proposed future expansion area flows are implemented

As discussed in Section 1.5, average daily flow entering the Richmond WWTF is not anticipated to exceed the design flow of 0.222 MGD over the 20-year planning horizon.

3.5.3. Regional Benefits

The Richmond WWTF serves as a regional septage receiving facility. The facility has recently purchased 2 new channels for their existing rotary press due to the existing equipment being at the end-of-usefullife. While this purchase will allow Richmond to continue offering septage acceptance, additional upgrades are required to allow Richmond to continue offering septage receiving for equipment that is at the end of useful life including, but not limited to, the existing septage receiving unit, sludge transfer pumps, sludge storage mixing systems, sludge dewatering feed pumps, dewatering equipment, and unsafe conditions in the existing Dewatering Building due to lack of sufficient ventilation. In order to allow Richmond to continue operating as a regional septage receiving facility, equipment replacement and upgrades are needed.

3.6 Design Criteria

3.6.1. Influent

The original influent design criteria, current influent conditions, and proposed influent design criteria for the liquid treatment processes at the Richmond WWTF are presented in Table 3-5 on the following page. Historical operating data is discussed in Section 2.5.

In order to determine the peak hour flow and current peaking factor, additional data collection is required due to the historical inaccuracy of the effluent flow meter. This peaking factor will be used to determine the future peak hour flow throughout the wastewater treatment facility. Since this data collection issue was identified, the effluent flow meter has been corrected and recalibrated. Additional data collection beginning in October 2023 through the spring of 2024 will help inform decisions
surrounding pending peak hourly flow. During the December 18, 2023, flooding event, the WWTF experienced a peak instantaneous flow through the plant of 2.3 MGD. While this peak flow event was due to flood waters entering into the collection system, it is not representative of a peak hourly flow. For the time being, the historical peak hourly flow of 1.152 MGD will be used until further data can be collected and the recorded peak instantaneous flow of 2.3 MGD will be used to size pumps to convey flow through the plant.

A biological model was created to analyze the current loadings seen at the WWTF and to determine the treatment capacity of the existing facility at permitted flows to define proposed design criteria. See Appendix D for a summary of the modeling results and calculations. The following influent design criteria represents the limit of nutrient loading capacity that the Richmond WWTF can accommodate without significant biological treatment process upgrade, i.e. building additional tankage or a switch to an MBBR process such as IFAS, or the addition of a side stream treatment process to collect and treat dewatering pressate that is returned to the liquid stream for treatment. As the Richmond WWTF is operating at 33% of its hydraulic capacity and the Town would like to continue to be a regional septage receiving facility, the recommendation is to continue with the current biological treatment process and proposed influent design criteria presented below in Table 3-6. As the WWTF hydraulic flows increase over the years, it will become important to monitor influent loadings and may be necessary to either reduce the amount of septage received or to implement a side stream treatment process to limit nutrient loading to the biological process.

Parameter	Original Design ^{1,2}	Current Conditions ^{3,4}	Proposed Design Criteria
Average Daily Flow	0.222 MGD	0.073	0.222 MGD
Peak Hour Flow	1.152 MGD	-	1.152 MGD
Peaking Instantaneous	-	2.3 MGD ⁵	2.3 MGD ⁵
Biochemical Oxygen	324 mg/L	670 mg/L	324 mg/L
Demand	600 lbs/day	411 lbs/day	600 lbs/day ⁶
Total Suspended Solids	270 mg/L	932 mg/L	421 mg/L
Total Suspended Solids	500 lbs/day	573 lbs/day	780 lbs/day ⁶
Total Phosphorus	10 mg/L	19.5 mg/L	19.5 mg/L
Total Nitrogen	-	-	-
Temperature (min/avg/max)	10/_/20°C	4/15/26°C	4/15/26°C

Table 3-6	Richmond	WWTF	Proposed	Influent	Design	Criteria

Notes:

- 1. Source: Basis of Design, 2003
- 2. Original design criteria BOD and TSS concentration are back calculated using design loads and design ADF. Original design criteria TP load is back calculated using design concentration and design ADF.
- Based on Monthly Operating Report data from January 2018 to February 2023. Influent sampling is taken at the influent channel of the wet well, upstream of RAS and pressate side streams. Samples can, at times, include return activated sludge (RAS) & pressate, when the wet well is used for flow equalization and an isolated influent sample is not possible.
- 4. Historical BOD and TSS loads are back calculated using historical average flows and

concentrations.

- 5. Peak instantaneous flow defined as the peak instantaneous flow experienced during the 12/18/2023 rain event.
- 6. Proposed design BOD and TSS loads determined from a biological model at average daily flow.

3.6.2. Effluent

Effluent design criteria for the Richmond WWTF are based on the existing NPDES permit and are provided in Table 3-7 on the following page. The existing NPDES permit expires on December 31, 2025.

Table 3-7	' Richmond	WWTF	Upgrade	Effluent	Design	Criteria

Parameter	Original Design Criteria ¹	Proposed Design Criteria ²
Flow (Annual Average)	0.222 MGD	0.222 MGD
BOD (Monthly Average)	30 mg/L	30 mg/L
TSS (Monthly Average)	30 mg/L	30 mg/L
Total Phosphorus (Monthly Average)	0.8 mg/L	0.8 mg/L
Total Phosphorus (Annual Load)	134 lbs/year	134 lbs/year
Total Nitrogen (Annual Average)	Monitor Only	Monitor Only
Total Kjeldahl Nitrogen (TKN) (Daily Maximum)	Monitor Only	Monitor Only
Nitrate/Nitrite Nitrogen (NOx) (Daily Maximum)	Monitor Only	Monitor Only
Settleable Solids (Instantaneous Maximum)	1.0 mL/L	1.0 mL/L
E. coli (Instantaneous Maximum)	77 CFU/100 ml	77 CFU/100 mL
рН	6.5-8.5 S.U.	6.5-8.5 S.U.

Notes:

- 1. Source: Richmond WWTF current NPDES Discharge Permit No. 3-1173, effective date January 1, 2021.
- 2. Proposed Effluent Design Criteria is from the WWTF's NPDES Discharge Permit No. 3-1173, effective date January 1, 2021.

4. Alternatives Evaluation

The Richmond WWTF has effectively met its permit requirements over the past 5 years and has not had any exceedances during this time. The WWTF is currently operating at 33% of its design flow capacity and with projected growth described in Section 3.4, is projected to operate at roughly 55% of its design flow capacity in the next 20 years. A physical expansion of existing process tankage to meet the 20-year flows is not required. As discussed in Section 2.6 and Section 3, many of the infrastructure needs for the Richmond WWTF consist of age-related refurbishment and upgrades of existing equipment.

As the existing facility site is located within a floodplain and space is severely restricted for any expansion of the facility and process components, the primary focus of this evaluation is for age-related, in-kind replacement of the existing equipment. For the majority of the needs identified, in-kind replacement is recommended, and an alternatives evaluation is not warranted. Section 4 therefore includes a description of the recommended upgrade or improvement at each process area, conceptual layout if new, cost estimate, and evaluation of alternatives, if alternatives are considered for the specific process area.

4.1 Influent Pumping

As discussed in Section 2.6.1, there are two (2) influent pumps in the lower level of the Operations Building. Wastewater is pumped from the influent wet well up to the screening unit in the Headworks Building. Influent pump #1 was purchased in used condition and installed in 2023 as a temporary replacement for the existing pump which had failed. Influent pump #2 is original to the facility and is not in use as it has a leaking seal and sprays the room when operating. Due to the poor condition of both pumps and lack of redundancy, recommended improvements to influent pumping consist of the replacement of all influent pumps and associated suction and discharge piping with a triplex pumping system with variable frequency drives and flow matching level controls. Pumps will be sized to accommodate the design peak instantaneous flow with the largest pump out of service. Since the historical average daily flow is 0.073 MGD, which is 33% of the design flow, VFDs will be used to maximize the turn-down capacity of each pump. The new pumps will be dry-pit non-clog submersible type pumps.

Design Criteria

Design criteria for the influent pump replacement is presented in Table 4-1 on the following page.

Item Description	Existing Design	Proposed Design	Design Standard
Flows			_
Current ADF	0.073 MGD (50 gpm)	0.073 MGD (50 gpm)	
ADF	0.222 MGD (154 gpm)	0.222 MGD (154 gpm)	
PHF	1.152 MGD (800 gpm)	1.152 MGD (800 gpm)	
Peak Instantaneous Flow		2.3 MGD ¹ (1,597 gpm)	
Influent Pumps			
Number of Units	2	3	
Туре	Centrifugal, Vertical Mounted, Flooded Suction	Dry-pit Submersible	
Capacity, each	800 gpm @ 65' TDH	800 gpm @ 70' TDH	
Total Pump Capacity	1.152 (MGD) (800 gpm)	2.3 MGD (1,597 gpm)	Peak Q w/ one unit out of service
Motor Size	#1: 25 HP #2: 40 HP	approx. 40 HP (pending final design)	
Variable Speed Driven	Yes	Yes	

Table 4-1 Influent Pumping Design Criteria

Notes:

1. Peak instantaneous flow defined as the peak instantaneous flow experienced during the 12/18/2023 rain event.

Exhibit

Figure 4-1 on the following page presents a conceptual layout of the proposed influent pump layout.

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Figure 4-1 Influent Pump Conceptual Layout

Description

Modifications to the influent pumping system will address age-related and hydraulic capacity-related replacement and selection of equipment to meet hydraulic design criteria while taking into account energy efficiency. Proposed influent pumping modifications include:

- Three (3) influent pumps
 - Dry-pit, non-clog, submersible
 - Each rated for 540 gpm
- Variable frequency drives (VFDs)
- Control panel
- New level control system for wetwell
- Replacement of suction and discharge piping and valves
- New discharge magnetic flow meter

Cost Estimate

A preliminary opinion of probable construction cost for improvements to influent pumping is provided in Table 4-2. A detailed breakdown of this opinion of probable cost is provided in Appendix F.

Table 4-2 Influent Pumping – Construction Cost Estimate

Item	Cost ¹
Influent Pumps (3 17 HP dry-pit submersible)	\$221,000
Piping and Valves	\$50,000
VFDs, Control Panel	\$94,400
Level Control System	\$15,000
Magnetic Flow Meter	\$10,000
Instrumentation & Control and Integration Allowance	\$10,000
Total Capital Cost Subtotal	\$400,400
Contractor Mark-Up ²	\$88,000
Total Construction Cost ^{3,4}	\$488,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.2 Headworks

As discussed in Section 2.6.2, the existing Headworks Facility does not provide adequate screening and grit removal. The existing equipment is reaching the end of its useful life and is in need of replacement. The existing screening and grit removal system in the existing Headworks Building consists of a Lakeside Complete Plant Unit System. The existing packaged system incorporates a rotary-type mechanical fine screen and an aerated grit removal chamber with a grit transport screw and grit dewatering screw. Improvements to the Headworks include replacement of the existing screening and grit removal system. Specifically, the following two alternatives were investigated:

- 1. Headworks Alternative #1: Replace-in-Kind Microstrainer and Grit Removal Packaged System
- 2. Headworks Alternative #2: New Multi-Rake Screen and Grit Removal

A third alternative for a completely stand-alone Headworks Building upstream of the existing wet well was considered, but not deemed feasible based on site constraints and encroachment into the flood plain and was therefore not taken developed or evaluated further.

4.2.1. Headworks Alternative #1: Replace-in-Kind - Microstrainer and Grit Removal Package System

Headworks Alternative #1 consists of replacing the existing Lakeside Complete Plant screening and grit removal unit in-kind with a new microstrainer and grit removal packaged system consisting of an aerated grit chamber, grit transfer screw, and grit dewatering screw.

Micro strainers have small apertures that catch small debris. Screenings are then washed, compacted, and dewatering as they are transported up the inclined screw for disposal. Aerated grit chambers rely on air injected into the chamber to create a downward circulating flow pattern keeping the organics in suspension while allowing the heavier grit to settle at the bottom of the chamber where it is collected in a hopper and removed using a grit screw. Removed grit is sent to a grit classifier for washing and dewatering. Washed and dewatered grit will be discharged into a container for disposal.

Vortex type grit chambers are another common grit removal technology. However, a vortex type grit chamber was not considered an appropriate technology for the Richmond WWTF due to space constraints in the existing Headworks Building and limited available land area for a new Headworks Building.

New replacement screening and grit removal equipment will be housed in the existing Headworks Building rated for Class I, Division 1 space. A HVAC upgrade will be included to provide adequate air changes for the classified space, and a gas detection system will be included to monitor for the buildup of hazardous gases.

Design Criteria

The design criteria for Headworks Alternative #1 is presented in Table 4-3 on the following page.

Item Description	Existing Design	Proposed Design	Design Standard
Flow			
Design ADF	0.222 MGD	0.222 MGD	
Design PHF	1.152 MGD	1.152 MGD	
Peak Instantaneous		2.3 MGD ¹	
Screening			
Capacity	2.97 MGD	2.3 MGD	
Туре	Rotary – Mechanical, Fine Screen	16" Microstrainer	
Dimensions	3/8" bar spacings ½" screen openings spacing 31" screen basket diameter	1/8" perforated plate screen	0.25-1.5 inch (TR-16)
Motor Drive	2 HP	2 HP	
Grit Removal	·		
Type of System	Lakeside Complete Plant – Aerated Grit Removal Chamber w/ Grit Transfer Screw & Dewatering Screw	Lakeside Complete Plant – Aerated Grit Removal Chamber w/ Grit Transfer Screw & Dewatering Screw	
Transfer Screw	8" dia. w/ 1 HP drive	8" dia. w/ 1 HP drive	
Dewatering Screw	8" dia. w/ 2 HP drive	8" dia. w/ 2 HP drive	
Grit Blower	2 HP	2 HP	
Diffusers	SS Coarse bubble	SS Coarse bubble	
Chamber Dimensions	26' x 3.5'	26' x 3.5'	
SWD	9.17'	9.17'	
Material	Stainless Steel	Stainless Steel	

Table 4-3 Headworks Alternative #1 Design Criteria

Notes:

1. Peak instantaneous flow defined as the peak instantaneous flow experienced during the 12/18/2023 rain event.

Description

Headworks Alternative 1 to replace the existing screening and grit removal equipment in-kind includes the following components:

- Building modifications to remove existing equipment and install new
- Lakeside Headworks Acceptance Plant with:
 - Microstrainer
 - 16" diameter w/ spray wash manifold and solenoid valves
 - 1/8" diameter perforated plate
 - 2 HP drive
 - Aerated grit chamber
 - 8" diameter transfer screw w/ 1 HP drive

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- 8" diameter dewatering screw w/ 2 HP drive
- Air header with stainless steel diffusers
- Blower package w/ 2 HP motor
- NEMA 4X main control panel with Allen Bradley PLC
- HVAC upgrade for Headworks Building
- Gas detection system

Cost Estimate

A preliminary opinion of probable construction cost for Headworks Alternative #1 is provided in Table 4-4. A detailed breakdown of this opinion of probable cost is provided in Appendix F.

Table 4-4 Headworks Alternative #1 – Replace-in-Kind – Construction Cost Estimate

Item	Cost ¹
Building Modifications for Demolition and Installation	\$60,000
New Screening/Grit Headworks Acceptance Plant	\$696,500
HVAC Upgrade for Headworks Building	\$75,000
Gas Detection System	\$15,000
Total Capital Cost Subtotal	\$846,500
Contractor Mark-Up ²	\$186,000
Total Construction Cost (rounded) ^{3,4}	\$1,033,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

Advantages/Disadvantages

Advantages

- Microstrainers are ideal screening technology for small facilities
- Stainless steel construction resists corrosion
- Package unit eliminates piping, valves, and gates in Headworks system
- Screens, washes, and dewaters solids in one unit
- Pre-assembled components

Disadvantages

- Does not provide screening prior to the influent pumps to protect pumps from damage and ongoing operational and maintenance issues.
- Requires modifications to existing Headworks Building to remove and install new equipment

4.2.2. Headworks Alternative #2: Multi-Rake Screen and Grit Removal

Headworks Alternative #2 consists of installing a multi-rake screen in the influent wetwell ahead of the influent pumps and replacing the existing combined screening and grit removal package in the Headworks Building with a grit removal system. The multi-rake screen will be located in the influent channel in the existing influent wetwell. A new Screening Building with an intermediate floor at grade level will be built over the top of the existing influent wet well to house the new screen, wash press, and screenings disposal container. Implementing screening ahead of influent pumping will protect the pumps from ongoing damage from rocks, rags, and debris.

In addition to a new Screening Building, improvements to the existing Headworks Building will be made. The existing screening and grit removal package system will be replaced with a grit removal system consisting of a package unit with an aeration zone, a settling area, baffles, and grit screw in the existing screen/grit removal unit location. A HVAC upgrade of the existing Headworks Building will be included to provide adequate air changes for the classified space, and a gas detection system will be included to monitor the buildup of hazardous gases.

Design Criteria

The design criteria for Headworks Alternative #2 is presented in Table 4-5 on the following page.

Item Description	Existing Design	Proposed Design	Design Standard
Flow			
Design ADF	0.222 MGD	0.222 MGD	
Design PHF	1.152 MGD	1.152 MGD	
Peak Instantaneous		2.3 MGD ¹	
Wetwell Influent Channel			
Dimonsions	1'-7" wide	2'-0" wide	
Dimensions	1'-7" deep	2'-0" deep	
Screening			
Туре	Rotary – Mechanical, Fine Screen	Multi-Rake Screen	
Dimensions	3/8" bar spacings ½" screen openings spacing 31" screen basket diameter	¼" bar spacings	0.25-1.5 inch (TR-16)
Screen Angle		80-degree	
Bar Rack Screening Width		1'-2"	
Total invert elevation to operating floor level		22'-0"	
Discharge Height		4'-6"	
Peak Capacity	2.97 MGD	1.5 MGD	
Motor Drive	2 HP	3 HP	
Wash Press			
Diameter of Hollow Shaft		8.5″	
Motor Drive		5 HP	
Water requirements	5-20 gpm @ 60 psi	19 gpm @ 35 psi	
Grit Removal			•
	Lakeside Complete Plant –	Lakeside Aerated Grit	
Tupo of System	Aerated Grit Removal	Removal Chamber w/	
Type of System	Chamber w/ Grit Transfer	Grit Transfer Screw &	
	Screw & Dewatering Screw	Dewatering Screw	
Transfer Screw	8" dia. w/ 1 HP drive	8" dia. w/ 1 HP drive	
Dewatering Screw	8" dia. w/ 2 HP drive	8" dia. w/ 2 HP drive	
Grit Blower	2 HP	2 HP	
Diffusers	SS Coarse bubble	SS Coarse bubble	
Chamber Dimensions	26' x 3.5'	26' x 3.5'	
SWD	9.17′	9.17'	
Material	Stainless Steel	Stainless Steel	

Table 4-5 Headworks Alternative #2 Design Criteria

Notes:

1. Peak instantaneous flow defined as the peak instantaneous flow experienced during the 12/18/2023 rain event.

Exhibit

Figure 4-2 presents a sectional view of the proposed new multi-rake screen in the existing influent wetwell.



Figure 4-2 Headworks Alternative 2 – New Multi-Rake Screen in Existing Influent Wetwell

Description

Headworks Alternative 2 to replace the existing screening and grit removal equipment with a multi-rake screen ahead of influent pumping and new aerated grit removal unit includes the following components:

New Screening Building

- Multi-rake Screen
 - o 304 stainless steel construction
 - 80-degree setting angle
 - Clear bar spacing: 1/4"
 - Discharge height: 4'-6"
 - Explosion-proof 3.0 HP motor
 - o Stainless steel drive chains
 - o Static guide rail bearing lower bar rack engagement system
 - Screen side frames recessed in the channel walls giving a 1'-2" effective bar rack width
 - Stainless steel covers above floor level

- Wash Press
 - 304 Stainless steel construction
 - Spray wash system with explosion proof solenoid valves
 - Washing Press inlet hopper
 - Washing Press deep drain pan design
 - Washing Press discharge piping
 - Explosion-proof 5.0 HP motor
- NEMA 7 Local Control Stations rated for Class 1, Division 1, Group D hazardous area
- NEMA 4X Main Control Panel, to include VFD (screen), motor starter (press), Milltronics HydroRanger 200 HMI Differential Level Controller, Allen-Bradley Micro 800 PLC and Red Lion OIT
- Headworks Building (29' x 13.5') rated for Class 1, Division I hazardous space
- Provide ventilation for compliance with current NFPA 820 requirements

Existing Headworks Building Modifications

- Building modifications to remove existing equipment and install new
- Lakeside Headworks Acceptance Plant with:
 - Aerated grit chamber
 - 8" diameter transfer screw w/ 1 HP drive
 - 8" diameter dewatering screw w/ 2 HP drive
 - o Air header with stainless steel diffusers
 - Blower package w/ 2 HP motor
 - \circ ~ NEMA 4X main control panel with Allen Bradley PLC ~
- HVAC upgrade for Headworks Building
- Gas detection system

Cost Estimate

A preliminary opinion of probable construction cost for Headworks Alternative #2 is provided in Table 4-6 on the following page. A detailed breakdown of this opinion of probable cost is provided in Appendix F.

Table 4-6 Headworks Alternative #2 – Construction Cost Estimate

Item	Cost ¹
New Screening in Existing Wetwell	
Screening (Multi-Rake Screen and Wash Pressing)	\$363,000
New Headworks Building Above Influent Wetwell	\$315,500
Grit Removal in Existing Headworks Building	
Grit Removal System (Aeration Zone, Settling Area, Baffles, Grit Screw)	\$527,500
Building Modifications for Demolition and Installation	\$60,000
HVAC Upgrade for Headworks Building	\$75,000
Gas Detection System	\$15,000
Total Capital Cost Subtotal	\$1,356,000
Contractor Mark-Up ²	\$298,000
Total Construction Cost (rounded) ^{3,4}	\$1,654,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

Advantages/Disadvantages

Advantages

- Allows for screening prior to influent pumping to protect influent pumps from rocks and other debris that have routinely caused operation and maintenance issues with influent pumping
- Durable and strong-wear resistance
- Brings screenings to upper level for easier removal
- No motors or electrical components in lower area subject to flooding

Disadvantages

- Construction of new Headworks Building structure
- Requires modifications to existing Headworks Building to remove and install new equipment

4.2.3. Headworks Alternative #3: New Headworks Building

Headworks Alternative #1 proposes the existing Lakeside Complete Plant Unit screening & grit removal system be replaced in-kind. Headworks Alternative #2 proposes installing a multi-rake screen in the existing wet well to provide protection to the influent pumps and replacing the existing screening & grit removal system with a grit removal system in the same channel.

A third Headworks Alternative was taken into consideration, consisting of construction of a new Headworks Building upstream of the wet well. However, the Richmond WWTF lacks available land space for a new Headworks Building to implement screening & grit removal technologies upstream of the wet well. Additionally, any available space for a new Headworks Building is located below the 100-year and 500-year flood elevations. Due to these reasons, Headworks Alternative #3 was not deemed feasible and was therefore not taken into consideration in further alternatives evaluation.

4.2.4. Comparison of Construction Cost Estimates of Headworks Alternatives

Detailed opinions of probable construction cost for each of the Headworks alternatives are provided in Appendix F. Table 4-7 on the following page provides a summary of the opinion of probable construction cost for each alternative that was considered.

Cable 4-7 Summary of Headworks Alternatives	s – Preliminary Opinion of Probable Capital	Cost
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Item	Alternative 1 Screening & Grit Removal Package System	Alternative 2 Multi-Rake Screen in New Headworks Building & Grit Classifier System
Package Screening/Grit Removal System	\$696,500	
Screen (Multi-Rake Screen)		\$363,000
Grit Removal System (Aerated Grit Chamber, Grit Transfer Screw, Grit Dewatering Screw)		\$527,500
New Headworks Building		\$315,500
Existing Headworks Modifications	\$60,000	\$60,000
Existing Headworks HVAC Upgrade	\$75,000	\$75,000
Existing Headworks Gas Detection System	\$15,000	\$15,000
Total Capital Cost Subtotal ¹	\$846,500	\$1,356,000
Contractor Mark-Up ²	\$186,000	\$298,000
Total Construction Cost (rounded) ^{3,4}	\$1,033,000	\$1,654,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.2.5. Comparison of Life Cycle Cost of Headworks Alternatives

Life Cycle Costs

Life cycle costs were prepared for each Headworks Alternative. Life cycle costs include estimates of annual power consumption, replacement parts, and operation and maintenance time. Detailed information for each alternative is included in Appendix G.

The following assumptions and variables were considered in the development of life cycle costs:

- Estimated annual electrical cost of operation based on equipment operating horsepower and operating hours and an average utility electric rate of \$0.149/kW-hr taking into account on-peak and off-peak rates and \$16.7/day customer service charge as per Richmond WWTF's Green Mountain Power bill including customer charge, total energy efficiency charge, extreme storm restoration fund, electric assistance program fee, and energy/major storm adjustor.
- Estimated labor requirements for operation and maintenance at an assumed labor rate of \$50.00/hr.

Table 4-8 summarizes the life cycle costs for each Headworks alternative.

Table 4-8 Headworks Alternatives – Opinions of Life Cycle Cost

	Alt 1. Replace-in-Kind	Alt 2. Multi-Rake Screen & Grit Removal
Annual Power Cost	\$9,100	\$10,500
Annual Equipment Replacement Costs	\$7,600	\$3,600
Annual Labor Cost	\$18,200	\$18,200
Total Annual O&M Cost (rounded)	\$35,000	\$32,000

Present Worth Analysis

A present worth analysis was performed to further compare the various alternatives and the results are summarized below.

The following assumptions and variables were considered in the present worth analysis:

- Planning period of 20 years
- Escalation rate of 3% annually
- Discount rate of 2.5% (based on EPA Fiscal Year 2023)

A summary of present worth analysis for the Headworks Alternatives is presented in Table 4-9 on the following page.

Table 4-9 Present Worth Analysis of Headworks Alternatives
--

	Alt 1. Replace-in-Kind	Alt 2. Multi-Rake Screen & Grit Removal
Total Project Cost of Alternative	\$1,033,000	\$1,654,000
Annual O&M Cost of Alternative	\$35,000	\$32,000
Present Worth of Alternative	\$1,784,000	\$2,341,000

Notes:

- 1. Total project costs are inclusive of construction costs and contractor mark-up, but do not include contingency, engineering services, legal and administrative costs.
- 2. ENR Construction Cost Index = 13514.76 (December 2023)

As shown in Table 4-9, Alternative 2 is the highest present worth value based on the highest project cost. Alternative 1 has the lowest present worth based on the lowest project cost, 37.5% less than the cost of Alternative 2.

4.3 Biological Process - Anoxic Selectors

The biological process at Richmond WWTF starts in the Headworks Building where flow is directed through three (3) anoxic selectors. The first two (2) selectors are 3,000 gallons while the third selector is 6,000 gallons. Anoxic selectors are used to denitrify nitrified mixed liquor in the return stream and improve settling. Assessment of the existing anoxic selectors equipment in Section 2.6.3 revealed that age-related upgrades are needed for the submersible mixers. Anoxic selector improvement will involve replacement of the submersible mixers. Specifically, two alternatives were investigated:

- 1. Replace submersible mixers in-kind
- 2. Replace submersible mixers with a compressed gas mixing system

A third alternative explored the concept of replacing the existing submersible mixers with hyperbolic mixers. Hyperbolic mixers are designed to operate at the bottom of a tank, draw water down, and discharge it in a horizontal direction. However, the existing anoxic selectors at the Richmond WWTF are too small to make this a feasible mixing alternative and therefore it was not developed or evaluated further.

4.3.1. Anoxic Selectors Alternative #1: Submersible Mixers

Anoxic Selectors Alternative #1 proposes replacing the existing submersible mixers in-kind.

Design Criteria

The design criteria for Anoxic Selectors Alternative #1 is provided in Table 4-10 on the following page.

Item Description	Existing Design	Proposed Design	Design Standard
Anoxic Selectors 1 & 2			
Quantity	2	2	
Capacity	3,000 gal (each)	3,000 gal (each)	
Dimensions	8.25' x 6.0'	8.25' x 6.0'	
SWD	8.10′	8.10′	
Anoxic Selector 3			
Quantity	1	1	
Capacity	6,000 gal	6,000 gal	
Dimensions	16.50' x 6.0'	16.50' x 6.0'	
SWD	8.10′	8.10′	
Submersible Mixers			
Quantity	3 (1/anoxic tank)	3 (1/anoxic tank)	TR-16: Independent mixing should be provided.
Туре	Submersible Mixers	Submersible Mixers	
Power Draw	1.21 HP (each)	1.2 HP (each)	

Table 4-10 Anoxic Selectors Alternative #1 – Replace In-Kind – Design Criteria

Description

Anoxic Selectors Alternative #1 is to replace the three (3) existing submersible mixers in-kind, consisting of the following:

- Three (3) submersible mixers
 - 8.31" diameter, 2-blade stainless steel propeller
 - o 1.2 HP motor
 - Explosion proof

Cost Estimate

A preliminary opinion of probable construction cost for Anoxic Selectors Alternative #1 is provided in Table 4-11 on the following page. A detailed breakdown of this opinion of probable cost is provided in Appendix F.

Item	Cost ¹
Submersible Mixer Replacements	\$31,400
Process Electrical & Instrumentation	\$10,000
Total Capital Cost Subtotal	\$41,400
Contractor Mark-Up ²	\$9,000
Total Construction Cost (rounded) ^{3,4}	\$50,000

Table 4-11 Anoxic Selectors Alternative #1 – Replace-in-Kind – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

Advantages/Disadvantages

Advantages

• Conventional method for anoxic selector mixing.

Disadvantages

- Uses more energy than compressed gas systems, which translates to higher annual operating costs.
- Mixers are located in tanks and require removal to be serviced.
- Less reliable than compressed air mixing systems

4.3.2. Anoxic Selectors Alternative #2: Compressed Gas Mixing System

Anoxic Selectors Alternative #2 is to provide a compressed gas mixing system in the anoxic selector tanks. Compressed gas mixing provides bursts of compressed air fired through nozzles located near the selector floor, providing uniform mixing of tank contents. The large size of the bubbles and intensity of delivery inhibits transfer of oxygen to the wastewater, maintaining anoxic conditions in the selectors.

Design Criteria

The design criteria for Anoxic Selectors Alternative #2 is provided in Table 4-12 on the following page.

Item Description	Existing Design	Proposed Design	Design Standard
Mixing System			
Tuno	Submarsible Miyers	Compressed Gas	
туре		Mixing System	
		1 norale stid/teals	TR-16: Independent
Quantity	3 (1/anoxic tank)	1 nozzie grid/tank	mixing should be
		1 compressor	provided.
		Compressors (2)	100% redundancy
Туре	Mixers (3)	1 duty, 1 stand-by	with largest unit out
		13 scfm @ 116 psi	of service
Motor Size, each	1.21 HP	3 HP	
Receiver Tank		120-gallon	
Anoxic Selector 1 & 2	L		
Quantity	2	2	
Capacity	3,000 gal (each)	3,000 gal (each)	
Dimensions	8.25' x 6.0'	8.25' x 6.0'	
SWD	8.10'	8.10'	
Solids Conc.	0.5%	0.5%	
Headers/Tank		1	
Nozzles/Header		2	
Nozzles/Tank		2	
Nozzle Density		24.8 ft ² /nozzle	
Anoxic Selector 3			
Quantity	1	1	
Capacity	6,000 gal	6,000 gal	
Dimensions	16.50' x 6.0'	16.50' x 6.0'	
SWD	8.10'	8.10'	
Solids Conc.	0.5%	0.5%	
Headers/Tank		1	
Nozzles/Header		3	
Nozzles/Tank		3	
Nozzle Density		33 ft ² /nozzle	

Table 4-12 Anoxic Selectors Alternative #2 – Compressed Gas Mixing – Design Criteria

Description

Anoxic Selectors Alternative #2 is to replace the three (3) existing submersible mixers with a compressed gas mixing system, consisting of:

- Implementation of a compressed gas mixing system in the three (3) anoxic selectors
 - \circ ~ Valve Module Control Panel with electrically actuated valves
 - \circ $\;$ 304 Stainless steel nozzles, headers, and header supply piping $\;$

- (2) 3 HP Rotary Screw Compressor (combined with the system for septage & sludge storage)
- 120-Gallon Receiver Tank (combined with the system for septage & sludge storage)

A layout of the compressed gas mixing system in the anoxic selectors is provided in Figure 4-3.



Figure 4-3 Anoxic Selectors Alternative #2 - Compressed Gas Mixing System Layout

Cost Estimate

A preliminary opinion of probable construction cost for Biological Process – Anoxic Selectors Alternative #2 is provided in Table 4-13. A detailed breakdown of this opinion of probable cost is provided in Appendix F.

Item	Cost ¹
Compressed Gas Mixing System	\$ 44,2 00
Process Electrical and Instrumentation	\$15,000
Total Capital Cost Subtotal	\$59,200
Contractor Mark-Up ²	\$13,000
Total Construction Cost (rounded) ^{3,4}	\$72,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

Advantages/Disadvantages

Advantages

- Provides over 50% energy savings over mechanical mixing.
- EnviroMix provides unlimited turn-down capability.
- EnviroMix has zero in-tank maintenance with no mechanical or electrical components in the wastewater.
- Compressors can be combined with mixing systems for sludge holding tanks to perform dual duty and save costs.

Disadvantages

• Less conventional methodology and equipment to maintain.

4.3.3. Comparison of Construction Cost Estimates of Anoxic Selector Alternatives

Detailed opinions of probable construction cost for each of the Anoxic Selector alternatives are provided in Appendix F. Table 4-14 provides a summary of the opinion of probable construction cost for each alternative that was considered.

Table 4-14 Summary	v of Anoxic Selector	Alternatives – Preliminary	v Opinion of Proba	able Capital Cost
			,	

Item	Alternative 1 Mixer Replacement-in- Kind	Alternative 2 Compressed Gas Mixing System
Mixing System	\$31,400	\$44,200
Process Electrical & Instrumentation	\$10,000	\$15,000
Total Capital Cost Subtotal ¹	\$41,400	\$59,200
Contractor Mark-Up ²	\$9,000	\$13,000
Total Construction Cost (rounded) ^{3,4}	\$50,000	\$72,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.3.4. Comparison of Life Cycle Cost of Anoxic Selector Alternatives

Life Cycle Costs

Life cycle costs were prepared for each Anoxic Selector Alternative. Life cycle costs include estimates of annual power consumption, replacement parts, and operation and maintenance time. Detailed information for each alternative is included in Appendix G.

The following assumptions and variables were considered in the development of life cycle costs:

- Estimated annual electrical cost of operation based on equipment operating horsepower and operating hours and an average utility electric rate of \$0.149/kW-hr taking into account on-peak and off-peak rates and \$16.7/day customer service charge as per Richmond WWTF's Green Mountain Power bill including customer charge, total energy efficiency charge, extreme storm restoration fund, electric assistance program fee, and energy/major storm adjustor.
- Estimated labor requirements for operation and maintenance at an assumed labor rate of \$50.00/hr.

Table 4-15 summarizes the life cycle costs for each Anoxic Selector alternative.

	Alt 1. Submersible Mixers	Alt 2.
		compressed das mixing
Annual Power Cost	\$7,300	\$6,900
Annual Equipment Replacement Costs	\$400	\$600
Annual Labor Cost	\$2,600	\$50
Total Annual O&M Cost (rounded)	\$10,000	\$8,000

Table 4-15 Anoxic Selector Alternatives – Opinions of Life Cycle Cost

Present Worth Analysis

A present worth analysis was performed to further compare the various alternatives and the results are summarized below.

The following assumptions and variables were considered in the present worth analysis:

- Planning period of 20 years
- Escalation rate of 3% annually
- Discount rate of 2.5% (based on EPA Fiscal Year 2023)

A summary of present worth analysis for the Anoxic Selector Alternatives is presented in Table 4-16 on the following page.

	Alt 1.	Alt 2.	
	Submersible Mixers	Compressed Gas Mixing	
Total Project Cost of Alternative	\$50,000	\$72,000	
Annual O&M Cost of Alternative	\$10,000	\$8,000	
Present Worth of Alternative	\$265,000	\$244,000	

Table 4-16 Present Worth Analysis of Anoxic Selector Alternatives

Notes:

- 1. Total project costs are inclusive of construction costs and contractor mark-up, but od not include contingency, engineering services, legal and administrative costs.
- 2. ENR Construction Cost Index = 13514.76 (December 2023)

As shown in Table 4-16, Alternative 1 is the highest present worth value based on the highest O&M cost. Alternative 2 has the lowest present worth based on the lowest O&M. The present worth of Alternative 2 is 7.9% less than Alternative 1.

4.4 Biological Process – Aeration Tanks

Flow from the anoxic selectors is split between two (2) 150,000 gallon aeration tanks. A biological model was created to analyze the current loadings seen at the WWTF and to determine treatment capacity of the existing facility at permitted flow to define design criteria. See Appendix D for a summary of the modeling results and calculations. Following the model optimization, the existing aeration tank equipment was then assessed to determine if it could treat future loadings. The assessment revealed that the existing blowers and diffused aeration system are undersized for the design nutrient loadings and cannot supply the needed air for maximum loading conditions at design flows.

The recommended improvements include replacing the diffusers in the aeration tanks, replacing the aeration tank blowers with larger capacity blowers that can supply enough air for the maximum loadings at design flows as well as at current flows, re-piping the blower air intake to eliminate drawing foul, corrosive air from the sludge holding tanks, replacing the DO probes in the aeration tanks, implementing D.O. control logic for automatic blower output, and providing a second air discharge to the aeration tanks to improve air balancing between the basins.

Design Criteria

Design criteria for Biological Process – Aeration Tanks is presented in Table 4-17.

Description	Existing Design	Proposed Design	Design Standard
Flow			
Design ADF	0.222 MGD	0.222 MGD	
Design PHF	1.152 MGD	1.152 MGD	
Aeration Basins			
Quantity	2	2	
Dimensions	38' x 38'	38' x 38'	
SWD	14'	14'	
Volume	150,000 gal (each)	150,000 gal (each)	

Table 4-17 Biological Process – Aeration Tanks – Design Criteria

Description	Existing Design	Proposed Design	Design Standard
AOR	 Max: 1350 lb/day	Min: 377 lb O2/day Max: 1810 lb O2/day	TR-16 Mixing Requirements (minimum): 0.12 scfm/sf of tank area TR-16 Oxygen Supply: 0.85- 1.2 lbs Oxygen/lbs BOD removed + 4.2 lbs Oxygen/lbs Ammonia Oxidized at maximum daily loading conditions
SOTR		Min: 1080 lb O2/day Max: 5185 lb O2/day	
Air Flow	Min: 220 scfm	Min: 115 scfm	
	Max: 450 scfm	Max: 550 scfm	

Description	Existing Design	Proposed Design	Design Standard			
Blowers						
Quantity	2	2	Peak Q w/ one unit out of service Peak Q w/ one unit out of service			
Туре	Positive Displacement	Positive Displacement				
Capacity, each	220 to 450 scfm @ 7 psi	77 to 550 scfm @ 7 psi				
Control Strategy	DO Control (not operable)	DO Control				
Motor	25 HP (each)	30 HP (each)				
VFDs	Yes	Yes				
Diffusers						
Туре	Fine Bubble Flexible Membrane Diffuser	Fine Bubble Diffusers/Aeration System	TR-16: diffuser design must provide max and min air flow rates while maintaining tank mixing. Individual diffuser grids should be equipped with control valves for throttling or complete shutoff.			
Quantity	162/tank	189/tank				
Discharge Pressure	7 psig	7 psig				

Description

The proposed Biological Process – Aeration Tanks improvements would include the following components:

- (2) New diffused aeration grids each with:
 - (9) 4" PVC headers
 - (21) 9" diameter fine bubble EDPM diffusers per header
 - Total of 189 diffusers
 - 4" PVC drop leg
 - Purge sump assembly
- (2) new positive displacement blowers
 - Sound enclosures
 - o VFDs
- New D.O. Probes (2) in aeration tanks
- New outdoor air intake
- Demolition of air intake from sludge holding tanks

- New air discharge piping to aeration tanks
- SCADA integration for D.O. pacing of blowers

Cost Estimate

A preliminary opinion of probable construction cost for the proposed Biological Process – Aeration Tanks improvements is provided in Table 4-18. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Table 4-18 Biological	Process – Aeration	Tanks – Construction	Cost Estimate
146.6 1 20 5.0.08.04			0000 2000000

Item	Cost ¹
New Fine Bubble Diffusers	\$68,300
New Blowers (VFDs, new air intake & associated piping modifications)	\$237,000
New Separate Air Header to Aeration Tank #2	\$24,000
Process Electrical, Instrumentation & SCADA programming	\$48,000
Total Capital Cost Subtotal	\$377,300
Contractor Mark-Up ²	\$84,000
Total Construction Cost (rounded) ^{3,4}	\$462,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.5 Coagulant Chemical Feed & Storage

Chemical feed and storage at the Richmond WWTF currently consists of one (1) 1,500 gallon sodium aluminate storage tank and two (2) positive displacement diaphragm type feed pumps (one in-use, one in storage) located in the basement of the Process Building. Additionally, the existing chemical spill containment volume is insufficient and does not meet TR-16 standards. Chemical feed and storage improvements will involve constructing additional height to the existing containment wall to provide sufficient spill containment volume and installation of new duplex chemical feed pumps for the existing coagulant storage and feed system. A second dosing point to the influent wet well will be provided along with the existing dosing point in the aeration tank effluent channel. A description of the chemical feed systems at the dewatering building is included in Section 4.14.

Design Criteria

Design criteria for the new chemical feed and storage improvements is presented in Table 4-19 on the following page.

Item Description Existing Design		Proposed Design	Design Standard			
Coagulant Storage for Biological Process						
Congulant Storage	1,500 gallon	1,500 gallon				
Coagulant Storage	storage tank	storage tank				
Spill Containment Volume	1 EOE gallons	1 97E gallons	125% of storage tank			
spin containment volume	1,505 galions	1,075 galions	volume (TR-16)			
Coagulant Feed for Biological P	rocess					
Rump Quantity	2	2	Minimum of 2 (1 duty, 1			
Pump Quantity	(1 duty, 1 on shelf)	(1 duty, 1 stand-by)	stand-by) (TR-16)			
	Aaratad Tank	Influent Wet Well,				
Dosing Points	Effluent Channel	Aeration Tank				
		Effluent Channel				

Table 4-19 Chemical Feed & Storage Design Criteria

Description

The proposed chemical feed and storage facilities includes:

- Additional height to concrete containment wall for existing chemical storage area
- Chemical feed pumps
 - Biological Process Chemical Feed: skid-mounted duplex pump chemical feed system

Cost Estimate

A preliminary opinion of probable construction cost for coagulant chemical storage and feed improvements is provided in Table 4-20. A detailed breakdown of this opinion of probable cost is provided in Appendix F.

Table 4-2	0 Coagulant	Chemical Fe	ed and St	orage – Co	onstruction (Cost Estimate
	e eeagararre					Jobe Lotiniate

Item	Cost ¹
Chemical Spill Containment – 8" wall addition	\$1,400
Chemical Feed Pumps (duplex pump skid), installed	\$16,900
Miscellaneous Chemical Feed Piping and Valves	\$5,000
Electrical, Instrumentation & Controls	\$5,000
Total Capital Cost Subtotal	\$28,300
Contractor Mark-Up ²	\$6,000
Total Construction Cost (rounded) ^{3,4}	\$34,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.6 Secondary Clarification

The two (2) rectangular secondary clarifiers and associated equipment and parts are in good working condition and do not require an overhaul beyond regular cleaning and maintenance for proper operation of the biological treatment process.

4.7 Return Activated Sludge & Waste Activated Sludge Pump System

The existing waste activated sludge (WAS) pump system consists of one (1) WAS pump located in the Pump Room of the lower level of the Operations Building. RAS flows by gravity and is controlled by an electrically actuated pinch valve and measured with a magnetic flow meter. Due to the age of existing equipment, replacement of the existing RAS pinch valve and magnetic flow meter are recommended and discussed in Section 4.6.1 below. Due to the existing WAS pump being obsolete, lack of pump redundancy, frozen plug valves, and condition of existing equipment, improvements are recommended to the existing WAS pumping system.

4.7.1. Return Activated Sludge System

Improvements to the return activated sludge system include replacement of the existing pinch valve and electric actuator and flow meter with new components. No provisions for redundancy are recommended as the pinch valve would fail in the open position and flow could be controlled by manually throttling valves while replacement parts are procured.

Design Criteria

Design criteria for the RAS system are presented in Table 4-21.

	0 / 0				
Item Description	Existing Design	Existing Design Proposed Design			
Return Activated Sludge (RAS)					
RAS Flow Target (150% ADF)	90-150% ADF	90-150% ADF			
RAS Capacity	0.0657 - 0.333 MGD	0.0657 - 0.333 MGD			
RAS Control Valve Quantity	1	1			
RAS Valve Size	4-inch	4-inch			
RAS Value Tures	Electrically Actuated	Electrically Actuated			
RAS valve Type	Pinch Valve	Pinch Valve			
RAS Flow Meter Quantity	1	1			
RAS Flow Meter Type	Magnetic	Magnetic			
RAS Flow Meter Size	4-inch	4-inch			

Table 4-21 Return Activated Sludge System Design Criteria

Description

The proposed return activated sludge system improvements would include the following components:

- RAS Valve Replacement
 - One (1) Pinch Valve
 - One (1) Electric Actuator
 - One (1) Magnetic Flow Meter

Cost Estimate

A preliminary opinion of probable construction cost for the proposed return activated sludge system is provided in Table 4-22. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item	Cost ¹
RAS Pinch Valve & Actuator Replacement	\$33,500
New RAS Flow Meter	\$13,000
Total Capital Cost Subtotal	\$46,500
Contractor Mark-Up ²	\$10,000
Total Construction Cost (rounded) ^{3,4}	\$57,000

Notes:

- 5. ENR Construction Cost Index = 13514.76 (December 2023)
- 6. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 7. Total Construction Costs do not include engineering services, legal and administrative costs.
- 8. Total Construction Cost does not include contingency.

4.7.2. Waste Activated Sludge System

Improvements to the existing waste activated sludge system include the replacement in-kind of the existing double-disc, positive-displacement WAS pump and suction and discharge valving. It should be noted that replacement of the sole WAS pump does not provide redundancy for WAS pumping in the event that the pump is inoperable. Contingencies should be in place to provide temporary bypass WAS pumping by means of lowering a trash pump into either clarifier and running a discharge hose to the outdoor aerobic sludge holding tank.

Design Criteria

Design criteria for the WAS system are presented in Table 4-23 on the following page.

Item Description	Existing Design	Proposed Design	Design Standard			
Waste Activated Sludge (WAS)						
Number of Pumps	1	1				
	Double-Disc,	Double-Disc,				
Pump Type	Positive-	Positive-				
	Displacement	Displacement				
Capacity	150 gpm @ 18' TDH	150 gpm @ 18' TDH	At facilities with an ADF of 10 MGD or less, WAS pumping should have a maximum capacity of			
Minimum Flow Rate	-	80 gpm	25% of ADF and provide a minimum flow rate of ~80 gpm. (10 State Standards)			
Suction Condition	Flooded	Flooded				
Motor Power	7.5 HP	7.5 HP				
VFD	Yes	Yes				
Material	Sludge, 0.5-1% solids	Sludge, 0.5-1% solids				

Table 4-23 Waste Activated Sludge System Design Criteria

Description

The Waste Activated Sludge System would include the following components:

- WAS Pump
 - One (1) Double-Disc, Positive-Displacement pump
- New plug valves on suction and discharge piping

Cost Estimate

A preliminary opinion of probable construction cost for replacement of the Waste Activated Sludge Pump System is provided in Table 4-24 on the following page. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item	Cost ¹
WAS Pump Replacement	\$53,000
Plug Valves	\$24,000
Total Capital Cost Subtotal	\$77,000
Contractor Mark-Up ²	\$18,000
Total Construction Cost (rounded) ^{3,4}	\$95,000

Table 4-24 Waste Activated Sludge System Replacements – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.8 Filtration

As the existing cloth media filters have routinely been meeting the effluent phosphorus limits, and as it is not feasible to replace the existing filter tanks with larger package filters due to space constraints of the existing building, the recommendation is to remain with the existing units and make systematic repairs and replace parts. Therefore, the recommended upgrade of the two (2) cloth media filter units in the Filter/UV Room on the upper level of the Process Building includes a structural assessment of the existing steel tanks to determine in-place tank repairs needed, replacement of the cloth media, backwash and sludge removal pumps, drive motors, and valves and piping, and SCADA programing.

Design Criteria

Design criteria for the filtration system is presented in Table 4-25.

Description	Existing Design	Proposed Design	Design Standard
Flows			
Average Flow	0.222 MGD	0.222 MGD	
Peak Hourly Flow	1.152 MGD	1.152 MGD	Treat PHF with 1 unit off-line
Туре	Cloth Media Disk	Cloth Media Disk	
Filter Tanks			
Number of Tanks	2	2	
Manufacturer/ Model Number	Aqua-Aerobic Systems, Inc. Cloth Media Filter ADFP- 54X2E-PC	Aqua-Aerobic Systems, Inc. Cloth Media Filter ADFP- 54X2E-PC	
Number of Disks, Total	4 (2, 2-Disk Units)	4 (2, 2-Disk Units)	
Dimensions	8' x 9'-2"	8' x 9'-2"	
SWD	7.84'	7.84'	
Max Water Level	9.61′	9.61'	

Table 4-25 Filtration System Design Criteria

Richmond Wastewater Treatment Facility 20-Year Evaluation Section 4 – Alternatives Evaluation

Description	Existing Design	Proposed Design	Design Standard	
	53.8 sf/disk	53.8 sf/disk		
Filter Area Provided	107.6 sf/unit	107.6 sf/unit		
	215.2 sf total	215.2 sf total		
	1.43 gpm/sf @ ADF (0.222	1.43 gpm/sf @ ADF (0.222		
	MGD) – one filter unit	MGD) – one filter unit		
	(2 disks)	(2 disks)		
	7.43 gpm/sf @ PHF (1.152	7.43 gpm/sf @ PHF (1.152		
	MGD) – one filter unit	MGD) – one filter unit	ONDE: 2 2E anon /of	
Hydraulic Loading	(2 disks)	(2 disks)	@ADL: 5.25 gpm/st	
Rate	0.72 gpm/sf @ ADF (0.222	0.72 gpm/sf @ ADF (0.222		
	MGD) – two filter units	MGD) – two filter units	(TR-16)	
	(4 disks)	(4 disks)		
	3.72 gpm/sf @ PHF (1.152	3.72 gpm/sf @ PHF (1.152		
	MGD) – two filter units	MGD) – two filter units		
	(4 disks)	(4 disks)		
	0.52 lbs TSS/sf/day @ ADF	0.52 lbs TSS/sf/day @ ADF		
	(0.222 MGD) – one filter unit	(0.222 MGD) – one filter unit		
	(2 disks)	(2 disks)		
	2.68 lbs TSS/sf/day @ PHF	2.68 lbs TSS/sf/day @ PHF		
	(1.152 MGD) – one filter unit	(1.152 MGD) – one filter unit		
Solids Loading Pato ¹	(2 disks)	(2 disks)	< 2.0 lb TSS/cf/day	
Solius Loauling Rate	0.26 lbs TSS/sf/day @ ADF	0.26 lbs TSS/sf/day @ ADF	< 2.0 10 155/51/udy	
	(0.222 MGD) – two filter units	(0.222 MGD) – two filter units		
	(4 disks)	(4 disks)		
	1.34 lbs TSS/sf/day @ PHF	1.34 lbs TSS/sf/day @ PHF		
	(1.152 MGD) – two filter units	(1.152 MGD) – two filter units		
	(4 disks)	(4 disks)		
Sludge Removal Pump				
Quantity	2	2		
Power Draw	3 HP	3 HP		
Motor Drives				
Quantity	2	2		
Power Draw	3 HP	3 HP		

1. Assuming tertiary influent concentration of 30 mg/L TSS.

Description

Filtration System improvements would include the following components:

- Structural assessment of the existing steel tanks to determine in-place tank repairs needed
- Replacement of the following parts for each filter:
 - o Cloth media
 - o Backwash pumps
 - Sludge removal pumps
 - o Drive motors
 - \circ $\,$ Valves and piping $\,$

• Instrumentation and SCADA programing

Cost Estimate

A preliminary opinion of probable construction cost for replacement of the Filtration System improvements is provided in Table 4-26. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Table 4-26 Filtration Sy	vstem Improvem	ents – Construction	Cost Estimate
	ystern improvern		COSt Estimate

Item	Cost ¹
Structural Assessment and Tank Repair	\$40,000
Filter Replacement Parts	\$116,600
Instrumentation & SCADA Programming	\$20,000
Total Capital Cost Subtotal	\$176,600
Contractor Mark-Up ²	\$40,000
Total Construction Cost (rounded) ^{3,4}	\$217,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.9 Ultraviolet Disinfection

Due to the age of the existing ultraviolet (UV) disinfection system, a new replacement UV disinfection system is proposed as part of a comprehensive upgrade. The disinfection system will be sized to treat a peak hourly flow of 1.152 MGD with full redundancy and an influent daily maximum TSS concentration of 10 mg/L. As the existing stainless steel channel is in good condition, the recommendation is to replace the UV modules, instrumentation, and monitoring/control system inside the existing channel located in the existing UV disinfection location in the Filter/UV room in the upper level of the Process Building.

Design Criteria

Design criteria for the UV Disinfection system is presented in Table 4-27 on the following page.

Item Description	Existing Design	Proposed Design	Design Standard
Average Daily Flow (ADF)	0.222 MGD	0.222 MGD	
Peak Hourly Flow (PHF)	1.0 MGD	1.152 MGD	
TSS Concentration	10 mg/L	10 mg/L	< 30 mg/L TSS (TR- 16)
UV Transmittance	-	65%	65% minimum
UV Radiation Wavelength (nm)	-	254 nm	254 nm (TR-16)
UV Dose at PHF	-	30 mJ*sec/cm ²	> 30 mJ*sec/cm ²
Number of LIV Paper	2	2	
Number of UV Banks	(1 duty, 1 stand-by)	(1 duty, 1 stand-by)	
Number of UV Modules (Total)	20 (10/bank)	20 (10/bank)	
Number of Lamps per Module	4	4	
Total Number of UV Lamps	80	80	
Liquid Depth	12.6″	12.6″	

Table 4-27 Ultraviolet Disinfection Design Criteria

Description

The proposed UV disinfection system includes the following components:

- Two (2) UV Banks, each containing:
 - 8 Type 316 stainless steel modules
 - 4 UV low pressure lamps/module
- Monitoring system for indication of UV intensity, lamp age, and alarms
- Remote indication of UV intensity
- Remote indication of low UV intensity alarm
- Maintenance module cleaning rack

Cost Estimate

A preliminary opinion of probable construction cost for a replacement UV disinfection system is provided in Table 4-28. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F on the following page.

Table 4-28 UV Disinfection – Construction Cost Estimate

Item	Cost ¹
UV System Equipment Replacement	\$189,000
Capital Cost Subtotal	\$189,000
Contractor Mark-Up ²	\$41,000
Total Construction Cost (rounded) ^{3,4}	\$230,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.10 Effluent Flow Measurement

Existing effluent flow measurement consists of a 90° v-notch weir, an ultrasonic level detector, and an effluent flow meter. The geometry of the existing 90° v-notch weir is such that at the peak instantaneous flow of 2.3 MGD experienced at the plant on December 18, 2023, the head on the weir exceeded the top of the v-notch resulting in flow going over the top of the weir and inaccurate measurement.

Due to the capacity of the existing weir being exceeded and the poor condition of effluent flow measurement equipment, the proposed effluent flow measurement improvements include a new 90° v-notch weir and a new radar level sensor and associated flow meter for improved effluent flow measurement. The geometry of the 90° v-notch weir would be increased, and the invert elevation of the v-notch would be lowered to allow for measurement of higher flows and allow for more flow over the v-notch without overtopping the top of the weir. Geometrical changes would maintain the existing hydraulic profile at maximum flow.

Design Criteria

Design criteria for the effluent flow measurement is presented in Table 4-29 on the following page.
Item Description	Existing Design	Proposed Design	Design Standard			
Effluent Flow Measurement						
Control Device	90° V-Notch Weir	90° V-Notch Weir	Accurate measurement over full range of design flows (TR-16)			
Invert of V-Notch Weir Elevation	306.13 ft	305.58 ft				
Measurement	Flow Meter, Ultrasonic Level Detector	Flow Meter, Radar Level Sensor				
Flow Meter Capacity	Minimum: 0.03 MGD at 0.2' above v-notch Maximum: 1.616 MGD at 1' above v-notch	Minimum: 0.03 MGD at 0.2' above v-notch Maximum: 3.883 MGD at 1.42' above v-notch	0.2 ft ¹ – 1.12 ft of head			

Table 4-29 Effluent Flow Measurement Design Criteria

Notes:

1. Low measurement range is the minimum flows at the suggested minimum and maximum head level for the control device as per Isco Open Channel Flow Measurement Handbook, 6th Edition.

Description

The proposed effluent flow measurement improvements include the following:

- One (1) new stainless steel 90° v-notch weir plate
- One (1) radar level sensor and flow meter
- SCADA program allowance

Cost Estimate

A preliminary opinion of probable construction cost for a new effluent flow measurement system is provided in Table 4-30 on the following page. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item	Cost ¹
New V-Notch Weir	\$5,000
Radar Level Sensor & Flow Meter	\$5,000
SCADA Program Allowance	\$2,000
Capital Cost	Subtotal \$12,000
Contractor Mark-Up ²	\$3,000
Total Construction Cost (ro	unded) ^{3,4} \$15,000

Table 4-30 Effluent Flow Measurement – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.11 Effluent Pumping & Outfall

Effluent leaves the effluent wetwell through a 12" pipe and flows into manhole MH13 prior to being discharged to the Winooski River through an 18" reinforced concrete outfall pipe. After the flooding event that occurred during Tropical Storm Irene, the outfall was equipped with a check valve and a sump pump was installed in the wetwell to discharge effluent flows to a higher elevation and prevent backing up into the facility. In the recent July 2023 flooding event, emergency use of a sump pump to lift flow to an external higher discharge point was used to prevent flooding of the Filter/UV Room. As previously discussed in Section 3, many of the top of wall elevations in the Filter/UV Building are lower than the 500-year flood elevation and the top of the UV channel is below the 100-year flood elevation.

The proposed effluent pumping modification, as part of the comprehensive upgrade, includes the construction of an effluent pump station adjacent to the Process Building to prevent the Winooski River from backing up and overtopping the UV channel walls during a 100 and 500-year flooding event. Flow from the existing effluent wetwell located inside the building will pass through to a new effluent pump station wetwell constructed outdoors and adjacent to the building. Under normal river levels, effluent flow will continue to flow through to existing manhole MH13 and out the existing 18" outfall. The effluent line from MH13 to MH14 will be provided with a backflow preventer and the wetwell with level detection. In the case of elevated river levels and an increasing level in the wet well, two (2) vertical turbine effluent pumps located in the new effluent pump station wet well will turn on to pump effluent to a higher discharge elevation to flow to the Winooski River. The pumps will be provided with variable frequency drives and a level detection system in the new wetwell to control the speed and output of the effluent pumps and match incoming flow. The new backflow preventer and effluent pumps will prevent the river from backing up into the Process Building and flooding the effluent wet well and UV disinfection system during flooding events. A small enclosure for the pumps and controls will be provided. Top slab of the pump station wetwell will be above the 500-year flood elevation.

Design Criteria

Design criteria for the effluent pumping and outfall is summarized in Table 4-31.

Table 4-31 Effluent Pumping & Outfall – Design Criteria

Parameter	Existing Design	Design Value		
Current Average Daily Flow	0.073MGD	0.073 MGD		
Average Daily Flow	0.222 MGD	0.222 MGD		
Peak Hour Flow	1.152 MGD	1.152 MGD		
Peak Instantaneous Flow	2.3 MGD	2.3 MGD		
Effluent Pumps				
Quantity	1	2		
Capacity	unknown	1080 gpm @ 15.1' TDH		
Туре	Submersible	Vertical Turbine		
Motor	unknown	7.5 HP		

Exhibit

Figure 4-4 presents a conceptual layout of the proposed effluent pump station.



Figure 4-4 Effluent Pump Station Conceptual Layout

Description

The proposed effluent pump station will include:

- New effluent pump station with concrete wet well, approx. 7' x 9' x 15' depth
- Two (2) effluent pumps
 - Two (2) variable speed, vertical turbine pumps, 7.5 HP each
 - Design capacity 1080 gpm @ 15.1' TDH each
- One (1) control panel with integral VFDs
- Level control system
- Climate controlled enclosure, approx. 10' x 9'
- 18" backflow preventer

Cost Estimate

A preliminary opinion of probable construction cost for the effluent pumping and outfall is provided in Table 4-32. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Table 4-32 Effluent Pumping & Outfall – Construction Cost Estimate

Item	Cost ¹
Effluent Pumps	\$169,000
Effluent Pump Station (wet well, enclosure structure, HVAC, electrical)	\$162,200
Control Panel, VFDs, Level Control System	\$58,800
Process Piping & Valves	\$32,000
Capital Cost Subtotal	\$422,000
Contractor Mark-Up ²	\$92,000
Total Construction Cost (rounded) ^{3,4}	\$514,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.12 Septage Receiving Facilities

The Richmond WWTF serves the larger community through septage acceptance. The WWTF would like to receive 50,000 gallons of septage per day (gpd), however due to aging septage receiving equipment and downstream unit treatment processes, i.e. dewatering equipment, septage holding aeration equipment, transfer pumps, etc., the WWTF has had to scale back the amount of septage they can receive to 25,000 gpd. The reliability of the downstream treatment equipment is vital to Richmond's ability to be a regional septage acceptance facility.

The existing septage receiving facilities consist of a septage receiving unit located in the garage area on the upper level of the Process Building. The septage receiving unit is reaching the end of its useful life and was found to be in poor condition with the bearings that have failed, the auger having significant wear, and the unit needing to be manually cleaned out by operators. The existing ventilation system in the garage is not operable and vents have been cut into the adjacent electrical room to provide airflow between the two spaces.

The recommended upgrade for septage receiving involves a new septage receiving unit that incorporates a key card system with a flow meter for recording septage hauler discharges. Screened septage will flow by gravity to the two (2) sludge holding tank located below the floor of the upper level of the Process Building. Recommended upgrades to the sludge and septage holding tanks are discussed in Section 4.13.

In order to address the inoperable ventilation system and airflow between the septage receiving area and the electrical room, a new ventilation system is proposed in order to make the septage receiving room compliant with NFPA 820 hazardous classifications. The septage receiving area in the garage will be separated from the rest of the garage with a newly constructed wall and the vent openings between the electrical room and the garage will be closed.

Design Criteria

Design criteria for the septage receiving facilities is summarized in Table 4-33.

Item Description	Existing Design ¹	Current Loadings	Proposed Design	Design Standard
Septage Flows				
Volume (average)	166,667	400,000	50,000 gpd ²	
volume (average)	gal/month	gal/month ²	250,000 gal/week ⁴	
Historical Daily Max		$97,000,\text{and}^3$		
Septage Received	-	87,000 gpu		
Total Volume of	2 000 000 gal/yr	4,800,000	12 000 000 gal/yr	
Septage	2,000,000 gai/ yi	gal/yr ²	13,000,000 gal/ yi	
Septage Receiving Unit				
Max. Septage	400 gpm		400 gpm	
Hydraulic Capacity	(< 3% solids)		(< 3% solids)	
Max. Hydraulic	2061 gpm		2061 gpm	
Capacity	2001 gpill		2001 gpill	
Tank Sizo	3.25' w x 6' l x		3.25' w x 6' l x	
TATIK SIZE	3.92' h		3.92' h	
Tank Material	Stainless steel		Stainless steel	
Screening Basket Size	31" diameter		31" diameter	
Screening Bar Spacing	1⁄4″		1/4"	

Table 4-33 Septage Receiving Facilities Design Criteria

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Item Description	Existing Design ¹	Current Loadings	Proposed Design	Design Standard
Max. Upstream Water Level	13.75 in		13.75 in	
Max. Headloss	8″		8″	
Screw Conveyor Diameter	10"		10"	
Water Requirements	20 gpm @ 60 psi		20 gpm @ 60 psi	
Inlet Valve	4" pinch valve		4" pinch valve	
Motor Drive	2 HP		2 HP	

Notes:

- 1. Source: Basis of Design, 2003
- 2. Based on response in 2023 VTDEC WWTF Septage Acceptance Questionnaire
- 3. Based on Monthly Operating Report data from January 2018 to February 2023.
- 4. Based on 5 days/week of septage acceptance.

Exhibit

Figure 4-5 presents a conceptual layout of the proposed septage receiving upgrade.



Figure 4-5 Proposed Septage Receiving Upgrade

Description

The proposed septage receiving unit and septage receiving facilities upgrade will include the following:

Septage Receiving Unit

- One (1) Septage Receiving Unit
 - 304 stainless steel construction
 - Tank assembly with vent
 - Fine Screen with 2 hp drive
 - 3-plane screen basket design with rotating rake assembly
 - 4-inch diameter pinch type inlet valve
 - 4-Zone wash system with solenoid valves
 - Ultrasonic level controller with one (1) transducer
 - Screenings bagger
- Control Panel
 - NEMA 4X 304 stainless steel main control panel
 - Allen-Bradley MicroLogix 1400 PLC
 - Variable frequency drive
 - RACS control station
- Management and Accounting System
- Replacement piping and valves

Building Modifications

- Structural Modifications
 - Construction of CMU wall to separate septage receiving facilities from rest of garage
 - Block off the vents that have been cut between the electrical room and septage receiving facilities
- New HVAC system for Garage area and new Septage Room

Cost Estimate

A preliminary opinion of probable construction cost for a new septage receiving unit and septage receiving facilities upgrade is provided in Table 4-34 on the following page. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item	Cost ¹
Septage Receiving Unit	\$399,000
Piping and Valve Replacements	\$50,000
HVAC Replacement for Garage and Septage Room	\$150,000
Structural Modifications for New Septage Room	\$115,000
Capital Cost Subtotal	\$714,000
Contractor Mark-Up ²	\$157,000
Total Construction Cost (rounded) ^{3,4}	\$871,000

Table 4-34 Septage Receiving Facilities – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.13 Septage and Sludge Storage Facilities

As discussed in Section 4.12, the Richmond WWTF's septage receiving facilities rely on the proper functioning of the downstream septage storage facilities and equipment, i.e. holding tanks, aeration equipment, odor control, and transfer pumping.

Septage and sludge storage consist of two (2) aerated holding basins below the floor in the upper level of the Processing Building and one (1) aerobic sludge holding tank. Screen septage discharged from the septage receiving unit enters the two (2) aerated holding basins where it is mixed with waste activated sludge (WAS). Septage and sludge from the two (2) aerated holding basins is then pumped to the aerobic sludge holding tank via sludge transfer pump #1 for processing through the solids handling stream. See Section 4.14 for the discussion on proposed odor control upgrades for the septage and sludge storage facilities and see Section 4.15 for the discussion on dewatering facilities.

As discussed in the condition assessment section of this report, the existing diffused aeration system in the two (2) aerated septage & sludge holding basins experiences clogging from accumulated solids and the tank decant has been disconnected. In the outdoor aerobic sludge holding tank, some diffusers appear to be clogged and need replacement.

Due to the poor condition of the septage and sludge storage aeration systems, two (2) mixing alternatives were developed for the existing septage and sludge storage facilities:

- 1. Conventional coarse bubble aeration system
- 2. Compressed gas mixing system

A third alternative considered the concept of replacing the existing submersible mixers with hyperbolic mixers. Hyperbolic mixers are designed to operate at the bottom of a tank, draw water down, and discharge it in a horizontal direction. However, hyperbolic mixing has historically been unsuccessful in

prior application for sludge mixing, requiring significant equipment replacement and operation and maintenance. Therefore, this alternative was developed and evaluated further.

Due to the age of septage and sludge transfer pump #1, and the lack of redundancy, it is proposed that existing septage and sludge transfer pump #1 is replaced with two (2) new transfer pumps.

Odor Control

There are no working odor control systems currently in place at the WWTF and as the facility is located in a public park and adjacent to a residential neighborhood, a new odor control system is recommended as part of the upgrade to the septage receiving facilities. The recommended upgrade for both alternatives is the implementation of a Vapex odor control system. Vapex odor control systems use ionization technology. Ionization technologies are a group of processes that use oxygen and water vapor in the air to create highly reactive oxygen molecules such as ozone O₃, OH-, O₂-, O₂+, and O-. These technologies use highly reactive molecules to oxidize odorous compounds produced at a WWTF.

The Vapex odor control system would be implemented to dose the enclosed air spaces within the two (2) septage and sludge holding tanks, as well as a newly covered outdoor aerobic sludge holding tank with a vaporized oxidant to treat odors generated. The Vapex odor control unit is relatively small in size and could be housed in the garage area of the Process Building, with the oxidant piped to the target enclosed storage tanks.

Design Criteria

Design description for the septage and sludge holding tanks is summarized in Table 4-35 on the following page, and a design description for the Vapex odor control system is summarized in Table 4-36.

Item Description	Existing Design ¹	Current Loadings	Proposed Design			
Septage						
Septage Volume (average)	166,667 gal/month	400,000 gal/month ²	50,000 gpd ² 250,000 gal/week ⁴			
Historical Daily Max Septage Received	-	87,000 gpd ³				
Total Volume of Septage	2,000,000 gal/yr	4,800,000 gal/yr ²	13,000,000 gal/yr			
Secondary Sludge Production						
Dry Solids Produced from Secondary Process	Not available		1,524 lbs/day			
Percent Solids	Not available		1% ⁵			
WAS from Secondary Process	Not available	2,662 gpd ³	17,393 gpd ⁵			
Total Septage & WAS Storage Needed						
Daily Volume Needed	Not available	22,662 gal	67,393 gal			

Table 4-35 Septage & Sludge Storage Design Criteria

Notes:

- 1. Source: Basis of Design, 2003
- 2. Based on response in 2023 VTDEC WWTF Septage Acceptance Questionnaire
- 3. Based on Monthly Operating Report data from January 2018 to February 2023.
- 4. Based on 5 days/week of septage acceptance.
- 5. Results from Biological Model Results

Table 4-36 Vapex Odor Control System – Design Criteria

Parameter	Design Value
Volumes Treated	
Septage & Sludge Holding Basin #1	531 cf
Septage & Sludge Holding Basin #2	531 cf
Aerobic Holding Basin	862 cf
Odor Control System	
Туре	Ionization Technology
Number of Units	1
Dimensions	54" x 35" x 44"
Atomizing Nozzles per Unit	6
Oxidant Output	≤ 50 lbs/day
Average Water Usage	≤ 8 gal/hr/nozzle
Nozzle Air Output	20 cfm/nozzle
Treatment Coverage Volume	26,000 cf (maximum)
Power Requirements	220 VAC, 23A, 60 Hz

4.13.1. Septage & Sludge Storage Alternative #1: Conventional Diffused Aeration Mixing System

Septage and Sludge Storage Alternative #1 proposes the replacement of the existing diffused aeration system with a coarse bubble diffused aeration system with air supplied from new positive displacement blowers. The coarse bubble diffused aeration system will provide mixing of the contents of each tank. Coarse bubble diffused aeration systems also provide uniform airflow and prevent clogging.

One new replacement blower will be provided at the septage and sludge holding tanks and one new replacement blower for the outdoor aerated sludge holding basin. Redundancy for air supply for the septage and sludge holding tanks exists with an interconnection of a 4" supply line from the aeration tank blowers. New yard air piping will be installed to connect the aeration tank blowers to supply air to the outdoor sludge holding tank to provide redundancy for that system. A new concrete cover with access hatch will be constructed over the outdoor aerobic sludge holding tank to contain odors.

In addition, two (2) new rotary type positive displacement sludge transfer pumps are proposed to replace the single pump in the lower level of the Process Building to transfer septage and sludge to the outdoor aerobic holding tank prior to dewatering.

This alternative also includes covering of the aerobic holding tank and implementation of a Vapex odor control system as previously described in Section 4.13.

Design Criteria

Design criteria for septage and sludge storage Alternative #1 are presented in Table 4-37 on the following page.

Item Description	Existing Design	Proposed Design	Design Standard		
Aerated Septage & Sludge Holding Tank					
Quantity	2	2			
Tank Covering	Covered, Concrete	Covered, Concrete			
Dimensions	21.58' x 14.58'	21.58' x 14.58'			
SWD	10'	10'			
Capacity	23,000 gal (each) 46,000 gal (total)	23,000 gal (each) 46,000 gal (total)			
Aeration System	Fine Bubble, Membrane Type Diffusers	Coarse Bubble Diffusers			
Blower Type	Positive Displacement	Positive Displacement			
Blower Capacity	200 scfm @ 5 psi	200 scfm @ 5 psi			
Mixing Air Requirement (total)	188 scfm	188 scfm	15-20 cfm/1000 ft ³ (TR-16)		
Motor	10 HP	10 HP			
VFD	Yes	Yes			
Aerobic Sludge Holding Basin	1	1	1		
Quantity	1	1			
Tank Covering	Open	Covered			
Dimensions	38' x 8.5'	38' x 8.5'			
SWD	14'	14'			
Capacity	34,000 gal	34,000 gal			
Aeration System	Coarse Bubble Diffusers	Coarse Bubble Diffusers			
Blower Type	Positive Displacement	Positive Displacement			
Blower Capacity	170 scfm @ 7 psi	170 scfm @ 7 psi			
Mixing Air Requirement	140 scfm	140 scfm	15-20 cfm/1000 ft ³ (TR-16)		
Motor	10 HP	10 HP			
VFD	Yes	Yes			
Total Septage & Sludge Storage Ca	pacity		1		
Total Storage Volume Provided	80,000 gal	80,000 gal			
Sludge Transfer Pump #1	Γ	Γ	Γ		
Quantity	1	2			
Туре	Double-Disc, Positive- Displacement	Rotary Lobe, PD			
Capacity	150 gpm @ 18' TDH	150 gpm @ 18' TDH			
Solids Content	Sludge, 1-3%	Sludge, 1-3%			
Motor	7.5 HP	5 HP			
VFD	Yes	Yes			

Table 4-37 Septage & Sludge Storage Alternative #1 Design Criteria

Description

Septage and Sludge Storage Alternative #1 includes the following:

- Coarse Bubble Diffusers (1 grid per tank)
- Positive Displacement Blowers
 - 2 total (1 for septage and sludge holding tanks, 1 for outdoor aerobic sludge holding basin)
 - o VFDs
- Instrumentation, including
 - Air flow meters
 - Pressure transducer level indication with redundant floats for high and low level alarms
 - Magnetic flow meter for sludge transfer
- Sludge transfer pumps
 - Two (1) Rotary Lobe type sludge transfer pumps
- Concrete cover with hatches for the aerobic sludge holding basin for odor containment
- Site Work including
 - Yard air piping and valves to interconnect aeration tank blower to outdoor aerobic sludge holding basin

A Vapex odor control system would include the following:

- One (1) Milli Vapex Unit
- 6 nozzles flexible
- Water tubing
- Oxidant tubing
- Air tubing

Cost Estimate

A preliminary opinion of probable construction cost for Sludge Storage Alternative #1 is provided in Table 4-38 on the following page. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item	Cost ¹
Diffused Aeration Mixing System (equipment, piping & valves, diffusers)	\$65,000
Blowers	\$132,600
Electrical, Instrumentation & Controls	\$70,000
Sludge Transfer Pumps	\$99,400
Aerobic Sludge Holding Tank Cover	\$30,000
Yard Air Piping	\$12,000
Odor Control	\$266,800
Capital Cost Subtotal	\$675,800
Contractor Mark-Up ²	\$149,000
Total Construction Cost (rounded) ^{3,4}	\$825,000

Table 4-38 Septage & Sludge Storage Alternative #1 – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

Advantages/Disadvantages

Advantages

- Conventional method for aerated sludge storage with blowers and diffusers
- Prevents contents from turning anaerobic and releasing ammonia and phosphorus from decaying biomass

Disadvantages

• Uses more energy than compressed gas systems, which translates to high annual operating costs

4.13.2. Septage & Sludge Storage Alternative #2: EnviroMix Compressed Gas Mixing System

Septage and Sludge Storage Alternative #2 is to provide EnviroMix compressed gas mixing systems in the aerated holding tanks and the aerobic sludge holding basin. EnviroMix provides bursts of compressed air which are fired through nozzles located near the tank floor. These bursts of compressed air provide uniform mixing of the tank contents through rapid upward velocity without transferring oxygen to the tank contents. New compressors and receiving tank for the compressed gas mixing system could be housed in the lower level of the process building or in the existing structure over the existing aerobic sludge holding basin. A new concrete cover with access hatch will be constructed over the outdoor aerobic sludge holding tank to contain odors.

In addition, two (2) new rotary type positive displacement sludge transfer pumps are proposed to replace the single pump in the lower level of the Process Building to transfer septage and sludge to the outdoor aerobic holding tank prior to dewatering.

This alternative also includes covering of the aerobic holding tank and implementation of a Vapex odor control system as previously described in Section 4.13.



Figure 4-6 EnviroMix Compressed Gas Mixing System

Design Criteria

Item Description	Existing Design	Proposed Design	Design Standard
Aerated Septage & Sludge Ho	lding Tank		
Quantity	2	2	
Tank Covering	Covered, Concrete	Covered, Concrete	
Dimensions	21.58' x 14.58'	21.58' x 14.58'	
SWD	10'	10'	
Capacity	23,000 gal (each) 46.000 gal (total)	23,000 gal (each) 46.000 gal (total)	

Table 4-39 Septage & Sludge Storage Alternative #2 Design Criteria

Item Description	Item Description Existing Design Proposed Design		Design Standard
Mixing System	Fine Bubble, Membrane Type Diffusers	Compressed Gas Mixing System	
Blower Type	Positive Displacement	Positive Displacement Rotary Lobe Compressor	
Number	1	2 (1 duty, 1 standby)	100% redundancy with largest unit out of service
Blower Capacity	200 scfm @ 5 psi	20 scfm @ 116 psi	
Motor	10 HP	5 HP	
VFD	Yes	Yes	
Receiver Tank		120-gallon	
Aerobic Sludge Holding Basin			
Quantity	1	1	
Tank Covering	Open	Covered	
Dimensions	38' x 8.5'	38' x 8.5'	
SWD	14'	14'	
Capacity	34,000 gal	34,000 gal	
Mixing System	Coarse Bubble Diffusers	Compressed Gas Mixing System	
Blower Type	Positive Displacement	Shared with Septage	
Blower Capacity	170 scfm @ 7 psi	Shared with Septage	
Motor	10 HP	Shared with Septage	
VFD	Yes	Shared with Septage	
Sludge Transfer Pump #1			
Quantity	1	2	
Туре	Double-Disc, Positive- Displacement	Rotary Lobe	
Capacity	150 gpm @ 18' TDH	150 gpm @ 8' TDH	
Solids Content	Sludge, 1-3%	Sludge, 1-3%	
Motor	7.5 HP	5 HP	
VFD	Yes	Yes	

Description

Sludge Storage Alternative #2 includes the following:

- Implementation of an EnviroMix compressed gas mixing system in the two (2) sludge holding tank and the aerobic sludge holding basin.
 - Valve Module Control Panel with electrically actuated valves
 - o 304 Stainless steel nozzles, headers, and header supply piping

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- (2) 3 HP Rotary Screw Compressor (combined with the system for anoxic selectors)
- 120-Gallon Receiver Tank (combined with the system for anoxic selectors)
- Sludge Transfer Pumps
 - Two (1) Rotary Lobe type sludge transfer pumps
 - Instrumentation, including
 - Air flow meters
 - Pressure transducer level indication with redundant floats for high and low level alarms
 - Magnetic flow meter for sludge transfer
- Concrete cover with hatches for the aerobic sludge holding basin for odor containment
- Site Work including
 - Yard compressed gas piping

A Vapex odor control system would include the following:

- One (1) Milli Vapex Unit
- 6 nozzles flexible
- Water tubing
- Oxidant tubing
- Air tubing

Figure 4-7 presents a layout of the EnviroMix compressed gas mixing system within the sludge storage tanks.



Figure 4-7 EnviroMix Layout for Septage & Sludge Holding Tanks and Aerobic Sludge Holding Tank

Cost Estimate

A preliminary opinion of probable construction cost for Septage and Sludge Storage Alternative #2 is provided in Table 4-40. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Table 4-40 Septage & Sludge Storage Alternative #2 – Construction Cost Estimate

Item	Cost ¹
Compressed Gas Mixing System (equipment, piping & valves, nozzles)	\$215,800
Electrical, Instrumentation & Controls	\$50,000
Sludge Transfer Pumps	\$99,400
Aerobic Sludge Holding Tank Cover	\$30,000
Yard Air Piping	\$10,500
Odor Control	\$266,800
Capital Cost Subtotal	\$672,500
Contractor Mark-Up ²	\$148,000
Total Construction Cost (rounded) ^{3,4}	\$821,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

Advantages/Disadvantages

Advantages

- Provides over 50% energy savings over mechanical mixing or diffused aeration
- EnviroMix provides unlimited turn-down capability
- EnviroMix has zero in-tank maintenance with no mechanical or electrical components in wastewater

Disadvantages

• Less conventional methodology and equipment to maintain

4.13.3. Comparison of Construction Cost Estimates of Sludge Storage Alternatives

Detailed opinions of probable construction cost for each of the sludge storage alternatives are provided in Appendix F. Table 4-41 on the following page provides a summary of the opinion of probable construction cost for each alternative that was considered.

ltem	Alternative 1 – Conventional Diffused Aeration System ¹	Alternative 2 – EnviroMix Compressed Gas Mixing System ¹
Mixing System	\$65,000	\$215,800
Blowers	\$132,600	
Electrical, Instrumentation & Controls	\$70,000	\$50,000
Sludge Transfer Pumps	\$99,400	\$99,400
Aerobic Sludge Holding Tank Cover	\$30,000	\$30,000
Yard Air Piping	\$12,000	\$10,500
Odor Control – Vapex System	\$266,800	\$266,800
Capital Cost Subtotal	\$675,800	\$672,500
Contractor Mark-Up ²	\$149,000	\$148,000
Total Construction Cost (rounded) ^{3,4}	\$825,000	\$821,000

 Table 4-41 Summary of Septage & Sludge Storage Alternatives – Preliminary Opinion of Probable

 Capital Cost

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.13.4. Comparison of Life Cycle Cost of Septage & Sludge Storage Alternatives

Life Cycle Costs

Life cycle costs were prepared for each Septage & Sludge Storage Alternative. Life cycle costs include estimates of annual power consumption, replacement parts, and operation and maintenance time. Detailed information for each alternative is included in Appendix G.

The following assumptions and variables were considered in the development of life cycle costs:

- Estimated annual electrical cost of operation based on equipment operating horsepower and operating hours and an average utility electric rate of \$0.149/kW-hr taking into account on-peak and off-peak rates and \$16.7/day customer service charge as per Richmond WWTF's Green Mountain Power bill including customer charge, total energy efficiency charge, extreme storm restoration fund, electric assistance program fee, and energy/major storm adjustor.
- Estimated labor requirements for operation and maintenance at an assumed labor rate of \$50.00/hr.

Table 4-42 summarizes the life cycle costs for each Septage & Sludge Storage alternative.

	Alt 1. Conventional Diffused Aeration System	Alt 2. Compressed Gas Mixing
Annual Power Cost	\$21,500	\$10,000
Annual Equipment Replacement Costs	\$1,200	\$2,000
Annual Labor Cost	\$18,200	\$200
Total Annual O&M Cost (rounded)	\$41,000	\$12,000

Table 4-42 Septage & Sludge Storage Alternatives – Opinions of Life Cycle Cost

Present Worth Analysis

A present worth analysis was performed to further compare the various alternatives and the results are summarized below.

The following assumptions and variables were considered in the present worth analysis:

- Planning period of 20 years
- Escalation rate of 3% annually
- Discount rate of 2.5% (based on EPA Fiscal Year 2023)

A summary of present worth analysis for the Septage & Sludge Storage Alternatives is presented in Table 4-43.

Table 4-43 Present Worth Analysis of Septage & Sludge Storage Alternatives

	Alt 1. Conventional Diffused Aeration System	Alt 2. Compressed Gas Mixing
Total Project Cost of Alternative	\$825,000	\$821,000
Annual O&M Cost of Alternative	\$41,000	\$12,000
Present Worth of Alternative	\$1,705,000	\$1,079,000

Notes:

3. Total project costs are inclusive of construction costs and contractor mark-up, but od not include contingency, engineering services, legal and administrative costs.

4. ENR Construction Cost Index = 13514.76 (December 2023)

As shown in Table 4-16, Alternative 1 is the highest present worth value based on the highest project cost and highest O&M cost. Alternative 2 has the lowest present worth based on the lowest highest project cost and lowest O&M cost, approved 36.7% less than the present worth of Alternative 1.

4.14 Dewatering Facilities

Mixed septage and sludge from the aerobic sludge holding basin is transported to a 3,000-gallon fiberglass sludge day tank in the Dewatering Building via a transfer pump located in the Dewatering

Building. Sludge from the day tank is then fed to the dewatering equipment via a sludge feed pump. Dewatering equipment consists of a flocculator where sludge is mixed with polymer from the polymer feed system and a two-channel rotary press, all located in the Dewatering Building. Currently, only one (1) channel in the two (2) channel rotary press is operational. The dewatering equipment has an estimated 45,000 hours of operation since installation and has not had significant maintenance of replacement parts since installation. A manufacturer's report was completed on the rotary press after an inspection on the condition of the existing rotary press was performed. As of late October 2023, the Town of Richmond has moved forward with the purchase of two (2) new dewatering channels to replace the existing channels, one (1) flocculator, and a new control panel with equipment installation assistance and coordination.

While the purchase of two (2) new dewatering channels will help solve the immediate dewatering operational issues that have been experienced and will ensure Richmond's ability to continue offering septage receiving services, the Town has expressed interest in accepting up to 50,000 gallons per day of septage to process through their solids handling facilities. In order to make this feasible, an upgrade to the Dewatering Facilities will be required with a properly sized rotary press to process the sludge generated at the WWTF and the desired amount of septage received. The proposed dewatering facilities upgrade will include a rotary press that is equipped to handle 4-channels. The new rotary press would come with two (2) new channels, with the intention of placing the recently purchased two (2) channels on the new rotary press. The recently purchased control panel will be reused for the new rotary press. Overall, this would allow the Richmond WWTF to accept increased volumes of septage and make use of recently purchased equipment.

The recommended dewatering facilities includes an expansion of the existing Dewatering Building to house the larger rotary press as well as a new polymer feed system, new coagulant feed system to provide coagulant dosing to the rotary press pressate stream, new sludge day tank mixer and level detection system, new sludge transfer and feed pumps, new dewatered sludge cake conveyor system, new electrical room, and new HVAC system. The Dewatering Building extension will also provide storage area and a bay to park the skid steer. The storage area provided in the Dewatering Building extension will replace the area in the old Storage Garage that is used for storage currently. A new interior wall will be incorporated inside the existing building to separate the sludge trailer area from the dewatering equipment area.

Design Criteria

Design criteria for the dewatering facilities is presented in Table 4-44 on the following page.

Item Description	Existing Design	Proposed Design	Design Standard
Sludge Characteristics			
Solids Feed ¹	26,445 gpd	58,300 gpd (WAS: 8,300 gpd Septage: 50,000 gpd)	
Solids Concentration ²	WAS: 9,300 mg TSS/L Septage: 1,760 mg TSS/L	WAS: 25,000 mg TS/L ⁶ Septage: 8,000 mg TS/L ⁶	
Average Solids Feed % ³	0.26%	1.0% TS	
Sludge Disposal	98 wet tons/month ¹ 28.6 dry tons/month ³	52 dry tons/month ⁷	
Rotary Press	Γ	I	
Manufacturer/Model	Fournier Industries Inc. 2-900/2000CV	Fournier Industries Inc. 4-900/4000CV	
Туре	Rotary Press with Flocculator	Rotary Press with Flocculator	
Number of Units	1	1	
Channels	2	4	
Channel Diameter	36"	36"	
Motor	7.5 HP	5 HP	
Through-put ⁴	100 dry Ibs/hr/channel	110 dry lbs/hr/channel	
Dewatered Sludge Cake Average Solids % ⁵	29.2%	29.2% ⁷	
Operational Hours		16 hours/day 5 days/week	
Expected Maximum Filtrate Flow Rate		17 gpm	
Expected Solids Capture ⁴		90%	
Sludge Transfer Pumps (Fr	om Digester to Day Tanl	k)	
Quantity	1	2	
Туре	Double Disc, Positive Displacement	Rotary Lobe, Positive Displacement	
Manufacturer	Penn Valley Pump Co. Inc.	Borger	
Solids Content	1-3%	1-3%	
Capacity	50 gpm	50 gpm	
Max TDH	25.5 ft	25.5 ft	
Min TDH	11.5 ft	11.5 ft	
Max Suction Lift	19 ft	19 ft	
Min Suction Lift	5 ft	5 ft	
Motor	5 HP	5 HP	
VFD	Yes	Yes	

Table 4-44 – Dewatering Facilities – Design Criteria

Richmond Wastewater Treatment Facility 20-Year Evaluation Section 4 – Alternatives Evaluation

Item Description	Existing Design	Proposed Design	Design Standard
Sludge Day Tank			
Quantity	1	1	
Capacity	3,000 gal	3,000 gal	
Туре	Fiberglass Tank	Fiberglass Tank	
Mixer	Shaft impeller	Shaft impeller	
Sludge Feed Pumps (From	Day Tank to Rotary Pres	ss)	
Quantity	1	2	
Туре	Double Disc, Positive	Rotary Lobe, Positive	
	Displacement	Displacement	
Manufacturar	Penn Valley Pump	Dorgor	
Manufacturer	Co. Inc.	borger	
Solids Content	1-3%	1-3%	
Flow Rate	0-60 gpm	0-60 gpm	
Max TDH	20 ft	20 ft	
Min TDH	0 ft	0 ft	
Motor	3 HP	5 HP	
VFD	Yes	Yes	
Control	Rotary Press PLC	Rotary Press PLC	
Polymer Feed System			
Туре	Liquid polymer activation, dilution, and feed system including mixing chamber and feed	Liquid polymer activation, dilution, and feed system including mixing chamber and feed pump	
Chemical Feed Pump Type	Positive Displacement, Diaphragm Type	Peristaltic Type	
Number of Pumps	1	1	
Metering Pump Flow	0.4 – 8.0 gph	0.4 – 8.0 gph	
Dilution Water Flow	120 to 1,200 gph	120 to 1,200 gph	
Application Points	Sludge Flocculator	Sludge Flocculator	
Polymer Storage			
Polymer Storage	55-gallon drums	Two (2) 250-gallon totes	
Spill Containment Vol.		625 gallons	125% of storage tank volume (TR-16)
Coagulant Storage for Dev	vatering Pressate		
Coagulant Storage		Two (2) 250-gallon totes	
Spill Containment Vol.		625 gallons	125% of storage tank volume (TR-16)
Coagulant Feed for Dewat	ering Pressate		
Pump Quantity		2 (1 duty, 1 stand-by)	Minimum of 2 (1 duty, 1 stand-by) (TR-16)
Dosing Points		Rotary Press Pressate	

Notes:

- 1. Average is from historical operating data from January 2018-February 2023.
- 2. Concentration is from 9/21/2022 sampling event.
- 3. Calculated from historical operating data and sampling event.
- 4. From Rotary Press Basis of Design
- 5. Cake solids is average of five dewatered sludge testing results from 2022.
- 6. Further testing recommended.
- 7. Results anticipated based on similar sludge, but to be confirmed.

Exhibit

Figure 4-8 on the next page presents a conceptual layout of the proposed dewatering facilities upgrade with an expanded Dewatering Building.

Richmond Wastewater Treatment Facility 20-Year Evaluation Section 4 – Alternatives Evaluation



Figure 4-8 Proposed Dewatering Facilities Upgrade

Description

Proposed upgrades to the Dewatering Facilities will include the following:

Dewatering Building:

- Approx. 32' building expansion (1,258 ft²)
- New Electrical/Control Room
- New internal block wall to separate sludge trailer area from dewatering area
- New electrical
- New HVAC System
- New plumbing with hose bibbs and slop sink
- Emergency eye wash and shower
- New gas detection system

Dewatering Equipment:

- New 4-channel rotary press
 - Two (2) new channels
 - Reuse two (2) existing channels
- Two (2) new sludge transfer pumps
 - o VFDs
 - Magnetic flow meter
- Relocated 3000-gallon Sludge Day Tank
 - New shaft impeller mixer
 - New level detection
- Two (2) new sludge feed pump
 - o VFDs
 - Magnetic flow meter
- New shaftless screw conveyor system

Polymer Feed System:

- Peristaltic dosing pump
- Static mixing chamber
- Water pressure regulator
- Control panel
- 250-gallon polymer totes
- Containment area

Coagulant Feed System:

- Chemical feed pumps for dewatering pressate
 - o Skid-mounted duplex pump chemical feed system
- Injection quill
- Containment area

Cost Estimate

A preliminary opinion of probable construction cost for the proposed new Dewatering Facilities is provided in Table 4-45. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Table 4-45 New Dewalering Facilities – Construction Cost Estimate	Table 4-45	New Dewatering	Facilities –	Construction Cost Estimate
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Item	Cost ¹
Dewatering Building Expansion & Upgrade	\$1,061,700
Dewatering Press (4-Channel Unit with 2 new channels & 2 reused channels)	\$279,300
Sludge Transfer and Feed Pumps	\$90,600
Polymer Feed System	\$51,200
Sludge Cake Conveyors	\$116,400
Sludge Day Tank Mixer	\$16,300
Coagulant Feed System	\$35,100
Process Piping & Valves	\$100,000
Capital Cost Subtotal	\$1,750,600
Contractor Mark-Up ²	\$386,000
Total Construction Cost (rounded) ^{3,4}	\$2,137,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.15 Operations Building

Description

The existing Operations Building was built in 1972, with modifications made to it during the 2005 upgrade. Recommended upgrades to the existing Operations Building include replacement of the existing boiler, HVAC systems, and laboratory refrigerator. Renovations to the existing building include replacement of doors and windows with energy efficient windows and doors, as well as upgrades to the existing plumbing and repainting and finishes.

The Town of Richmond has expressed interest in an effluent heat recovery system. An effluent heat recovery system presents an opportunity to install a high efficiency, unique system that aims to significantly reduce the use of fossil fuels on the site. The system is unconventional in that it looks to use the processed wastewater as a location to both reject to and take heat from. In summer months, the effluent is expected to provide unlimited heat rejection. In winter months, additional information would need to be provided to determine the minimum water temperature. Any water above 60 deg F would provide a "free heat" source for the system. Water temperatures below 60 deg F would need to be

supplemented by a hot water boiler. The effluent is connected to the mechanical system through a heat exchanger that would send a water source heat pump loop throughout the site. The loop could serve all the buildings on site, and each building would contain heat pumps sized to serve each space.

Currently, there is no method to hoist the influent pumps in the basement of the Operations Building up. The recommended project will include installation of a hatch in the floor of the laboratory space to hoist the influent pumps through.

Cost Estimate

A preliminary opinion of probable construction cost for proposed improvements to the existing Operations Building is provided in Table 4-46. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Table 4-46 Proposed	Operations	Building Improvements	 Construction 	Cost Estimate
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Item	Cost ¹
Floor Access Door for Influent Pumps	\$32,500
New Energy Efficient Windows and Doors	\$65,000
New Laboratory Equipment Allowance	\$15,000
Plumbing and HVAC Upgrades	\$130,000
New Effluent Heat Recovery System Allowance	\$300,000
New Paint and Finishes	\$20,000
Capital Cost Subtotal	\$562,500
Contractor Mark-Up ²	\$123,000
Total Construction Cost (rounded) ^{3,4}	\$686,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.16 Site

Description

Flood Mitigation:

The ground elevation around the Process Building, which houses the chemical storage, UV disinfection, and filter units is at elevation **313.50 feet**, which is above the 100-year flood elevation, but below the 500-year flood elevation. The recommendation is to install flood barriers at all exterior doors at the Process Building to protect against the 500-year flood of **313.77 ft**.

WWTF Electrical System & Stand-by Generator:

A detailed electrical and instrumentation review by an electrical engineer was not included in the scope of the planning study work. While the operators indicated that the existing 150-kW diesel engine driven

Richmond Wastewater Treatment Facility 20-Year Evaluation Section 4 – Alternatives Evaluation

generator operates well, it is noted that there is a need for an electrical assessment for the Richmond WWTF. The recommendation is to hire an electrical engineer to perform an assessment of the WWTF's existing electrical system and standby generator to determine its existing condition and ability to meet the demands of the facility for the next 20 years.

PLC/SCADA System:

The recommended upgrade project will include upgrades and modifications to the existing SCADA system to integrate new equipment and control logic for unit processes.

Plant Water System:

Currently the WWTF uses Town water throughout the facility for non-potable water demands. The operators expressed a desire to implement a plant water system to reduce the Town water consumption and cost. A new plant water system would use disinfected effluent for non-potable water services for screen wash water, septage receiving unit wash water, polymer dilution, and dewatering wash water. The recommended upgrade includes a package plant water duplex pump skid to be installed in the basement of the Process Building adjacent to the effluent sump.

Site Fence:

The facility is surrounded by a security fence with access gates at the driveway entrance and behind the Storage Garage. Feedback from the trucking company that delivers the sludge trailer indicates the need for a wider gate opening at the main entrance. The recommended upgrade will include a wider site entrance and gate.

Pavement:

Site restoration of the proposed upgrade will include cold planing of the existing pavement and placement of 3" of new pavement.

Cost Estimate

A preliminary opinion of probable construction cost for proposed site improvements is provided in Table 4-47 on the following page. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item	Cost ¹
Flood Mitigation – Flood Doors	\$22,100
Plant Water System	\$138,000
Entrance Gate Replacement and Modifications to Fence	\$25,000
Pavement Restoration	\$56,800
Facility-Wide Electrical Assessment	\$25,000
Yard Process Piping & Electrical/Instrumentation Conduit & Wiring Allowance	\$100,000
SCADA Programming Allowance	\$50,000
New generator	\$52,000
Capital Cost Subtotal	\$468,900
Contractor Mark-Up ²	\$102,000
Total Construction Cost (rounded) ^{3,4}	\$571,000

Table 4-47 Proposed Site Improvements – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.17 Collection System Improvements

Description

Improvements to the collection system include the following:

- The following 15 manholes were identified for cleaning
 - o BK 01
 - o BR 02
 - o BR 06
 - CH 01
 - CH 05
 - ML 01
 - NM 05
 - o NM 08
 - RR 02
 - o SM 01
 - o TD 06
 - o TD 08
 - o TH 06
 - o VG 01
 - WW 07
- The following 6 manholes were identified for maintenance:
 - ES 01 Ring and frame replacement

- NM 04 Replace bricks in riser
- NM 05 Replace bricks in riser
- PL 02 Ring and frame replacement
- TD 06 Ring and frame replacement
- VG 02 Reset frame and cover

Cost Estimate

A preliminary opinion of probable construction cost for proposed site improvements is provided in Table 4-48. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Table 4-48 Proposed Collection System Improvements – Construction Cost Estimat	е
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Item	Cost ¹
Manhole Cleaning	\$37,500
Manhole Ring and Frame Replacement	\$10,400
Manhole Refurbishment	\$5,200
Capital Cost Subtotal	\$53,100
Contractor Mark-Up ²	\$12,000
Total Construction Cost (rounded) ^{3,4}	\$65,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.18 Bridge Street Pump Station

Description

Due to the age and the critical nature of the Bridge Street Pump Station and the damage sustain to the force main bridge crossing due the December 2023 flooding event, the recommended upgrade is full pump station replacement with a submersible type pump station with separated valve vault. It is also recommended to replace the existing force main under the bridge with a new insulated force main that is located at a higher elevation under the bridge deck. Design criteria for the new pump station and force main bridge crossing will be determined during final engineering.

Cost Estimate

A preliminary opinion of probable construction cost for proposed site improvements is provided in Table 4-49 on the following page. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item	Cost ¹
Site	\$122,000
Valve Vault	\$124,000
Wet Well and Pumping Equipment	\$379,000
Electrical, includes Stand-by Generator	\$160,000
Bridge Force Main Crossing	\$90,000
Capital Cost Subtotal	\$875,000
Contractor Mark-Up ²	\$193,000
Total Construction Cost (rounded) ^{3,4}	\$1,068,000

Table 4-49 Bridge Street Pump Station and Force Main Replacement – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

4.19 Summary of Recommended Upgrade Construction Costs

Total construction costs are grouped into the following four upgrade categories and summarized in Table 4-50 on the following page:

- 1. Collection System Upgrades
- 2. Flood Mitigation Upgrades
- 3. WWTF Upgrade
- 4. Septage Receiving Facilities Upgrade

Table 4-50 Summary of Upgrade Construction Co	sts
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Item	Cost ¹
Collection System Upgrades	
Collection System Rehabilitation and Maintenance	\$65,000
Collection System Total Construction Cost (rounded) ^{2,3,4}	\$65,000
Flood Mitigation Upgrades	
Bridge Street Pump Station Upgrade	\$1,068,000
Effluent Pump Station	\$514,000
Flood Doors	\$26,000
Flood Mitigation Upgrades Total Construction Cost (rounded) ^{2,3,4}	\$1,608,000
WWTF Upgrade	
Influent Pumping Upgrade	\$488,000
Headworks Upgrade Alt. 2 Multirake Screen	\$1,654,000
Anoxic Selector Upgrade Alt. 2 - Compressed Gas Mixing	\$72,000
Biological Process Upgrade	\$462,000
RAS System Upgrade	\$57,000
WAS Pumping System Upgrade	\$95,000
Filtration System Upgrade	\$217,000
UV System Upgrade	\$230,000
Effluent Flow Measurement	\$15,000
Operations Building	\$686,000
Site	\$545,000
WWTF Upgrades Total Construction Cost (rounded) ^{2,3,4}	\$4,521,000
Septage Receiving Facilities Upgrade	¢074.000
Septage Receiving Unit Upgrade	\$871,000
Septage & Sludge Storage Alt. 2 - Compressed Gas Mixing, Transfer Pumps,	\$821,000
Odor Control	*****
Dewatering Facilities Upgrade	\$2,137,000
Septage Receiving Facilities Upgrades Total Construction Cost (rounded) ^{2,3,4}	\$3,829,000
	¢10.022.000
Construction Cost Subtotal	\$10,023,000
Engineering & Construction Contingency @30%	\$3,007,000
Total Construction Cost ³	\$13,030,000

Notes:

1. ENR Construction Cost Index = 13514.76 (December 2023)

- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Subtotal Construction Costs do not include contingency.

5. Proposed Project

5.1 Introduction

To address age-related needs, redundancy requirements, peak hydraulic flows, flood resiliency, and provide the Town of Richmond with a reliable and robust treatment process, upgrades to the existing facility are recommended. Section 5 summarizes the design criteria for each unit process, buildings, environmental impacts, land requirements, construction sequencing, and provides recommended site modifications to accommodate new structures. A proposed site plan is presented in Appendix H.

5.2 Design Criteria

The original influent design criteria and proposed influent design criteria for liquid treatment processes are presented in Table 5-1. Historical operating data is discussed in Section 2.5. Proposed design criteria were determined by the development of a biological model of the existing biological treatment tankage at design flow, as discussed in Section 3.6.

Parameter	Original Design ^{1,2}	Current Conditions ^{3,4}	Proposed Design Criteria
Average Daily Flow	0.222 MGD	0.073	0.222 MGD
Peak Hour Flow	1.152 MGD	-	1.152 MGD
Peaking Instantaneous	-	2.3 MGD ⁵	2.3 MGD ⁵
Biochemical Oxygen Demand	324 mg/L	670 mg/L	324 mg/L
	600 lbs/day	411 lbs/day	600 lbs/day ⁶
Total Suspended Solids	270 mg/L	932 mg/L	421 mg/L
Total Suspended Solids	500 lbs/day	573 lbs/day	780 lbs/day ⁶
Total Phosphorus	10 mg/L	19.5 mg/L	19.5 mg/L
Total Nitrogen	-	-	-
Temperature (min/avg/max)	10/_/20°C	4/15/26°C	4/15/26°C

Table 5-1 Richmond	WWTE Pron	osed Influent	Design Criteria
Table J-T Michinolia	wwwii Flop	Joseu minuent	Design Citteria

Notes:

- 1. Source: Basis of Design, 2003
- 2. Original design criteria BOD and TSS concentration are back calculated using design loads and design ADF. Original design criteria TP load is back calculated using design concentration and design ADF.
- 3. Based on Monthly Operating Report data from January 2018 to February 2023. Influent sampling is taken at the influent channel of the wet well, upstream of RAS and pressate side streams. Samples can, at times, include return activated sludge (RAS) & pressate, when the wet well is used for flow equalization and an isolated influent sample is not possible.
- 4. Historical BOD and TSS loads are back calculated using historical average flows and concentrations.
- 5. Peak instantaneous flow defined as the peak instantaneous flow experienced during the 12/18/2023 rain event.

6. Proposed design BOD and TSS loads determined from a biological model at average daily flow.

Effluent design criteria for the Richmond WWTF is based on the existing NPDES permit and is provided in Table 5-2. The existing NPDES permit expires on December 31, 2025, and no changes to the existing effluent limitations are anticipated.

Parameter	Original Design Criteria ¹	Proposed Design Criteria ²
Flow (Annual Average)	0.222 MGD	0.222 MGD
BOD (Monthly Average)	30 mg/L	30 mg/L
TSS (Monthly Average)	30 mg/L	30 mg/L
Total Phosphorus (Monthly Average)	0.8 mg/L	0.8 mg/L
Total Phosphorus (Annual Load)	134 lbs/year	134 lbs/year
Total Nitrogen (Annual Average)	Monitor Only	Monitor Only
Total Kjeldahl Nitrogen (TKN) (Daily Maximum)	Monitor Only	Monitor Only
Nitrate/Nitrite Nitrogen (NOx) (Daily Maximum)	Monitor Only	Monitor Only
Settleable Solids (Instantaneous Maximum)	1.0 mL/L	1.0 mL/L
E. coli (Instantaneous Maximum)	77 CFU/100 ml	77 CFU/100 mL
рН	6.5-8.5 S.U.	6.5-8.5 S.U.

Table 5-2 Richmond WWTF NPDES Discharge Limitations

Notes:

- 1. Source: Richmond WWTF current NPDES Discharge Permit No. 3-1173, effective date January 1, 2021.
- 2. Proposed Effluent Design Criteria is from the WWTF's NPDES Discharge Permit No. 3-1173, effective date January 1, 2021.

5.3 Hydraulic Profile

A proposed hydraulic profile based on the design peak instantaneous flow of 2.3 MGD has been prepared and is presented in Appendix I. Specific modifications to individual unit processes will be discussed under their respective sections under Section 5.6 – Proposed Project.

5.4 Environmental Impacts

The Richmond WWTF is located within a FEMA Special Flood Hazard Area and while the site sits above the 100-year flood elevation, some buildings are located below the 500-year flood elevation. There are no classified wetlands on the property and the property is not located in a regulatory floodway. The majority of the proposed modifications will take place within existing tankage, buildings, and impervious areas, with the exception of the construction of a new effluent pump station and expansion of the existing Dewatering Building.

5.5 Construction Sequencing

Richmond WWTF must maintain liquid treatment functions, solids storage functions, and receive chemical deliveries throughout construction. Where applicable, costs for bypass pumping, temporary treatment, and other requirements for maintaining facility operations during construction are accounted for in the construction cost.

5.6 Proposed Project

The recommended alternatives and proposed age-related upgrades for the various project components for the Richmond WWTF Upgrade are presented in this section. The project will consist of the construction of a new influent pumps, new screening facility, new grit removal equipment, new anoxic selector mixing technology, biological process upgrades to replace aging equipment and provide sufficient aeration, upgrades to the filter units, new UV disinfection system, new effluent pump station, new septage receiving unit, new septage & sludge holding mixing technology, an addition to the existing Dewatering Building, upgrades to the existing Operations and Process Buildings, and overall site improvements.

5.6.1. Influent Pumping

The Richmond WWTF currently operates with one influent pump that was purchased in used-condition to replace a failed influent pump that was original to the facility. This pump was purchased with the intention of being used as a temporary, emergency pump. Their second influent pump is original to the facility and has a leaking seal which sprays across the room when put into use. The second influent pump is past the end of its useful life. Recommended influent pumping improvements include a new triplex influent pump set-up.

Design Criteria

A summary of the existing and proposed design criteria is presented in Table 5-3 on the following page.
Item Description	Existing Design	Proposed Design	Design Standard
Flows			
Current ADF	0.073 MGD (50 gpm)	0.073 MGD (50 gpm)	
ADF	0.222 MGD (154 gpm)	0.222 MGD (154 gpm)	
PHF	1.152 MGD (800 gpm)	1.152 MGD (800 gpm)	
Peak Instantaneous Flow		2.3 MGD ¹ (1,597 gpm)	
Influent Pumps			
Number of Units	2	3	
Туре	Centrifugal, Vertical Mounted, Flooded Suction	Dry-pit Submersible	
Capacity, each	800 gpm @ 65' TDH	800 gpm @ 70' TDH	
Total Pump Capacity	1.152 (MGD) (800 gpm)	2.3 MGD (1,597 gpm)	Peak Q w/ one unit out of service
Motor Size	#1: 25 HP #2: 40 HP	approx. 40 HP (pending final design)	
Variable Speed Driven	Yes	Yes	

Table 5-3 Influent Pumping Design Criteria

Notes:

1. Peak instantaneous flow defined as the peak instantaneous flow experienced during the 12/18/2023 rain event.

Description

The recommended influent pumping improvements include the following:

- Three (3) influent pumps
 - Dry-pit, non-clog, submersible
 - Each rated for 800 gpm
- Variable frequency drives (VFDs)
- Control panel
- New level control system for wet well
- Replacement of suction and discharge piping and valves
- New discharge magnetic flow meter

Exhibit

Figure 5-1 presents a conceptual layout of the proposed influent pump layout.

Richmond Wastewater Treatment Facility 20-Year Evaluation Section 6 – Conclusions and Recommendations



Figure 5-1 Influent Pump Conceptual Layout

Non-Monetary Considerations

- Construction sequencing constraints are required to ensure wastewater can continue to be pumped to the biological process during construction.
- The proposed influent pumping project would bring the Richmond WWTF into compliance with pumping the peak flow through the plant with the largest unit out of service.

Cost Estimate

A preliminary opinion of probable construction cost for improvements to influent pumping is provided in Table 5-4 on the following page. A detailed breakdown of this opinion of probable cost is provided in Appendix F.

Table 5-4 Influent Pumping – Construction Cost Estimate

Item	Cost ¹
Influent Pumps (3 17 HP dry-pit submersible)	\$221,000
Piping and Valves	\$50,000
VFDs, Control Panel	\$94,400
Level Control System	\$15,000
Magnetic Flow Meter	\$10,000
Instrumentation & Control and Integration Allowance	\$10,000
Total Capital Cost Subtotal	\$400,400
Contractor Mark-Up ²	\$88,000
Total Construction Cost ^{3,4}	\$488,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.2. Headworks Facility

The existing Richmond WWTF Headworks facility does not provide adequate screening and grit removal, and the existing equipment is reaching the end of its useful life. Recommended improvements to the Headworks include implementing a new multi-rake screen in the existing influent wet well, building a new Screening Building above the existing wet well, and replacing the existing screening & grit removal unit with a new grit removal system.

Design Criteria

A summary of the existing and proposed design criteria is presented in Table 5-5 on the following page.

	Standard
Flow	
Design ADF 0.222 MGD 0.222 MGD	
Design PHF 1.152 MGD 1.152 MGD	
Peak Instantaneous 2.3 MGD ¹	
Wetwell Influent Channel	
Dimensione 1'-7" wide 2'-0" wide	
1'-7" deep 2'-0" deep	
Screening	
Type Rotary – Mechanical, Fine Screen Multi-Rake Screen	
3/8" bar spacings0.1Dimensions½" screen openings spacing¼" bar spacings31" screen basket diameter31" screen basket diameter	.25-1.5 inch (TR-16)
Screen Angle 80-degree	
Bar Rack Screening Width 1'-2"	
Total invert elevation to operating floor level22'-0"	
Discharge Height 4'-6"	
Peak Capacity 2.97 MGD 1.5 MGD	
Motor Drive 2 HP 3 HP	
Wash Press	
Diameter of Hollow Shaft 8.5"	
Motor Drive 5 HP	
Water requirements 5-20 gpm @ 60 psi 19 gpm @ 35 psi	
Grit Removal	
Type of SystemLakeside Complete Plant – Aerated Grit Removal Chamber w/ Grit TransferLakeside Aerated Grit Removal Chamber w/ Grit Transfer	
Screw & Dewatering Screw Dewatering Screw	
Iransfer Screw 8" dia. w/ 1 HP drive 8" dia. w/ 1 HP drive	
Dewatering Screw 8" dia. W/ 2 HP drive 8" dia. W/ 2 HP drive	
Grit Blower Z HP Z HP Diffusers CC Course bubble CC Course bubble	
Diffusers SS Coarse bubble SS Coarse bubble	
Chamber Dimensions 26' X 3.5' 26' X 3.5'	
SWD 9.1/ 9.1/ Material Staiplass Staal Staiplass Staal	

Table 5-5 Headworks Design Criteria

Description

The following items are included in the proposed Headworks upgrade:

New Screening Building

- Multi-rake Screen
 - 304 stainless steel construction
 - 80-degree setting angle
 - Clear bar spacing: 1/4"
 - Discharge height: 4'-6"
 - Explosion-proof 3.0 HP motor
 - Stainless steel drive chains
 - Static guide rail bearing lower bar rack engagement system
 - Screen side frames recessed in the channel walls giving a 1'-2" effective bar rack width
 - Stainless steel covers above floor level
- Wash Press
 - 304 Stainless steel construction
 - Spray wash system with explosion proof solenoid valves
 - o Washing Press inlet hopper
 - Washing Press deep drain pan design
 - Washing Press discharge piping
 - Explosion-proof 5.0 HP motor
- NEMA 7 Local Control Stations rated for Class 1, Division 1, Group D hazardous area
- NEMA 4X Main Control Panel, to include VFD (screen), motor starter (press), Milltronics HydroRanger 200 HMI Differential Level Controller, Allen-Bradley Micro 800 PLC and Red Lion OIT
- Headworks Building (29' x 13.5') rated for Class 1, Division I hazardous space
- Provide ventilation for compliance with current NFPA 820 requirements

Existing Headworks Building Modifications

- Building modifications to remove existing equipment and install new
- Lakeside Headworks Acceptance Plant with:
 - Aerated grit chamber
 - 8" diameter transfer screw w/ 1 HP drive
 - 8" diameter dewatering screw w/ 2 HP drive
 - Air header with stainless steel diffusers
 - Blower package w/ 2 HP motor
 - o NEMA 4X main control panel with Allen Bradley PLC
- HVAC upgrade for Headworks Building
- Gas detection system

Exhibit



Figure 5-2 displays the proposed Headworks upgrade.

Figure 5-2 Headworks – Multi-Rake Screen

Non-Monetary Considerations

• Construction sequencing constraints are required to ensure maintenance of flows and operation of the existing Headworks during installation, startup, and testing.

Cost Estimate

A preliminary opinion of probable construction cost for Headworks Alternative #2 is provided in Table 5-6 on the following page. A detailed breakdown of this opinion of probable cost is provided in Appendix F.

Table 5-6 Headworks Alternative #2 – Construction Cost Estimate

Item	Cost ¹
New Screening in Existing Wet well	
Screening (Multi-Rake Screen and Wash Pressing)	\$363,000
New Headworks Building Above Influent Wet well	\$315,500
Grit Removal in Existing Headworks Building	
Grit Removal System (Aeration Zone, Settling Area, Baffles, Grit Screw)	\$527,500
Building Modifications for Demolition and Installation	\$60,000
HVAC Upgrade for Headworks Building	\$75,000
Gas Detection System	\$15,000
Total Capital Cost Subtotal	\$1,356,000
Contractor Mark-Up ²	\$298,000
Total Construction Cost (rounded) ^{3,4}	\$1,654,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.3. Biological Process – Anoxic Selectors

The existing submersible mixers in the Anoxic Selectors are in need of an age-related upgrade. The recommended upgrade involves replacing the existing submersible mixers with compressed gas mixing technology. The compressed gas mixing in the anoxic selectors will share a compressor with the compressed gas mixing system for the septage and sludge holding tank, as discussed in Section 5.6.12.

Design Criteria

Design criteria for the recommended anoxic selector upgrade is presented in Table 5-7 on the following page.

Item Description	Existing Design	Proposed Design	Design Standard
Mixing System			
Turne		Compressed Gas	
туре		Mixing System	
		1 nozzle grid/tank	TR-16: Independent
Quantity	3 (1/anoxic tank)		mixing should be
		1 compressor	provided.
		Compressors (2)	100% redundancy
Туре	Mixers (3)	1 duty, 1 stand-by	with largest unit out
		13 scfm @ 116 psi	of service
Motor Size, each	1.21 HP	3 HP	
Receiver Tank		120-gallon	
Anoxic Selector 1 & 2			L
Quantity	2	2	
Capacity	3,000 gal (each)	3,000 gal (each)	
Dimensions	8.25' x 6.0'	8.25' x 6.0'	
SWD	8.10'	8.10′	
Solids Conc.	0.5%	0.5%	
Headers/Tank		1	
Nozzles/Header		2	
Nozzles/Tank		2	
Nozzle Density		24.8 ft ² /nozzle	
Anoxic Selector 3			
Quantity	1	1	
Capacity	6,000 gal	6,000 gal	
Dimensions	16.50' x 6.0'	16.50' x 6.0'	
SWD	8.10'	8.10′	
Solids Conc.	0.5%	0.5%	
Headers/Tank		1	
Nozzles/Header		3	
Nozzles/Tank		3	
Nozzle Density		33 ft ² /nozzle	

Table 5-7 Biological Process - Anoxic Selector Design Criteria

Description

Upgrades to the anoxic selectors will include the following:

- Implementation of a compressed gas mixing system in the three (3) anoxic selectors
 - Valve Module Control Panel with electrically actuated valves
 - \circ $\;$ 304 Stainless steel nozzles, headers, and header supply piping $\;$

- (2) 3 HP Rotary Screw Compressor (combined with the system for septage & sludge storage)
- 120-Gallon Receiver Tank (combined with the system for septage & sludge storage)

Exhibit

Figure 5-3 presents a schematic of the compressed gas mixing system in the anoxic selectors.



Figure 5-3 Anoxic Selectors Compressed Gas Mixing System Layout

Non-Monetary Considerations

- Compressed gas mixing will require less O&M time for the operators than conventional submersible mixers.
- 50% energy savings over mechanical mixing or diffused aeration
- Unlimited turn-down capability
- Zero in-tank maintenance with no mechanical or electrical components in wastewater
- Vapex odor control will provide an odor-less environment

Cost Estimate

A preliminary opinion of probable construction cost for Biological Process – Anoxic Selectors Alternative #2 is provided in Table 5-8 on the following page. A detailed breakdown of this opinion of probable cost is provided in Appendix F.

Item	Cost ¹
Compressed Gas Mixing System	\$44,200
Process Electrical and Instrumentation	\$15,000
Total Capital Cost Subtotal	\$59,200
Contractor Mark-Up ²	\$13,000
Total Construction Cost (rounded) ^{3,4}	\$72,000

Table 5-8 Biological Process – Anoxic Selectors Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.4. Biological Process – Aeration Tanks

The existing biological process requires age-related upgrades to equipment including new diffusers and the installation of new blowers and air piping to supply required air demand at peak flow as the existing blowers cannot meet peak demand.

Design Criteria

Design criteria for the recommended Biological Process – Aeration Tanks upgrade is presented in Table 5-9 on the following page.

Description	Existing Design	Proposed Design	Design Standard
Flow			
Design ADF	0.222 MGD	0.222 MGD	
Design PHF	1.152 MGD	1.152 MGD	
Aeration Basins			
Quantity	2	2	
Dimensions	38' x 38'	38' x 38'	
SWD	14'	14'	
Volume	150,000 gal (each)	150,000 gal (each)	
AOR	 Max: 1350 lb/day	Min: 377 lb O2/day Max: 1810 lb O2/day	TR-16 Mixing Requirements (minimum): 0.12 scfm/sf of tank area TR-16 Oxygen Supply: 0.85- 1.2 lbs Oxygen/lbs BOD removed + 4.2 lbs Oxygen/lbs Ammonia Oxidized at maximum daily loading conditions
SOTR		Min: 1080 lb O2/day Max: 5185 lb O2/day	
Air Flow	Min: 220 scfm	Min: 115 scfm	
7.11 T 10 W	Max: 450 scfm	Max: 550 scfm	

Table 5-9 Biological Process – Aeration Tanks – Design Criteria

Description

The proposed Biological Process – Aeration Tanks improvements will include the following components:

- (2) New diffused aeration grids each with:
 - (9) 4" PVC headers
 - o (21) 9" diameter fine bubble EDPM diffusers per header
 - Total of 189 diffusers
 - 4" PVC drop leg
 - Purge sump assembly
- (2) new positive displacement blowers
 - \circ Sound enclosures
 - o VFDs
- New D.O. Probes (2) in aeration tanks
- New outdoor air intake
- Demolition of air intake from sludge holding tanks
- New air discharge piping to aeration tanks

• SCADA integration for D.O. pacing of blowers

Non-Monetary Considerations

• New, larger blowers will bring the Richmond WWTF into compliance providing the ability to treat peak flow with the largest unit out of service.

Cost Estimate

A preliminary opinion of probable construction cost for the proposed Biological Process – Aeration Tanks improvements is provided in Table 5-10. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item	Cost ¹
New Fine Bubble Diffusers	\$68,300
New Blowers (VFDs, new air intake & associated piping modifications)	\$237,000
New Separate Air Header to Aeration Tank #2	\$24,000
Process Electrical, Instrumentation & SCADA programming	\$48,000
Total Capital Cost Subtotal	\$377,300
Contractor Mark-Up ²	\$84,000
Total Construction Cost (rounded) ^{3,4}	\$462,000

Table 5-10 Biological Process – Aeration Tanks – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.5. Coagulant Chemical Feed & Storage

The existing coagulant chemical feed & storage system is in the basement of the Process Building and includes 1,500 gallon storage tank, one (1) chemical feed pump that feeds to the aeration tank effluent, and one (1) chemical feed pump stored on the shelf. The existing secondary containment for the bulk coagulant storage tank does not meet TR-16 standards which recommend containment that is 125% of the total chemical storage volume.

The recommended project includes implementation of two (2) coagulant feed pumps (1-duty, 1 standby). Two discharge locations will be provided, namely to the (i) influent wet well and (ii) aeration tank effluent channel. The existing secondary containment for the 1,500 gallon sodium aluminate storage tank will be increased to 125% of the total volume of chemical stored in order to meet modern code and standards.

Design Criteria

Existing and proposed design criteria for the coagulant chemical feed & storage are presented in Table 5-11.

Table 5-11	Chemical	Feed &	Storage	Design	Criteria
	cilcilicai	I CCU O	JULIA	Design	Cificilia

Item Description	Existing Design	Proposed Design	Design Standard			
Coagulant Storage for Biologica	Coagulant Storage for Biological Process					
Congulant Storago	1,500 gallon	1,500 gallon				
Coagulant Storage	storage tank	storage tank				
Spill Containment Volume	1,505 gallons	1,875 gallons	125% of storage tank			
			volume (TR-16)			
Coagulant Feed for Biological P	rocess					
Pump Quantity	2	2	Minimum of 2 (1 duty, 1			
	(1 duty, 1 on shelf)	(1 duty, 1 stand-by)	stand-by) (TR-16)			
	Aerated Tank	Influent Wet Well,				
Dosing Points		Aeration Tank				
		Effluent Channel				

Description

The recommended chemical feed and storage improvements will include the following:

- Additional height to concrete containment wall for existing chemical storage area
- Chemical feed pumps
 - Biological Process Chemical Feed: skid-mounted duplex pump chemical feed system

Non-Monetary Considerations

- Providing two (2) dosing locations allows for multi-point chemical dosing which can decrease the total coagulant used.
- Increasing secondary containment bring the Richmond WWTF into regulation with modern code and standards.

Cost Estimate

A preliminary opinion of probable construction cost for recommended coagulant chemical storage and feed improvements is provided in Table 5-12 on the following page. A detailed breakdown of this opinion of probable cost is provided in Appendix F.

Item	Cost ¹
Chemical Spill Containment – 8" wall addition	\$1,400
Chemical Feed Pumps (duplex pump skid), installed	\$16,900
Miscellaneous Chemical Feed Piping and Valves	\$5,000
Electrical, Instrumentation & Controls	\$5,000
Total Capital Cost Subtotal	\$28,300
Contractor Mark-Up ²	\$6,000
Total Construction Cost (rounded) ^{3,4}	\$34,000

Table 5-12 Coagulant Chemical Feed and Storage – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.6. Return Activated Sludge & Waste Activated Sludge

The existing waste activated sludge (WAS) pump and return activated sludge (RAS) electrically actuated pinch valve and magnetic flow meter are located in the basement of the Process Building. The existing equipment is reaching the end of its useful life. Therefore, the recommended project includes replacement of the existing double-disc, positive displacement WAS pump in-kind, replacement of the existing magnetic flow meter.

Due to lack of redundancy, contingencies are noted for the WAS system to provide temporary bypass WAS pumping by means of lowering a trash pump into either clarifier and running a discharge hose to the outdoor aerobic sludge holding tank, in case of pump failure.

No contingencies for the RAS system are recommended as the electrically actuated pinch valve would fail in the open position and flow could be controlled by manually throttling valves while replacement parts are procured.

Design Criteria

Design criteria for the WAS system are presented in Table 5-13 on the following page. Design criteria for the RAS system are presented in Table 5-14.

Item Description	Existing Design	Proposed Design	Design Standard		
Waste Activated Sludge (WAS)					
Number of Pumps	1	1			
	Double-Disc,	Double-Disc,			
Pump Type	Positive-	Positive-			
	Displacement	Displacement			
Capacity	150 gpm @ 18' TDH	150 gpm @ 18' TDH	At facilities with an ADF of 10 MGD or less, WAS pumping should have a maximum capacity of		
Minimum Flow Rate	-	80 gpm	25% of ADF and provide a minimum flow rate of ~80 gpm. (10 State Standards)		
Suction Condition	Flooded	Flooded			
Motor Power	7.5 HP	7.5 HP			
VFD	Yes	Yes			
Material	Sludge, 0.5-1% solids	Sludge, 0.5-1% solids			

Table 5-13 Waste Activated Sludge System Design Criteria

Table 5-14 Return Activated Sludge System Design Criteria

Item Description	Existing Design	Proposed Design	Design Standard	
Return Activated Sludge (RAS)				
RAS Flow Target (150% ADF)	90-150% ADF	90-150% ADF		
RAS Capacity	0.0657 - 0.333 MGD	0.0657 - 0.333 MGD		
RAS Control Valve Quantity	1	1		
RAS Valve Size	4-inch	4-inch		
RAS Value Ture	Electrically Actuated	Electrically Actuated		
RAS valve Type	Pinch Valve	Pinch Valve		
RAS Flow Meter Quantity	1	1		
RAS Flow Meter Type	Magnetic	Magnetic		
RAS Flow Meter Size	4-inch	4-inch		

Description

The recommended Waste Activated Sludge System will include the following:

- WAS Pump
 - One (1) Double-Disc, Positive-Displacement pump
- New plug valves on suction and discharge piping

The recommended Return Activated Sludge System will include the following:

• RAS Valve Replacement

- One (1) Pinch Valve
- One (1) Electric Actuator
- One (1) Magnetic Flow Meter

Non-Monetary Considerations

• Construction sequencing constraints are required to replace RAS and WAS system while ensuring continuous operation.

Cost Estimate

A preliminary opinion of probable construction cost for replacement of the Waste Activated Sludge Pump System is provided in Table 5-15. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Table 5-15	WAS &	RAS System -	Construction	Cost Est	imate
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Item	Cost ¹
RAS Pinch Valve & Actuator Replacement	\$33,500
New RAS Flow Meter	\$13,000
WAS Pump Replacement	\$53,000
Plug Valves	\$24,000
Total Capital Cost Subtotal	\$123,500
Contractor Mark-Up ²	\$28,000
Total Construction Cost (rounded) ^{3,4}	\$152,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.7. Filtration

The existing filter tanks cannot be replaced due to space constraints in the existing building. Therefore, the recommended project includes remaining with the existing units. The upgrade will include a structural assessment of the existing steel tanks to determine any in-place tank repairs needed, replacement of the existing cloth media, backwash and sludge removal pumps, drive motors, valves & piping, and associated SCADA programming.

Design Criteria

Design criteria for the filtration system are provided in Table 5-16 on the following page.

Description	Existing Design	Proposed Design	Design Standard
Flows		· · · · · · · · · · · · · · · · · · ·	
Average Flow	0.222 MGD	0.222 MGD	
Peak Hourly Flow	1.152 MGD	1.152 MGD	Treat PHF with 1 unit off-line
Туре	Cloth Media Disk	Cloth Media Disk	
Filter Tanks			
Number of Tanks	2	2	
Manufacturer/ Model Number	Aqua-Aerobic Systems, Inc. Cloth Media Filter ADFP- 54X2E-PC	Aqua-Aerobic Systems, Inc. Cloth Media Filter ADFP- 54X2E-PC	
Number of Disks, Total	4 (2, 2-Disk Units)	4 (2, 2-Disk Units)	
Dimensions	8' x 9'-2"	8' x 9'-2"	
SWD	7.84'	7.84'	
Max Water Level	9.61'	9.61'	
	53.8 sf/disk	53.8 sf/disk	
Filter Area Provided	107.6 sf/unit	107.6 sf/unit	
	215.2 sf total	215.2 sf total	
Hydraulic Loading	1.43 gpm/sf @ ADF (0.222 MGD) – one filter unit (2 disks) 7.43 gpm/sf @ PHF (1.152 MGD) – one filter unit (2 disks)	1.43 gpm/sf @ ADF (0.222 MGD) – one filter unit (2 disks) 7.43 gpm/sf @ PHF (1.152 MGD) – one filter unit (2 disks)	@ADF: 3.25 gpm/sf @PHF: 6.5 gpm/sf
Rate	0.72 gpm/sf @ ADF (0.222 MGD) – two filter units (4 disks) 3.72 gpm/sf @ PHF (1.152 MGD) – two filter units (4 disks)	0.72 gpm/sf @ ADF (0.222 MGD) – two filter units (4 disks) 3.72 gpm/sf @ PHF (1.152 MGD) – two filter units (4 disks)	(TR-16)
Solids Loading Rate ¹	0.52 lbs TSS/sf/day @ ADF (0.222 MGD) – one filter unit (2 disks) 2.68 lbs TSS/sf/day @ PHF (1.152 MGD) – one filter unit (2 disks) 0.26 lbs TSS/sf/day @ ADF (0.222 MGD) – two filter units (4 disks) 1.34 lbs TSS/sf/day @ PHF (1.152 MGD) – two filter units (4 disks)	0.52 lbs TSS/sf/day @ ADF (0.222 MGD) – one filter unit (2 disks) 2.68 lbs TSS/sf/day @ PHF (1.152 MGD) – one filter unit (2 disks) 0.26 lbs TSS/sf/day @ ADF (0.222 MGD) – two filter units (4 disks) 1.34 lbs TSS/sf/day @ PHF (1.152 MGD) – two filter units (4 disks)	< 2.0 lb TSS/sf/day
Sludge Removal Pum	p		
Quantity	2	2	
Power Draw	3 HP	3 HP	

Table 5-16. Filtration Design Criteria

Motor Drives				
Quantity	2	2		
Power Draw	3 HP	3 HP		

Description

The recommend filter upgrade will include the following:

- Structural assessment of the existing steel tanks to determine in-place tank repairs needed
- Replacement of the following parts for each filter:
 - o Cloth media
 - Backwash pumps
 - Sludge removal pumps
 - Drive motors
 - Valves and piping
- Instrumentation and SCADA programing

Non-Monetary Considerations

• Construction sequencing constraints are required to ensure adequate treatment capacity during equipment replacement, assessment, and testing.

Cost Estimate

A preliminary opinion of probable construction cost for replacement of the Filtration System improvements is provided in Table 5-17. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item	Cost ¹
Structural Assessment and Tank Repair	\$40,000
Filter Replacement Parts	\$116,600
Instrumentation & SCADA Programming	\$20,000
Total Capital Cost Subtotal	\$176,600
Contractor Mark-Up ²	\$40,000
Total Construction Cost (rounded) ^{3,4}	\$217,000

Table 5-17 Filtration System Improvements – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.8. Ultraviolet Disinfection

The existing ultraviolet (UV) disinfection system is located in the upper level of the Process Building and is sized to treat a peak hour flow 1.0 MGD. In order to address age-related equipment needs and meet the design peak hour flow, the recommended project includes a new UV disinfection system sized to treat a peak hour flow of 1.152 MGD. The existing stainless steel channel will be used. Recommended replacement includes the UV modules, instrumentation, and monitoring/control system inside the existing stainless steel channel.

Design Criteria

Design criteria for the UV disinfection system is provided in Table 5-18.

Item Description	Existing Design	Proposed Design	Design Standard
Average Daily Flow (ADF)	0.222 MGD	0.222 MGD	
Peak Hourly Flow (PHF)	1.0 MGD	1.152 MGD	
TSS Concentration	10 mg/L	10 mg/L	< 30 mg/L TSS (TR- 16)
UV Transmittance	-	65%	65% minimum
UV Radiation Wavelength (nm)	-	254 nm	254 nm (TR-16)
UV Dose at PHF	-	30 mJ*sec/cm ²	> 30 mJ*sec/cm ²
Number of UV Denks	2	2	
Number of OV Banks	(1 duty, 1 stand-by)	(1 duty, 1 stand-by)	
Number of UV Modules (Total)	20 (10/bank)	20 (10/bank)	
Number of Lamps per Module	4	4	
Total Number of UV Lamps	80	80	
Liquid Depth	12.6″	12.6″	

Description

The recommended UV disinfection upgrade will include the following:

- Two (2) UV Banks, each containing:
 - 8 Type 316 stainless steel modules
 - 4 UV low pressure lamps/module
- Monitoring system for indication of UV intensity, lamp age, and alarms
- Remote indication of UV intensity
- Remote indication of low UV intensity alarm
- Maintenance module cleaning rack

Non-Monetary Considerations

• The recommended project will bring the Richmond WWTF up to modern code and standards by disinfecting the peak hour flow with one unit out of service.

Cost Estimate

A preliminary opinion of probable construction cost for a replacement UV disinfection system is provided in Table 5-19. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F on the following page.

Table 5-19 UV	Disinfection –	Construction	Cost Estimate

Item	Cost ¹
UV System Equipment Replacement	\$189,000
Capital Cost Subtotal	\$189,000
Contractor Mark-Up ²	\$41,000
Total Construction Cost (rounded) ^{3,4}	\$230,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.9. Effluent Flow Measurement

The existing effluent flow measurement at the Richmond WWTF consists of a 90° v-notch weir, an ultrasonic level detector, and an effluent flow meter. During wet weather events, the head on the weir exceeds the top of the v-notch. This results in flow going over the top of the weir and inaccurate measurement. In order to alleviate inaccurate measurement issues and address age-related needs, upgrades to effluent flow measurement are recommended.

The recommended effluent flow measurement upgrade includes a new 90° v-notch weir, a new radar level sensor, and an associated flow meter. The new 90° v-notch weir will have a different geometry than the existing v-notch weir to allow higher flows to be measured and allow for more flow through the v-notch without overtopping the top of the weir. Recommended geometrical changes will maintain the existing hydraulic profile at maximum flow.

Design Criteria

Design criteria for effluent flow measurement is provided in Table 5-20 on the following page.

Item Description	Existing Design	Proposed Design	Design Standard			
Effluent Flow Measurem	Effluent Flow Measurement					
Control Device	90° V-Notch Weir	90° V-Notch Weir	Accurate measurement over full range of design flows (TR-16)			
Invert of V-Notch Weir Elevation	306.13 ft	305.58 ft				
Measurement	Flow Meter, Ultrasonic Level Detector	Flow Meter, Radar Level Sensor				
Flow Meter Capacity	Minimum: 0.03 MGD at 0.2' above v-notch Maximum: 1.616 MGD at 1' above v-notch	Minimum: 0.03 MGD at 0.2' above v-notch Maximum: 3.883 MGD at 1.42' above v-notch	0.2 ft ¹ – 1.12 ft of head			

Table 5-20. Effluent Flow Measurement Design Criteria

Description

The recommended effluent flow measurement upgrade will include:

- One (1) new stainless steel 90° v-notch weir plate
- One (1) radar level sensor and flow meter
- SCADA program allowance

Non-Monetary Considerations

• Flow measurement at significantly higher flows will be accommodated

Cost Estimate

A preliminary opinion of probable construction cost for a new effluent flow measurement system is provided in Table 5-21 on the following page. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item		Cost ¹
New V-Notch Weir		\$5,000
Radar Level Sensor & Flow Meter		\$5,000
SCADA Program Allowance		\$2,000
	Capital Cost Subtotal	\$12,000
Contractor Mark-Up ²		\$3,000
	Total Construction Cost (rounded) ^{3,4}	\$15,000

Table 5-21 Effluent Flow Measurement – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.10. Effluent Pumping & Outfall

The existing effluent wet well leads to manhole MH13 through a 12" pipe before being discharged to the Winooski River through an 18" reinforced concrete outfall pipe. The outfall was equipped with a check valve after Tropical Storm Irene flooding and a sump pump was installed in the effluent wet well. During flood events, the sump pump is used to discharge effluent to a higher elevation to prevent flows from backing up into the facility.

Several top of wall elevations are lower than the 500-year flood elevation and the top of the UV channel is lower than the 100-year flood elevation. In order to protect existing equipment from overflowing during wet weather conditions, upgrades including the construction of an effluent pump station are recommended. The new effluent pump station will be located adjacent to the Process Building and will include a new effluent pump station wet well equipped with two (2) vertical turbine effluent pumps. Flow from the existing effluent wet well located inside the Process Building will pass through the new effluent pump station wet well. Typically, effluent will continue to MH13 and out the existing 18" outfall. In the case of elevated river levels, the water level in the pump station wet well will rise and the vertical turbine effluent pumps will turn on to pump effluent to a higher discharge elevation.

The pumps will be equipped with variable frequency drives and a radar level detection system in the new wet well will control the speed and output of the effluent pumps to match incoming flow. The effluent line from MH13 to MH14 will be equipped with a backflow preventer to prevent he river from backing up into the Process Building. A small enclosure for the pumps and controls will be provided. The top slab of the pump station wet well will be above the 500-year flood elevation.

Design Criteria

Design criteria for effluent pumping and outfall is provided in Table 5-22 on the following page.

Parameter	Existing Design	Design Value		
Current Average Daily Flow	0.073MGD	0.073 MGD		
Average Daily Flow	0.222 MGD	0.222 MGD		
Peak Hour Flow	1.152 MGD	1.152 MGD		
Peak Instantaneous Flow	2.3 MGD	2.3 MGD		
Effluent Pumps				
Quantity	1	2		
Capacity	unknown	1080 gpm @ 15.1' TDH		
Туре	Submersible	Vertical Turbine		
Motor	unknown	7.5 HP		

Table 5-22 Effluent Pumping & Outfall Design Criteria

Description

The recommended upgrades to effluent pumping and the outfall will include:

- New effluent pump station with concrete wet well, approx. 7' x 9' x 15' depth
- Two (2) effluent pumps
 - \circ $\;$ Two (2) variable speed, vertical turbine pumps, 7.5 HP each
 - Design capacity 1080 gpm @ 15.1' TDH each
- One (1) control panel with integral VFDs
- Level control system
- Climate controlled enclosure, approx. 10' x 9'
- 18" backflow preventer

Exhibit

Figure 5-4 on the following page displays a conceptual layout of the recommended effluent pump station.

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Figure 5-4 Effluent Pump Station Conceptual Layout

Non-Monetary Considerations

- New effluent pump station will protect the WWTF from flooding events
- Construction sequencing will require temporary by-pass piping to the next outfall manhole downstream to maintain effluent flow.

Cost Estimate

A preliminary opinion of probable construction cost for the effluent pumping and outfall is provided in Table 5-23 on the following page. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item	Cost ¹
Effluent Pumps	\$169,000
Effluent Pump Station (wet well, enclosure structure, HVAC, electrical)	\$162,200
Control Panel, VFDs, Level Control System	\$58,800
Process Piping & Valves	\$32,000
Capital Cost Subtotal	\$422,000
Contractor Mark-Up ²	\$92,000
Total Construction Cost (rounded) ^{3,4}	\$514,000

Table 5-23 Effluent Pumping & Outfall – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.11. Septage Receiving Facilities

The existing septage receiving facilities consist of a septage receiving unit which is at the end of its useful life. The septage receiving facilities are located in the garage on the upper level of the Processing Building. The area lacks proper ventilation and vents have been cut into the adjacent electrical room.

In order to address age-related needs and safety concerns, the recommended project includes a new septage receiving unit that incorporates a key card system with a flow meter for recording septage hauler discharges. The recommended upgrade includes a new ventilation system, separation of the septage receiving area from the rest of the garage with a newly constructed wall, and closure of the vent opening between the septage receiving space and the electrical room. These recommended upgrades will bring the area up to compliance with NFPA 820 hazardous classifications and address safety concerns.

Design Criteria

The design criteria for the septage receiving facilities upgrade is provided in Table 5-24 on the following page.

Item Description	Existing Design ¹	Current Loadings	Proposed Design	Design Standard
Septage Flows				
Volume (average)	166,667 gal/month	400,000 gal/month ²	50,000 gpd ² 250,000 gal/week ⁴	
Historical Daily Max Septage Received	-	87,000 gpd ³		
Total Volume of Septage	2,000,000 gal/yr	4,800,000 gal/yr ²	13,000,000 gal/yr	
Septage Receiving Unit				
Max. Septage Hydraulic Capacity	400 gpm (< 3% solids)		400 gpm (< 3% solids)	
Max. Hydraulic Capacity	2061 gpm		2061 gpm	
Tank Size	3.25' w x 6' l x 3.92' h		3.25′ w x 6′ l x 3.92′ h	
Tank Material	Stainless steel		Stainless steel	
Screening Basket Size	31" diameter		31" diameter	
Screening Bar Spacing	1/4"		1⁄4"	
Max. Upstream Water Level	13.75 in		13.75 in	
Max. Headloss	8″		8″	
Screw Conveyor Diameter	10"		10"	
Water Requirements	20 gpm @ 60 psi		20 gpm @ 60 psi	
Inlet Valve	4" pinch valve		4" pinch valve	
Motor Drive	2 HP		2 HP	

Table 5-24. Septage Receiving Facilities Design Criteria

Notes:

- 1. Source: Basis of Design, 2003
- 2. Based on response in 2023 VTDEC WWTF Septage Acceptance Questionnaire
- 3. Based on Monthly Operating Report data from January 2018 to February 2023.
- 4. Based on 5 days/week of septage acceptance.

Description

The recommended septage receiving facility upgrade will include the following:

Septage Receiving Unit

- One (1) Septage Receiving Unit
 - o 304 stainless steel construction
 - Tank assembly with vent
 - Fine Screen with 2 hp drive
 - o 3-plane screen basket design with rotating rake assembly
 - 4-inch diameter pinch type inlet valve

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- o 4-Zone wash system with solenoid valves
- Ultrasonic level controller with one (1) transducer
- Screenings bagger
- Control Panel
 - NEMA 4X 304 stainless steel main control panel
 - Allen-Bradley MicroLogix 1400 PLC
 - Variable frequency drive
 - RACS control station
- Management and Accounting System
- Replacement piping and valves

Building Modifications

- Structural Modifications
 - Construction of CMU wall to separate septage receiving facilities from rest of garage
 - Block off the vents that have been cut between the electrical room and septage receiving facilities
- New HVAC system for Garage area and new Septage Room

Exhibit

Figure 5-5 displays the layout for the recommended septage receiving facility upgrade.



Figure 5-5 Septage Receiving Facility Layout

Non-Monetary Considerations

- Garage upgrades will bring the area into compliance with NFPA 820 hazardous classifications
- New septage receiving unit, card reader station, and flow meter will allow Town to track and charge septage receiving accordingly
- Construction sequencing will require the Richmond WWTF to stop accepting septage temporarily during construction.

Cost Estimate

A preliminary opinion of probable construction cost for a new septage receiving unit and septage receiving facilities upgrade is provided in Table 5-25. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Table 5-25	Septage	Receiving	Facilities -	Construction	Cost Estimate

Item	Cost ¹
Septage Receiving Unit	\$399,000
Piping and Valve Replacements	\$50,000
HVAC Replacement for Garage and Septage Room	\$150,000
Structural Modifications for New Septage Room	\$115,000
Capital Cost Subtotal	\$714,000
Contractor Mark-Up ²	\$157,000
Total Construction Cost (rounded) ^{3,4}	\$871,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.12. Septage & Sludge Storage Facilities

Septage and sludge storage consists of two (2) aerated holding basins below the floor in the upper level of the Process Building and one (1) aerobic sludge holding tank. The existing diffused aerated system in the two (2) aerated septage and sludge holding basins experiences clogging from accumulated solids and the tank decant has been disconnected. In the outdoor aerobic sludge holding tank, some diffusers are clogged and are reaching the end of their useful life. The recommended project includes replacing the existing aeration mixing system in each storage area with a compressed gas mixing system.

Additionally, septage and sludge transfer pump #1 transports sludge from the two (2) aerated holding basins to the one (1) aerobic sludge holding tank. The recommended upgrade includes replacing the existing pump with two (2) new transfer pumps to provide redundancy and address age-related needs.

The recommended upgrade includes implementation of a new Vapex odor control system. The Vapex odor control system will be implemented to dose the enclosed air spaces within the two (2) septage and

sludge holding basins and the one (1) aerobic sludge holding tank. The aerobic sludge holding tank will receive a new cover to allow for a closed space for odor control.

Design Criteria

Design criteria for the septage and sludge storage is provided in Table 5-26. Design criteria for the Vapex odor control system is located in Table 5-27. Design criteria for the septage and sludge storage mixing is provided in Table 5-28.

Item Description	Existing Design ¹	Current Loadings	Proposed Design		
Septage					
Septage Volume (average)	166,667 gal/month	400,000 gal/month ²	50,000 gpd ² 250,000 gal/week ⁴		
Historical Daily Max Septage Received	-	87,000 gpd ³			
Total Volume of Septage	2,000,000 gal/yr	4,800,000 gal/yr ²	13,000,000 gal/yr		
Secondary Sludge Production	Secondary Sludge Production				
Dry Solids Produced from	Not available		1,524 lbs/day		
Secondary Process	Netavailable		10/5		
Percent solius	NOT available		1%		
WAS from Secondary	Not available	2.662 gnd^3	17 393 gnd ⁵		
Process		2,002 600	1,333 860		
Total Septage & WAS Storage Needed					
Daily Volume Needed	Not available	22,662 gal	67,393 gal		

Table 5-26 Septage & Sludge Storage Facilities

Notes:

- 1. Source: Basis of Design, 2003
- 2. Based on response in 2023 VTDEC WWTF Septage Acceptance Questionnaire
- 3. Based on Monthly Operating Report data from January 2018 to February 2023.
- 4. Based on 5 days/week of septage acceptance.
- 5. Results from Biological Model Results

Table 5-27 Vapex Odor Control System – Design Criteria

Parameter	Design Value
Volumes Treated	
Septage & Sludge Holding Basin #1	531 cf
Septage & Sludge Holding Basin #2	531 cf
Aerobic Holding Basin	862 cf
Odor Control System	
Туре	Ionization Technology
Number of Units	1
Dimensions	54" x 35" x 44"
Atomizing Nozzles per Unit	6
Oxidant Output	≤ 50 lbs/day
Average Water Usage	≤ 8 gal/hr/nozzle
Nozzle Air Output	20 cfm/nozzle
Treatment Coverage Volume	26,000 cf (maximum)
Power Requirements	220 VAC, 23A, 60 Hz

Table 5-28 Septage & Sludge Storage Alternative #2 Design Criteria

Item Description	Existing Design	Proposed Design	Design Standard		
Aerated Septage & Sludge Holding Tank					
Quantity	2	2			
Tank Covering	Covered, Concrete	Covered, Concrete			
Dimensions	21.58' x 14.58'	21.58' x 14.58'			
SWD	10'	10'			
Capacity	23,000 gal (each) 46,000 gal (total)	23,000 gal (each) 46,000 gal (total)			
Mixing System	Fine Bubble, Membrane Type Diffusers	Compressed Gas Mixing System			
Blower Type	Positive Displacement	Positive Displacement Rotary Lobe Compressor			
Number	1	2 (1 duty, 1 standby)	100% redundancy with largest unit out of service		
Blower Capacity	200 scfm @ 5 psi	20 scfm @ 116 psi			
Motor	10 HP	5 HP			
VFD	Yes	Yes			
Receiver Tank		120-gallon			
Aerobic Sludge Holding Basin					
Quantity	1	1			
Tank Covering	Open	Covered			
Dimensions	38' x 8.5'	38' x 8.5'			

Item Description	Existing Design	Proposed Design	Design Standard
SWD	14'	14'	
Capacity	34,000 gal	34,000 gal	
Mixing System	Coarse Bubble Diffusers	Compressed Gas Mixing System	
Blower Type	Positive Displacement	Shared with Septage	
Blower Capacity	170 scfm @ 7 psi	Shared with Septage	
Motor	10 HP	Shared with Septage	
VFD	Yes	Shared with Septage	
Sludge Transfer Pump #1			
Quantity	1	2	
Туре	Double-Disc, Positive- Displacement	Rotary Lobe	
Capacity	150 gpm @ 18' TDH	150 gpm @ 8' TDH	
Solids Content	Sludge, 1-3%	Sludge, 1-3%	
Motor	7.5 HP	5 HP	
VFD	Yes	Yes	

Description

The recommended seepage and sludge storage facility upgrade will include the following:

- Implementation of an EnviroMix compressed gas mixing system in the two (2) sludge holding tank and the aerobic sludge holding basin.
 - \circ ~ Valve Module Control Panel with electrically actuated valves
 - o 304 Stainless steel nozzles, headers, and header supply piping
 - (2) 3 HP Rotary Screw Compressor (combined with the system for anoxic selectors)
 - 120-Gallon Receiver Tank (combined with the system for anoxic selectors)
- Sludge Transfer Pumps
 - Two (1) Rotary Lobe type sludge transfer pumps
- Instrumentation, including
 - o Air flow meters
 - Pressure transducer level indication with redundant floats for high and low level alarms
 - Magnetic flow meter for sludge transfer
- Concrete cover with hatches for the aerobic sludge holding basin for odor containment
- Site Work including
 - Yard compressed gas piping

A Vapex odor control system would include the following:

- One (1) Milli Vapex Unit
- 6 nozzles flexible
- Water tubing

- Oxidant tubing
- Air tubing

Exhibit

Figure 5-6 displays the Enviromix compressed gas mixing layout for the septage and sludge holding tanks.



Figure 5-6 Septage and Sludge Holding Tanks Compressed Gas Mixing Layout

Non-Monetary Considerations

- 50% energy savings over mechanical mixing or diffused aeration
- Unlimited turn-down capability
- Zero in-tank maintenance with no mechanical or electrical components in wastewater
- Vapex odor control will provide an odor-less environment
- Provides sludge transfer pump redundancy

Cost Estimate

A preliminary opinion of probable construction cost for the recommended Septage and Sludge Storage Facilities upgrade is provided in Table 5-29 on the following page. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item	Cost ¹
Compressed Gas Mixing System (equipment, piping & valves, nozzles)	\$215,800
Electrical, Instrumentation & Controls	\$50,000
Sludge Transfer Pumps	\$99,400
Aerobic Sludge Holding Tank Cover	\$30,000
Yard Air Piping	\$10,500
Odor Control	\$266,800
Capital Cost Subtotal	\$672,500
Contractor Mark-Up ²	\$148,000
Total Construction Cost (rounded) ^{3,4}	\$821,000

Table 5-29 Septage & Sludge Storage Alternative #2 – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.13. Dewatering Facilities

The existing Dewatering Facilities consist of a 3,000-gallon fiberglass sludge day tank, a flocculator, a polymer feed system, and a two-channel rotary press. There is one (1) sludge pump that transports sludge from the aerobic sludge holding tank to the day tank. One (1) sludge feed pump transfers sludge from the day tank to the dewatering equipment.

In October 2023, the Town of Richmond purchased two (2) new dewatering channels to replace the existing channels, one (1) flocculator, and a new control panel with equipment installation assistance and coordination. The Town of Richmond and the Richmond WWTF have also expressed interest in accepting up to 50,000 gpd of septage.

The recommended upgrade addresses equipment that is reaching the end of its useful life and provides Richmond the opportunity to accept up to 50,000 gpd of septage. The recommended upgrade includes a new rotary press that is equipped to handle 4-channels. The new rotary press will come with two (2) new channels, with the intention of placing the recently purchased two (2) channels on the new rotary press. The recently purchased control panel and flocculator will be reused for the new rotary press.

The recommended upgrade also includes a new polymer feed system, new coagulant feed system to provide coagulant dosing to the rotary press pressate, new sludge day tank mixer and level detection system, new sludge transfer and sludge feed pumps, new dewater sludge cake conveyor system, new electrical room, and new HVAC system. The existing Dewatering Building will be expanded to allow for room for new equipment and provide additional area for storage as the existing Storage Garage will be demolished.

Design Criteria

Item Description	Existing Design	Proposed Design	Design Standard
Sludge Characteristics	•	•	
Solids Feed ¹	26,445 gpd	58,300 gpd (WAS: 8,300 gpd Septage: 50,000 gpd)	
Solids Concentration ²	WAS: 9,300 mg TSS/L Septage: 1,760 mg TSS/L	WAS: 25,000 mg TS/L ⁶ Septage: 8,000 mg TS/L ⁶	
Average Solids Feed % ³	0.26%	1.0% TS	
Sludge Disposal	98 wet tons/month ¹ 28.6 dry tons/month ³	52 dry tons/month ⁷	
Rotary Press			
Manufacturer/Model	Fournier Industries Inc. 2- 900/2000CV	Fournier Industries Inc. 4- 900/4000CV	
Туре	Rotary Press with Flocculator	Rotary Press with Flocculator	
Number of Units	1	1	
Channels	2	4	
Channel Diameter	36"	36″	
Motor	7.5 HP	5 HP	
Through-put ⁴	100 dry lbs/hr/channel	110 dry lbs/hr/channel	
Dewatered Sludge Cake Average Solids % ⁵	29.2%	29.2% ⁷	
Operational Hours		16 hours/day 5 days/week	
Expected Maximum Filtrate Flow Rate		17 gpm	
Expected Solids Capture ⁴		90%	
Sludge Transfer Pumps (From	n Digester to Day Tank)		
Quantity	1	2	
Туре	Double Disc, Positive Displacement	Rotary Lobe, Positive Displacement	
Manufacturer	Penn Valley Pump Co. Inc.	Borger	
Solids Content	1-3%	1-3%	
Capacity	50 gpm	50 gpm	
Max TDH	25.5 ft	25.5 ft	
Min TDH	11.5 ft	11.5 ft	
Max Suction Lift	19 ft	19 ft	
Min Suction Lift	5 ft	5 ft	
Motor	5 HP	5 HP	
VFD	Yes	Yes	
Sludge Day Tank			
Quantity	1	1	
Capacity	3,000 gal	3,000 gal	
Туре	Fiberglass Tank	Fiberglass Tank	
Mixer	Shaft impeller	Shaft impeller	

Table 5-30 Dewatering Facilities Design Criteria

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Item Description	Existing Design	Proposed Design	Design Standard	
Sludge Feed Pumps (From Day Tank to Rotary Press)				
Quantity	1	2		
Туре	Double Disc, Positive	Rotary Lobe, Positive		
	Displacement	Displacement		
Manufacturer	Penn Valley Pump Co. Inc.	Borger		
Solids Content	1-3%	1-3%		
Flow Rate	0-60 gpm	0-60 gpm		
Max TDH	20 ft	20 ft		
Min TDH	0 ft	0 ft		
Motor	3 HP	5 HP		
VFD	Yes	Yes		
Control	Rotary Press PLC	Rotary Press PLC		
Polymer Feed System				
	Liquid polymer activation,	Liquid polymer activation,		
Туре	dilution, and feed system	dilution, and feed system		
Type	including mixing chamber	including mixing chamber		
	and feed pump	and feed pump		
Chemical Feed Pump Type	Positive Displacement,	Peristaltic Type		
	Diaphragm Type	i chistalile i ype		
Number of Pumps	1	1		
Metering Pump Flow	0.4 – 8.0 gph	0.4 – 8.0 gph		
Dilution Water Flow	120 to 1,200 gph	120 to 1,200 gph		
Application Points	Sludge Flocculator	Sludge Flocculator		
Polymer Storage		1	I	
Polymer Storage	55-gallon drums	Two (2) 250-gallon totes		
Spill Containment Vol		625 gallons	125% of storage	
		020 Sanons	tank volume (TR-16)	
Coagulant Storage for Dewat	ering Pressate	1	Γ	
Coagulant Storage		Two (2) 250-gallon totes		
Spill Containment Vol		625 gallons	125% of storage	
		0_0 80.000	tank volume (TR-16)	
Coagulant Feed for Dewateri	ng Pressate	1	Γ	
		2	Minimum of 2 (1	
Pump Quantity		(1 duty, 1 stand-by)	duty, 1 stand-by)	
		((TR-16)	
Dosing Points		Rotary Press Pressate		

Notes:

- 1. Average is from historical operating data from January 2018-February 2023.
- 2. Concentration is from 9/21/2022 sampling event.
- 3. Calculated from historical operating data and sampling event.
- 4. From Rotary Press Basis of Design
- 5. Cake solids is average of five dewatered sludge testing results from 2022.
- 6. Further testing recommended.
- 7. Results anticipated based on similar sludge, but to be confirmed.

Description

The recommended upgrade to the Dewatering Facilities will include the following:

Dewatering Building:

- Approx. 32' building expansion (1,258 ft²)
- New Electrical/Control Room
- New internal block wall to separate sludge trailer area from dewatering area
- New electrical
- New HVAC System
- New plumbing with hose bibbs and slop sink
- Emergency eye wash and shower
- New gas detection system

Dewatering Equipment:

- New 4-channel rotary press
 - Two (2) new channels
 - Reuse two (2) existing channels
- Two (2) new sludge transfer pumps
 - o VFDs
 - Magnetic flow meter
- Relocated 3000-gallon Sludge Day Tank
 - o New shaft impeller mixer
 - New level detection
- Two (2) new sludge feed pump
 - o VFDs
 - Magnetic flow meter
- New shaftless screw conveyor system

Polymer Feed System:

- Peristaltic dosing pump
- Static mixing chamber
- Water pressure regulator
- Control panel
- 250-gallon polymer totes
- Containment area

Coagulant Feed System:

- Chemical feed pumps for dewatering pressate
 - Skid-mounted duplex pump chemical feed system
- Injection quill
• Containment area

Exhibit

Figure 5-7 provides a layout for the Dewatering Building expansion.





Non-Monetary Considerations

- Allow Richmond WWTF to accept up to 50,000 gpd of septage
- Operator flexibility with coagulant dosing locations

 Proper ventilation and separation of the sludge trailer from the dewatering area will bring the area into compliance with modern codes and standards and improve Operator safety

Cost Estimate

A preliminary opinion of probable construction cost for the recommended Dewatering Facilities upgrade is provided in Table 5-31. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item	Cost ¹
Dewatering Building Expansion & Upgrade	\$1,061,700
Dewatering Press (4-Channel Unit with 2 new channels & 2 reused channels)	\$279,300
Sludge Transfer and Feed Pumps	\$90,600
Polymer Feed System	\$51,200
Sludge Cake Conveyors	\$116,400
Sludge Day Tank Mixer	\$16,300
Coagulant Feed System	\$35,100
Process Piping & Valves	\$100,000
Capital Cost Subtotal	\$1,750,600
Contractor Mark-Up ²	\$386,000
Total Construction Cost (rounded) ^{3,4}	\$2,137,000

Tahlo	5-31	Νοω	Dowatoring	Facilitios -	Construction	Cost Estimate
Table	J - JT		Devvalering	racincics	construction	COSt Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.14. Operations Building

Recommended upgrades to the Operations Building include replacement of the existing boiler, new HVAC systems, and a new laboratory refrigerator. Recommended renovations to the existing structure include new energy efficient doors and windows, repainting and finishes, and upgrades to existing plumbing. A new hatch in the floor of the laboratory space will be installed to allow operators to hoist the influent pumps up.

Description

The recommended upgrade to the Operations Building will include the following:

- New Boiler
- New HVAC system
- New Laboratory Refrigerator
- New energy efficient doors and windows
- Hatch in the floor of the laboratory space

• Plumbing upgrades

Non-Monetary Considerations

- New laboratory refrigerator will allow for improved preservation of samples collected
- Improved operator flexibility to hoist influent pumps through hatch for maintenance

Cost Estimate

A preliminary opinion of probable construction cost for proposed improvements to the existing Operations Building is provided in Table 5-32. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Table 5-32 Proposed Operations Building Improvements – Construction Cost Estimate

Item	Cost ¹
Floor Access Door for Influent Pumps	\$32,500
New Energy Efficient Windows and Doors	\$65,000
New Laboratory Equipment Allowance	\$15,000
Plumbing and HVAC Upgrades	\$130,000
New Effluent Heat Recovery System Allowance	\$300,000
New Paint and Finishes	\$20,000
Capital Cost Subtotal	\$562,500
Contractor Mark-Up ²	\$123,000
Total Construction Cost (rounded) ^{3,4}	\$686,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.15. Site

The site that the Richmond WWTF is located on is above the 100-year flood elevation, but the Process Building ground elevation is below the 500-year flood elevation. The recommended project includes installing flood barriers on all exterior doors of the Process Building to protect against the 500-year flood.

It is recommended that a detailed electrical and instrumentation review is conducted by an electrical on the existing electrical system and standby generator to determine its existing condition and ability to meet future demands.

The recommended project includes upgrades and modifications to the existing SCADA system to integrate new equipment and control logic for unit processes.

The recommended project includes a new plant water system to allow the Richmond WWTF to use disinfected effluent for non-potable water services such as screen wash water, septage receiving unit wash water, polymer dilution, and dewatering wash water instead of using pot-able Town water. A package plant water duplex pump skid will be installed in the basement of the Process Building.

The recommended upgrade will also include a wider site entrance and gate and cold planning of the existing pavement and placement of 3" of new pavement.

Description

The recommended upgrade to the Site will include the following:

Flood Mitigation

• New exterior doors on the Process Building

WWTF Electrical System & Stand-by Generator

- Electrical assessment by Electrical Engineer
- New generator

PLC/SCADA System

• SCADA programming to incorporate new equipment

Plant Water System

- New plant water system
 - Package plant water duplex pump skid

Site Fence

• Widen site entrance and gate

Pavement

- Cold planning existing pavement
- 3" of new pavement

Non-Monetary Considerations

- Flood protection against 500-year flood
- Conservation of Town water by implementation of a plant water system
- Improved site access for chemical deliveries

Cost Estimate

A preliminary opinion of probable construction cost for proposed site improvements is provided in Table 5-33 on the following page. A detailed breakdown of this opinion of probable construction cost is

provided in Appendix F.

Table 5-33 Proposed Site Improvements – Construction Cost Estimate

Item	Cost ¹
Flood Mitigation – Flood Doors	\$22,100
Plant Water System	\$138,000
Entrance Gate Replacement and Modifications to Fence	\$25,000
Pavement Restoration	\$56,800
Facility-Wide Electrical Assessment	\$25,000
Yard Process Piping & Electrical/Instrumentation Conduit & Wiring Allowance	\$100,000
SCADA Programming Allowance	\$50,000
New generator	\$52,000
Capital Cost Subtotal	\$468,900
Contractor Mark-Up ²	\$102,000
Total Construction Cost (rounded) ^{3,4}	\$571,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.16. Collection System Improvements

Description

The recommended upgrades include maintenance to the collection system include the following:

- Recommendation to clean the following 15 manholes are cleaned:
 - o BK 01
 - o BR 02
 - o BR 06
 - CH 01
 - CH 05
 - o ML 01
 - NM 05
 - o NM 08
 - o RR 02
 - o SM 01
 - o TD 06
 - o TD 08
 - o TH 06
 - o VG 01
 - WW 07
- Recommendation to perform maintenance on the following 6 manholes:

Richmond Wastewater Treatment Facility 20-Year Evaluation Section 6 – Conclusions and Recommendations

- ES 01 Ring and frame replacement
- NM 04 Replace bricks in riser
- NM 05 Replace bricks in riser
- PL 02 Ring and frame replacement
- TD 06 Ring and frame replacement
- VG 02 Reset frame and cover

Cost Estimate

A preliminary opinion of probable construction cost for recommended collection system improvements is provided in Table 5-34. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Table 5-34 Collection System Improvements – Construction Cost Estimate

Item	Cost ¹
Manhole Cleaning	\$37,500
Manhole Ring and Frame Replacement	\$10,400
Manhole Refurbishment	\$5,200
Capital Cost Subtotal	\$53,100
Contractor Mark-Up ²	\$12,000
Total Construction Cost (rounded) ^{3,4}	\$65,000

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.6.17. Bridge Street Pump Station

Design Criteria

The recommended upgrade includes full replacement of the existing pump station with a submersibletype pump station with separated valve vault due to the existing pump station being at the end of its useful life. Recommendations include replacing the existing force main under the bridge with a new insulated force main hat is located at a higher elevation under the bridge deck following damage sustained by the force main bridge crossing during the December 2023 flooding event.

Cost Estimate

A preliminary opinion of probable construction cost for proposed site improvements is provided in Table 5-35 on the following page. A detailed breakdown of this opinion of probable construction cost is provided in Appendix F.

Item	Cost ¹
Site	\$122,000
Valve Vault	\$124,000
Wet Well and Pumping Equipment	\$379,000
Electrical, includes Stand-by Generator	\$160,000
Bridge Force Main Crossing	\$90,000
Capital Cost Subtotal	\$875,000
Contractor Mark-Up ²	\$193,000
Total Construction Cost (rounded) ^{3,4}	\$1,068,000

Table 5-35 Bridge Street Pump Station and Force Main Replacement – Construction Cost Estimate

Notes:

- 1. ENR Construction Cost Index = 13514.76 (December 2023)
- 2. Contractor Mark-Up is inclusive of Contractor's overhead and profit (15%), mobilization and demobilization (5%), and bonds (2%).
- 3. Total Construction Costs do not include engineering services, legal and administrative costs.
- 4. Total Construction Cost does not include contingency.

5.7 Schedule

The proposed project schedule for the Richmond WWTF Upgrade project is presented in Table 5-36 on the following page.

Task	Start	End	Duration
Pre-Design Phase			
Bridge Street Pump Station PER Amendment	5/1/2024	8/29/2024	120
Public Outreach & Funding Assistance	5/1/2024	11/5/2024	189
Bond Vote	11/5/2024		
Step II Final Design			
Execute Final Design Agreement	1/2/2025	3/13/2025	70
Prepare Environmental Report	8/29/2025	10/23/2025	55
Survey	5/30/2025	7/29/2025	60
Wetlands Survey	5/30/2025	7/29/2025	60
Geotechnical Borings & Analysis	5/30/2025	8/28/2025	90
Prepare Basis for Final Design WWTF	9/12/2025	11/11/2025	60
Prepare 30% Drawings & Specs	11/12/2025	4/1/2026	140
Prepare 60% Drawings & Specs	4/1/2026	8/29/2026	150
Prepare 90% Drawings & Specs	8/29/2026	1/6/2027	130
Prepare Bid Documents	1/6/2027	2/15/2027	40
Step III Construction			
Bid Advertisement	3/16/2027	4/30/2027	45
Bid Review & Award	4/30/2027	5/30/2027	30
Notice to Proceed Issued, Execute Agreement	5/30/2027		
Contractor Mobilization	5/30/2027	6/29/2027	30
Initial Shop Drawing Prep	6/29/2027	8/28/2027	60
Shop Drawing Review & Approval	8/28/2027	9/27/2027	30
Major Equipment Lead Time	9/27/2027	1/25/2028	120
Overall Construction Period	5/30/2027	11/30/2028	550
1-Year Warranty Period	11/30/2028	11/30/2029	365

Table 5-36 Richmond WWTF Upgrade Schedule

5.8 Total Project Cost

Total construction costs are grouped into the following four upgrade categories:

- 1. Collection System Upgrades
- 2. Flood Mitigation Upgrades
- 3. WWTF Upgrade
- 4. Septage Receiving Facilities Upgrade

An opinion of probable construction cost for each unit process to be upgraded is presented in Sections 4.1 through 4.18 and includes contractor markups (overhead, profit, mobilization, demobilization, bonds and insurance) and a 30% contingency. Detailed cost estimates for each unit process are presented in Appendix G. The total project costs presented in Table 5-37 for the recommended projects include construction, engineering, surveying, geotechnical investigations, permitting, legal and administrative fees and are escalated to 2027 dollars using the ENR Construction Cost Index historical indices. A summary of total project costs for the Richmond WWTF Upgrade is presented in Table 5-37.

Table 5-37	Richmond	WWTF	Upgrade	Total I	Proiect	Cost
10010 0 07			- PO			

Process Area	E	Costs ¹ NR = 13514.76	Projected Costs ² ENR = 15800 (April 2027)		
Collection System Ungrades		(Dec 2023)		(April 2027)	
Collection System Repabilitation and Maintenance	¢	65.000	¢	76.000	
Collection System Kenabilitation and Maintenance	ې د	65,000 65,000	¢	76,000	
	Ş	05,000	Ş	78,000	
Influent Rumping Lingrade	ć	100 000	ć	571.000	
Hoodworks Ungrado Alt 2 Multi rako Scroon	ې د	400,000	ې د	1 024 000	
Apovic Selector Lingrade Alt. 2. Comproseed Gas Mixing	ې د	1,034,000	ې د	1,934,000	
Riological Process Lingrade	ې د	162,000	ې د	5/1 000	
BIOlogical Process Opgrade	ې د	402,000	ې د	67,000	
WAS Dumping System Ungrado	ې د	95,000	ې د	112,000	
Filtration System Ungrade	ې د	217 000	ې د	254 000	
	ې د	217,000	ې د	254,000	
Effluent Flow Measurement	ې د	15,000	ې د	18 000	
Operations Building	ې د	686.000	ې د	802.000	
Sito	ې د	545,000	ې د	638,000	
	ې د	4 531 000	ې د	538,000 5 301 000	
Flood Mitigation Ungrado	Ş	4,521,000	Ş	5,291,000	
Pridge Street Dump Station Ungrade	ć	1.069.000	ć	1 240 000	
Effluent Dump Station	ې د	1,068,000	ې د	1,249,000	
	ې د	314,000	ې د	31,000	
	ې د	26,000	Ş	31,000	
Flood Mitigation Upgrade Subtotal		1,608,000	Ş	1,881,000	
Septage Receiving Facilities Upgrade	<u> </u>	074.000	~	4 9 4 9 9 9 9	
Septage Receiving Unit Upgrade	Ş	8/1,000	Ş	1,019,000	
Septage & Sludge Storage Alt. 2 - Compressed Gas Mixing,	~	004 000	~	000 000	
Transfer Pumps, Odor Control	Ş	821,000	Ş	960,000	
Dewatering Facilities Upgrade	\$	2,137,000	\$	2,499,000	
Septage Receiving Facilities Upgrade Subtotal	Ş	3,829,000	Ş	4,478,000	
Construction Cost Subtotal ¹	\$	10,023,000	\$	11,726,000	
Engineering & Construction Contingency @ 30%	Ş	3,007,000	Ş	3,518,000	
Total Construction Cost ¹	\$	13,030,000	\$	15,244,000	
Engineering Costs					
Preliminary Engineering - Step I ³		\$154,000		\$154,000	
Pre-Design Phase (Survey, Geotechnical, Pre-Procurement,					
Funding Assistance)	\$87,000 \$8		\$87,000		
WWTF Upgrade Final Design - Step II ⁴	\$385,000 \$385		\$385,000		
Flood Mitigation Final Design - Step II ⁴		\$210,000		\$210,000	
Septage Upgrade Final Design - Step II ⁴		\$326,000		\$326,000	
Bid, Construction Administration & Inspection - Step III ⁴		\$1,245,000		\$1,456,000	
Legal, Administrative, Permitting 0.5%		\$65,000		\$76,000	
Total Project Cost		\$15,502,000		\$17,938,000	

Notes:

1. ENR Construction Cost Index = 13514.76 (December 2023)

2. ENR Construction Cost Index projected April 2027 – 15800

3. Executed contract dated 3/21/2023

4. Engineering Fee is calculated based on percentage of construction value and past project experience

5.9 Sustainability Considerations

5.9.1. Water and Energy Efficiency

Hoyle Tanner will work with Efficiency Vermont, Green Mountain Power, and the Town of Richmond during the design phase to identify opportunities to incorporate energy efficient design into the project.

Where feasible, existing HVAC systems for the existing buildings will be replaced with more efficient, more environmentally friendly systems that do not rely on fossil fuels, including heat pump and effluent heat exchange technology.

For water efficiency, equipment that can will be designed to use plant water instead of potable water. Low flow plumbing fixtures will be incorporated into the mechanical design to improve water use efficiency.

Energy efficiency will be incorporated into other aspects of design, where applicable, such as controlling the blowers based on dissolved oxygen measurements to avoid over-aeration, use of VFD's and level control on influent pump to optimize pumping, flow pacing the UV system with UV intensity to maximize energy savings and replacing conventional mixing with compressed gas mixing in the sludge holding tanks and anoxic zones to reduce energy consumption.

5.9.2. Green Stormwater Infrastructure

Implementation of green stormwater infrastructure is not anticipated as part of this project due to the limited size of the WWTF site.

5.10 Annual Operating Budget

The current fiscal year FY2023 budget and proposed FY2024 budget for the Richmond WWTF are presented in Appendix C. The following provides a summary of the budgeted annual operating budget for Richmond's Wastewater Department in FY2023 and FY2024.

5.10.1. Income

In fiscal year FY2023, the Town of Richmond has \$872,220 in annual department revenue budgeted. The proposed FY2024 budget estimates annual revenue to be \$857,874.

5.10.2. Annual O&M Costs

The fiscal year FY2023 annual O&M costs budgeted for the wastewater department are \$872,220. The budgeted fiscal year FY2024 budget estimates annual O&M costs to be \$857,874.

5.10.3. Debt Repayments

In fiscal year FY2023, the Richmond Wastewater Department budgeted an annual payment of \$77,274 on loans. In fiscal year FY2024, the Richmond Wastewater Department budgeted an annual payment of \$76,439.

5.10.4. Reserves

The Richmond Wastewater Department maintains a Wastewater Capital Reserve, a Short-term capital fund, and a Collection System Capital Fund. In fiscal year FY2023, \$70,000, \$50,000, and \$10,000 were budgeted to go to each fund, respectively. In fiscal year FY2024, \$10,000, \$10,000, and \$0 are budgeted to go to each fund, respectively.

6. Conclusions & Recommendations

In conclusion, the following next steps are recommended to advance the recommended upgrades of the Richmond WWTF and Bridge Street Pump Station:

- Complete Preliminary Engineering Report Amendment to 20-year Evaluation for the Bridge Street Pump Station and Force Main
- Complete Bond Vote
- Initiate Step II Final Design for WWTF Upgrade Project
- Complete survey at WWTF and Bridge Street Pump Station sites
- Complete geotechnical investigation, as needed, for WWTF and Bridge Street Pump Station sites
- Complete Basis for Final Design for Recommended WWTF Project
- Complete Environmental Report

APPENDIX A FIGURES











FIGURE A-2

DWLRODO ORRGEDUGDHU) 51 WWH



HHOG





Natural Resources Atlas

Vermont Agency of Natural Resources



VERM ONT





Richmond WWTF - Endangered Species

Vermont Agency of Natural Resources

vermont.gov

VERM ONT

FIGURE A-5





2_102601_00 - FIGS.D



NOTES:

- 1. SURFACE WATER ELEVATION (SWE) BASED ON MAXIMUM FLOW OF 1.14 MGD THROUGH ONLY ONE OF EACH DUPLICATE UNIT AS PER RECORD DRAWINGS BY WEBSTER-MARTIN, INC., JULY 1971.
- 2. HEADLOSS ACROSS SCREEN UNIT BASED ON CALCULATIONS BY LAKESIDE EQUIPMENT CORPORATION AT 2.97 MGD.
- 3. HEADLOSS AT EFFLUENT WEIR OF GRIT REMOVAL PACKAGE UNIT BASED ON CALCULATIONS BY LAKESIDE EQUIPMENT CORPORATION AT DESIGN PEAK HOURLY FLOW OF 1.152 MGD.
- 4. DESIGN PEAK HOURLY FLOW OF 1.152 MGD
- 5. HEADLOSS THROUGH FILTER UNIT BASED ON CALCULATIONS BY AQUA-AEROBICS SYSTEMS, INC. AT 3254 GPM THROUGH ONE FILTER UNIT.
- 6. HEADLOSS THROUGH UB UNIT BASED ON CALCULATIONS BY TROJAN TECHNOLOGIES, INC. AT PEAK HOURLY FLOW OF 1.0 MGD.
- 7. PEAK HOURLY FLOW OF 1.0 MGD.



APPENDIX B

FOURNIER ROTARY PRESS INSPECTION REPORT 9/30/2023



Richmond Vermont Visit Summary

Introduction

The purpose of this visit was to inspect the condition of the two channel Rotary Press located in Richmond Vermont and provide operational assistance to the personnel. The press has an estimated 45,000hrs on it. This is a two 36" channel press, model number 2-900/2000, and serial number PR-09-0/99.

Inspection

Screens: The facility only runs sludge through one channel and has recently complained about the filtrate quality. They do not run on the other channel due to damaged screens. Opening the restrictor arms on both channels and cleaning the channels out to inspect, I discovered that all four screens were very worn.

Frames: The frames on the channels were rusting and worn were the restrictor arms moved. You could see the yellow cover seal between the wheel and the frame starting to come through with trash.



Shaft: The shaft of the press was in ok condition. The shaft only supports one channel on each side, so there was limited exposure to the atmosphere. There is sone rust around on the end, however that should not effect the removal.



Three-way valve: The existing three-way valve does not work. It has been stuck in the dewatering position and does not turn. This causes problems with a proper startup. Sludge also leaks out of the supply tank, through the sludge pump and, because the valve is stuck in the dewatering position, into the channels.



Floccualtor: The flocculator seal shaft has been leaking for some time. It's hard to tell the extent of the damage.



September 30th, 2023

Panel: The panel is the old style with speed dials for the flocculator speed and rotary press speed. The outlet pressure is controlled directly at the channel. If you adjust it on the panel, it doesn't do anything.



Channel Wash Manifold: The Festo valves on the channel wash manifold do not seem to work. The piping and spray bars are not in good condition.



Sludge Pump: The facility has a double disc sludge pump and is in good condition. However, the sludge tank gravity feeds past the pump when it is in the off position.



Polymer System: The polymer system does not have an indication of concentration. A batch of diluted polymer is sent to a 50-gallon tank. From there it is pumped to the floccuator at a ratio of the sludge flow.

Conveyor System: The conveyor seems to be in good condition apart from a couple rusted out holes in the lower one. Also, on the inclined conveyor it is open at the top. The employees said this was from cake building up due to an unopened slide gate that has since been fixed.

Operation

The operators at the facility are operating the press as best as possible for the condition it is in. During a normal startup the three-way valve sends sludge down the drain, in recirculation, this gives the operator time to determine that they have a good flocculation. Because the valve does not work it sends it directly to the rotary press. If the sludge tank is full and the sludge supply valve is open to the pump, it passes the pump and goes into the rotary press without it even on or running. So, when they start instead of recirculating, they hit dewatering and it goes right to the rotary press. The first sludge the press receives is not flocculated. Once it starts to flocculate, the one operating channel begins to produce cake. However, a lot of solids are going through





September 30th, 2023

September 30th, 2023

the worn screens and down the filtrate. Because they are running a blend of digested and septic sludge the press is still able to produce some cake. I believe if they were just running digested sludge, they would have a hard time getting anything to come out of the channel. The only way to keep sludge producing cake is to treat it gently. If they produce high pressure inside the channel sludge comes out the screens instead of out the front as cake. The key is low outlet pressure 5psi, low inlet pressure 1.2psi, and moderate rotary press speed 30%. This minimizes the sludge in the filtrate being sent to the head of the plant. The press still produces 25-30% cake dryness with a flow of 20-30gpm through the channel. Sludge total solids was 1.45% Polymer consumption was 26 active lbs/dry ton.

Observations and Conclusion

The Richmond, Vermont facility needs:

- New Channels Everything on the channels needs replacement so I would recommend whole new channels. Even if you could spare some parts, we might need to cut the wheels from the shaft.
- 2. New Flocculator Assembly The top of the floccuator need to be dismantled and probably everything needs to be replaced below the gearbox.
- 3. Three-way Valve I would suggest they replace them with two two-way valves.
- 4. Channel Wash Valves If it doesn't already come with the new channels.
- 5. Air supply A new air regulator needs to be installed. As of right now I do not believe they have air going to the three-way valve and they only have it to the bellows.

Possible upgrades needed:

- Sludge pump The sludge supply tank feeding the pump is gravity feeding through it. The pump may just need new interior parts. Or the sludge valve needs to be closed off when the press is not running.
- Polymer system We determined the polymer was making down a batch of 0.25% concentration, with a polymer to sludge ratio of 8% and a consumption of 26 active lbs/dry ton. An upgrade of the polymer system could produce lower consumption.

- 3. Panel HMI The installation of a new panel would make it easier for the operators to use the press. The current old program is outdated and not optimal.
- 4. Conveyor system There are a couple holes that need to be patched or sections replaced.



APPENDIX C

FY24 WATER & SEWER BUDGET

FINAL Water FY24

		Budgeted	Actual	Budgeted	Budgeted	+INCREASE
Account #	Description	FY22	FY22	FY23	FY24	(DECREASE)
WATER REVENUE						
20-6-00-3-00.00	Water User Receipts	326,560	333,594	317,547	320,384	0.89%
20-6-00-3-01.00	Sale of Water from Hydrant	1,500	2,046	1,500	1,500	0.00%
20-6-03-5-40.05	Net Interest on Checking Account	500	856	500	6,000	1100.00%
20-6-00-4-10.02	Hook On Fees – Water	500	250	500	500	0.00%
20-0-00-0-00.00	Fund Balance Usage	-	-	27,339	-	-100.00%
20-6-10-4-10.04	Fire Service Fees	50,432	50,432	51,148	49,899	-2.44%
	Water Revenue Sub Totals	379,492	387,178	398,534	378,283	-5.08%
WATER RESOURCES	ADMINISTRATION EXPENSES (30% of total)					
20-7-80-0-10.00	Salaries	63,593	65,885	(1,211	83,002	7.41%
20-7-80-0-10.30	Insurance Opt Out	1,500	1,500	1,500	-	-100.00%
20-7-80-0-10.99	Overtime	900	3,135	2,400	2,400	0.00%
20-7-80-0-11.00	Social Security/Medicare	5,081	5,168	6,181	6,576	6.39%
20-7-80-0-12.00	Municipal Retirement	4,031	5,624	5,318	5,765	8.41%
20-7-80-0-15.00	Health Insurance	7,372	8,299	15,023	19,310	28.54%
20-7-80-0-15.01	Health Savings Account	1,248	420	458	313	-31.66%
20-7-80-0-15.03	Long Term Disablity	420	408	407	570	40.05%
20-7-80-1-16.00	Uniforms	400	164	400	400	0.00%
20-7-80-1-20.00	Office Supplies/Postage	300	296	300	670	123.33%
20-7-80-1-22.00	Office Equipment	200	155	200	200	0.00%
20-7-80-1-22.01	Computer	-	69	-	450	100.00%
20-7-80-1-22.02	Computer Support	-	646	1,///	1,800	1.29%
20-7-80-1-24.00	Advertising	200	-	200	200	0.00%
20-7-80-1-26.01	Administrative Expense	9,000	9,000	9,000	12,450	38.33%
20-7-80-1-26.03	Audit Expenses	6,673	3,030	1,305	1,450	11.11%
20-7-80-1-27.00	Staff Training/Education/Licenses	800	956	800	1,300	62.50%
20-7-80-1-27.01	Safety Training	100	-	100	100	0.00%
20-7-80-1-29.00	Travel	300	-	300	300	0.00%
20-7-80-1-30.00	Telephone	2,500	1,527	2,500	2,500	0.00%
20-7-80-1-42.00	Association Dues	200	140	200	200	0.00%
20-7-80-1-43.00	Legal		664		500	100.00%
20-7-80-1-48.00	W & S General Insurance	8,498	7,023	5,782	6,300	8.96%
	Water Administration Expense Totals	113,316	114,109	131,428	146,756	11.66%
WATER OPERATIONS	EAFENSES	500	244	500	500	0.00%
20-7-83-4-16.00	Personal Protective Equip	500	244	500	500	0.00%
20-7-83-4-31.00	Heat	600	626	600	600	0.00%
20-7-83-4-32.00		8,500	10,799	8,500	10,100	18.82%
20-7-83-4-34.00	I rash Removal	800	2,117	800	1,500	87.50%
20-7-83-4-41.00	System Permits/Fees/Licenses	1,900	1,293	1,900	1,900	0.00%
20-7-83-4-45.00	Water Contracted	5,000	3,041	5,000	4,000	-20.00%
20-7-83-4-45.02		500	220	500	500	0.00%
20-7-83-4-46.00	Engineering	2,000	145	2,000	1,000	-50.00%
20-7-83-4-50.00	Gas, Oll & Diesel Fuel	500	227	500	500	0.00%
20-7-83-4-52.00		1,000	46	1,000	1,000	0.00%
20-7-83-4-62.02	Water Line	20,000	2,261	20,000	15,000	-25.00%
20-7-83-4-62.03	rumps/ranks	5,000	5,251	5,000	5,000	0.00%
20-7-03-4-02.04	Asphalt Repair	5,000	-	5,000	5,000	0.00%
20-7-83-4-62.05	Equipment Purchase	500	33	500	500	0.00%
20-7-03-4-02.00	Supplies	1,000	96	1,000	1,000	0.00%
20-1-03-4-02.01	Weter Treetment Chemicale	3,000	53	3,000	3,000	0.00%
20-1-83-4-65.00		1,000	//9	1,000	2,600	160.00%
	Water Operating Expense Totals	56,800	27,831	56,800	53,700	-5.46%

WATER CAPITAL EXP	ENSES					F	23 Predicted Year End Balances
20-7-90-5-93.01	Water Capital Reserve	36,000	36,000	36,000	-	-100.00%	115,991
20-7-90-5-90.03	Short-term (10 yr) capital fund	20,000	20,000	20,000	20,000	0.00%	126,651
20-7-90-2-90.09	Distribution System Capital fund	15,000	15,000	15,000	20,000	33.33%	21,100
20-7-90-2-90.16	Water Reservoir gap principal (2025)	25,857	25,857	25,857	25,857	0.00%	263,742
20-7-90-2-90.17	Water Reservoir gap interest	1,975	1,482	1,482	990	-33.20%	
20-7-90-5-90.01	RF3-302 Water Reservoir principal (2048)	37,705	37,705	37,705	37,705	0.00%	
20-7-90-5-93.02	RF3-335 East Main principal	25,140	25,140	25,140	25,140	0.00%	
20-7-90-2-90.07	Jericho Road Loan Principal (2032)	26,208	26,208	26,208	26,208	0.00%	
20-7-90-2-90.08	Jericho Road Loan Interest	11,491	11,491	10,549	9,562	-9.36%	
20-7-90-5-90.13	RF3-365 Bridge Upper & Crossing Principal (2047)	10,000	9,865	9,865	9,865	0.00%	
	RF3-444 Bridge Street Middle (2062)	-	-	2,500	2,500	0.00%	
20-7-90-1-00.00	Unbudgeted Capital Expense			-	-	0.00%	
	Water Capital Expense Totals	209,376	208,748	210,306	177,827	-15.44%	
		370 402	387 178	308 534	378 283	E 0.99/	
		575,452	307,170	390,334	570,205	-5.08%	
	TOTAL WATER EXPENSES	379,492	350,688	398,534	378,283	-5.08%	
	BALANCE	-	36,490	-	-		

UNASSIGNED FUNDS FY22 YEAR END AUDIT	(64,135)
UNASSIGNED FUNDS FY23 USAGE/GROWTH	255,145
PREDICTED UNASSIGNED FUNDS YEAR END FY23	191,010
DRAFT FY24 WATER EXPENSES AS OF 05/01/2023	378,283
15% OF FY24 BUDGET EXPENSES	56,742
UNASSIGNED FUNDS IN EXCESS OF 15%	134,268

Available Unassigned funds & Total FY23 Reserve Funds	
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398,010

FINAL Wastewater FY24

Account #	Description	Budgeted FY22	Actual FY22	Budgeted FY23	Budgeted FY24	+INCREASE (DECREASE)
WASTEWATER REVENUE					-	
21-6-00-3-00.01	Sewer User Receipts	357,337	373,213	361,326	292,874	-19%
21-6-00-4-10.03	Hook On Fees – Sewer	1,000	3,213	1,000	1,000	0%
21-6-03-5-40.05	Net Interest on Checking Account	1,200	1,998	1,500	14,000	833%
21-0-00-0-00.00	Fund Balance Usage	-	-	48,394	-	-100%
21-6-01-4-11.10	Septage Receipts	430,000	483,577	460,000	550,000	20%
	Waste Water Revenue Subtotal	789,537	862,001	872,220	857,874	-2%
WASTEWATER RESOURCES	ADMINISTRATION EXPENSES (70% of total)					
21-7-80-0-10.00	Salaries	148,381	153,731	180,312	199,270	11%
21-7-80-0-10.30	Insurance Opt Out	3,500	3,500	3,500	-	-100%
21-7-80-0-10.99	Overtime	2,100	7,314	5,600	5,600	0%
21-7-80-0-11.00	Social Security/Medicare	11,857	12,058	14,423	15,344	6%
21-7-80-0-12.00	Municipal Retirement	9,405	13,146	12,408	13,451	8%
21-7-80-0-15.00	Health Insurance	17,201	19,363	35,053	45,056	29%
21-7-80-0-15.01	Health Savings Account	2,913	980	1,070	731	-32%
21-7-80-0-15.03	Long Term Disablity	960	953	949	1,330	40%
21-7-80-1-16.00	Uniforms	900	355	900	900	0%
21-7-80-1-20.00	Office Supplies/Postage	500	691	500	500	0%
21-7-80-1-22.00	Office Equipment	400	281	400	400	0%
21-7-80-1-22.01	Computer	-	2,299	-	1,050	100%
21-7-80-1-22.02	Computer Support	-	308	4,145	4,200	1%
21-7-80-1-24.00	Advertising	400	-	400	400	0%
21-7-80-1-26.01	Administrative Expense	21,000	21,000	21,000	29,050	38%
21-7-80-1-26.03	Audit Expenses	6,237	7,071	3,045	3,383	11%
21-7-80-1-27.00	Employee Training/Education/Licenses	1,800	739	1,800	2,770	54%
21-7-80-1-27.01	Safety Training	300	-	300	300	0%
21-7-80-1-29.00	Travel	700	-	700	700	0%
21-7-80-1-30.00	Telephone	3,800	3,548	3,800	3,800	0%
21-7-80-1-42.00	Association Dues	400	326	400	400	0%
21-7-80-1-43.00	Legal	3,000	2,856	3,000	3,000	0%
21-7-80-1-48.00	W & S General Insurance	15,868	13,958	11,341	13,000	15%
	Wastewater Administration Expense Subtotal	251,622	264,477	305,046	344,635	13%

WASTEWATER OPERA	ATIONS EXPENSES						
21-7-82-2-32.01	Electricity	1,000	-	1,000	1,000	0%	
21-7-82-2-62.03	Pump Station Maintenance	1,800	-	1,800	1,800	0%	
21-7-82-3-16.00	Personal Protective Gear	500	982	500	500	0%	
21-7-82-3-31.00	Heat	10,000	8,400	10,000	10,000	0%	
21-7-82-3-32.00	Plant Electricity	40,000	38,747	40,000	45,000	13%	
21-7-82-3-32.02	WWTF water bill	32,000	31,515	32,000	32,000	0%	
21-7-82-3-34.00	Rubbish Removal	1,500	4,213	1,500	4,300	187%	
21-7-82-3-41.00	System Permits/Certs/Licenses	800	1,128	800	800	0%	
21-7-82-3-45.00	Wastewater Contracted	7,500	5,339	7,500	7,500	0%	
21-7-82-3-45.01	Biosolids Contracted	4,500	5,994	4,500	4,500	0%	
21-7-82-3-45.02	Equipment Rental	500	70	500	500	0%	
21-7-82-3-45.03	Biosolids Disposal/CSWD	120,000	153,967	130,000	160,000	23%	
21-7-82-3-46.00	Engineering	500	2,281	500	500	0%	
21-7-82-3-50.00	Gas, Oil & Diesel Fuel	1,800	735	1,800	1,800	0%	
21-7-82-3-52.00	Fleet Maintenance	2,500	188	2,500	2,500	0%	
21-7-82-3-62.00	Wastewater Facil Repair	8,000	19,672	8,000	12,000	50%	
21-7-82-3-62.01	Biosolids Facility Repair	8,000	14,003	8,000	9,000	13%	
21-7-82-3-62.02	Collection System Repair	4,000	1,222	4,000	4,000	0%	
21-7-82-3-65.00	Wastewater Chemicals	10,000	7,947	10,000	17,000	70%	
21-7-82-3-65.01	Biosolids Chemicals	70,000	94,220	90,000	98,100	9%	
21-7-82-3-66.00	Supplies	5,000	3,076	5,000	4,000	-20%	
	Wastewater Operating Expense Subtotal	329,900	393,699	359,900	416,800	16%	
WASTEWATER CAPITA	AL EXPENSES						FY23 Predicted Year End Balances
21-7-90-5-93.00	Wastewater Capital Reserve	70,000	70,000	70,000	10,000	-86%	156,595
21-7-90-5-93.04	Short-term (10 yr) capital fund	50,000	50,000	50,000	10,000	-80%	158,305
21-7-90-5-93.11	Collection System Capital Fund	10,000	10,000	10,000	-	-100%	468,056
21-7-90-2-90.01	RFL-101 planning-ww (2027)	12,081	12,021	12,081	12,021	0%	782,956
21-7-90-2-90.06	Project 7a Sanitary Sewer (2032)	14,093	14,093	14,093	14,093	0%	
21-7-90-2-90.02	Phosphorus SRF(2026)	22,220	22,220	22,220	22,220	0%	
21-7-90-2-90.14	Jericho Rd Loan Principal (2032)	20,592	20,592	20,592	20,592	0%	
21-7-90-2-90.16	Jericho Rd Loan Interest	9,029	9,029	8,288	7,513	-9%	
21-7-82-1-00.00	Unbudgeted Capital Expense			-	-	0%	
	Wastewater Capital Subtotal	208,015	207,955	207,274	96,439	-53%	
	TOTAL WASTEWATER REVENUE	789,537	862,001	872,220	857,874	-2%	
		790 527	966 131	872 220	957 974	-20/	
		109,551	000,131	072,220	057,074	-2 /0	
	BALANCE	-	(4,130)	-	-		
			(.,)				
UNASSIGNED FUN	IDS FY22 YEAR END AUDIT				382,021		
UNASSIGNED FUN	IDS FY23 USAGE/GROWTH		159.314				
			5/1 335				
TREDICTED UNAS	SIGNED I OND DALANCE TEAR END FT23		—	J+1,333			
					057.074		
DKAFT FY24 SEWE		857,874					
15% OF FY 24 BUD	DGET EXPENSES				128,681		
UNASSIGNED FUNDS IN EXCESS OF 15%							

UNASSIGNED FUNDS IN EXCESS OF 15%

Available Unassigned funds & Total FY23 Reserve Funds



Water Budget - Fire Protection Calculation

48% Tank loan	37,705	0.48	18098
48% Gap loan	26,847	0.48	12887
5% Total Water Budget	378,283	0.05	18914
		_	49899

Proprietary Net Position Unrestricted				FY23 Projections are ONLY for Reserve funds					
						(see previous sheets for unassigned fund projections)			
	FY18	FY19	FY20	FY21	FY22	FY23	FY23	FY23	FY23
						Usage Budgeted	Contribution	Usage as of 04/30/23	Predicted Year-End
Water Audit Unrestricted	298,778	243,190	(58,859)	349,163	156,230				
Water Reserves									
Short Term Capital	40,000	55,575	75,270	95,270	114,714	0	20,000	(18,722)	115,991
Water Capital	85,817	21,070	53,742	66,359	90,651	0	36,000	0	126,651
Distribution	63,265	52,702	57,430	(11,708)	15,000	0	15,000	(8,900)	21,100
Total Reserves	189,082	129,347	186,442	149,921	220,365	0	71,000	(27,622)	263,743
Water Audit Unrestricted minus Reserves	109,696	113,843	(245,301)	199,242	(64,135)				
Sewer Audit Unrestricted	471,819	587,860	781,772	974,341	1,048,217				
Sewer Reserves									
Wastewater Capital	197,761	331,572	355,769	388,056	92,755	0	70,000	(6,160)	156,595
Short Term Capital	30,682	53,522	85,496	117,479	115,385	0	50,000	(7,080)	158,305
Collection System	36,735	78,405	87,630	82,755	458,056	0	10,000	0	468,056
Total Reserves	265,178	463,499	528,895	588,290	666,196	0	130,000	(13,240)	782,956
Sewer Audit Unrestricted minus Reserves	206,641	124,361	252,877	386,051	382,021				
Water and Wastewater Items Identified for Repair or Replacement in FY23 and FY24

Water		Outside Consultant	Estimated Cost		Fiscal Year
	Wire and calibrate PH meter at water house.	Tom Allen	TBD		FY23 or FY24
	Water tank mixer replacement		\$	20,000	FY24
	Water tank cleaning		\$	9,000	FY24
	Repair common alarm	Tom Allen	TBD		FY23 or FY24
	Excavation to locate Borden St. water valve		\$	10,000	FY23
Wastewate	er				
	Repair pager dialer	Tom Allen	TBD		FY23 or FY24
	Replace check valve for pump station	Phil Laramie	TBD		FY23
	Repair meters for hours of operation on each pump	Dan Pratt	TBD		FY23
	Rebuild backwash pump 1	Phil Laramie	\$	5,000	FY23
	Rebuild backwash pump 2	Phil Laramie	\$	5,000	FY24
	Purchase New backwash pump		TBD		FY23
	Grit motor assessment and repair	Dan Pratt	TBD		FY23
	Replace auger liner	Phil Laramie	TBD		FY23
	Replace air valve on dewatering press		TBD		FY23
	Repair hazardous gas alarm	Phil Laramie	TBD		FY23
	Purchase UV meter		\$	2,128	FY23
	Install UV meter	Tom Allen	TBD		
	Clean and repair aeration tanks and all holding tanks	Obtaining Quotes	\$30,000 - \$50,000		FY23 or FY24
	Purchase meter for septage receiving	Obtaining Quotes	TBD		FY23
	Wastewater Mixer		\$	8,500	FY23
	New Influent Pumps	Rough Estimate	\$	60,000	FY24

Prioritize projects.

Use Unassigned funds dows to 15% of the FY24 budgeted expenses. Then use Reserve Funds down to zero. Then back to unassigned funds only if the project is criticle.

APPENDIX D BIOLOGICAL MODEL



Richmond WWTF Biological Model Summary

A biological model was created for the Richmond WWTF with the goal of defining updated influent design criteria for the existing facility. Currently, Richmond WWTF has a historical average daily flow of 0.073 MGD, which is 33% of their permitted flow of 0.222 MGD. The past five years of data revealed that Richmond experiences the following influent loads:

Table 1. Historical Influent Loading – Richmond WWTF

Parameter	Current Loading (@ 0.073 MGD)					
Biochemical Oxygen Demand (BOD)	462 lb/day					
Total Suspended Solids (TSS)	681 lb/day					

Typically, Richmond has one (1) aeration basin and one (1) secondary clarifier in operation and can effectively treat the historical influent loading. In order to determine the treatment capacity of the existing facility at permitted flow, a biological model was created.

The model was first run using the historical average daily flow (0.073 MGD) and historical influent concentrations. Operators collected three (3) samples from the aeration basins and tested them for total suspended solids. These TSS concentrations were averaged to estimate the current MLSS in the aeration basins of 5,993 mg/L. Equipped with this existing data, a current solids retention time (SRT) of ~16 days was calculated.

The model was then run using the permitted flow (0.222 MGD) and was set to maintain the existing SRT of 16 days. The capacity of the existing process was determined by maintaining a solids loading rate (SLR) of 25 gpd/sf. The allowable SLR was determined using the graphs below from TR-16. First, in Figure 1, an sludge volume index (SVI) of 75 mL/g was estimated. Then, in Figure 2, the allowable SLR of 25 gpd/sf was determined based on an SVI of 75 mL/g and a return activated sludge flow per square foot of clarifier surface area (QR/A) of 140 gpd/sf.



Figure 6-3

RAS Rate vs. MLSS Concentration & SVI at Critical Loading



Figure 1. RAS Rate vs MLSS & SVI – TR-16



Figure 2. Secondary Clarifier Operating Diagram Using Unstirred SVI – TR-16



The biological model was then optimized, keeping in mind the following restrictions:

- Maintaining a SRT of ~16 days
- Two aeration basins in service
- One secondary clarifier in service for redundancy
- Allowable SLR of 25 gpd/sf

The model revealed that in order to maintain a SLR of 25 gpd/sf with one secondary clarifier in service at permitted flow, the following influent loads could be properly treated:

Table 2 Ori	iginal & Pronos	ed Influent D)esign Criteria –	Richmond W/W/TE
	igilial & Flopos		esign cincena	

Parameter	Original Design Criteria (@ 0.222 MGD)	Proposed Design Criteria (@ 0.222 MGD)
Biochemical Oxygen Demand (BOD)	600 lb/day	600 lb/day
Total Suspended Solids (TSS)	500 lb/day	780 lb/day

Lastly, it should be noted that VTDEC completed the preliminary reasonable potential analysis and theoretical water quality based effluent limit calculation for Total Ammonia Nitrogen and Total Phosphorus on December 4th, 2023. Given the dilution available in the receiving water, it was determined that there is no reasonable potential detected and, therefore, new limits are not anticipated to be implemented at the Richmond WWTF. For this reason, and for lack of historical influent data, design criteria for influent nitrogen was not determined. Richmond has effectively treated their historical influent nitrogen loads.

Influent Characteristics

Constituent	Value	Units	Notes
COD (Influent)	588.86	mg/L	Based on assumption that BOD is 55% of COD
sCOD	259.10	mg/L	44% of COD
BOD (Influent)	324	mg/L	assumed TSS:BOD ratio from existing conditions, solved from TSS
BOD (Primary Effluent)	323.87	mg/L	No primary clarifiers
sBOD	161.94	mg/L	50% of BOD
TSS	422	mg/L	solved for based on secondary loading
Influent percent of TSS that is VSS	74%		average percent VSS from average of sampling 10/11/2023
Influent VSS that is non-biodegradable VSS	33%		assumed
VSS	312.36	mg/L	
nbVSS	103.08	mg/L	
TP	19.50	mg/L	historical data - may include side streams
TKN	50	mg/L	assumed TSS:TKN ratio from existing conditions, solved from TSS
Xi,OHO	51.54	mg/L	Influent inert volatile solids (use 50% of nbVSS)
Temperature			
Influent Wastewater	8.00	deg C	Assumed
Air	0.00	deg C	Assumed
Flow Bates - MGD			
	0.222	MCD	
Annual Average Flow	0.222	MGD	
RAS	0.15	MGD	historical recirculation rate is 68.5%
h <u> </u>			
Percent RAS Flow	68.5	%	percent of influent
RAS Concentration	8,000	mg/L	assumed
Flow Rates - m ³ /d			
Influent Flow Bate	840.36	m ³ /d	
DAC	5-5.50 F7F CF		
CMA	575.65	m⁻/d	
Activated Sludge and Aeration Parameters			
Activated Sludge Basin Volume (V)	300.000	gal	Two (2) 150,000 gal tapks
Activated Sludge Basin Volume (V)	1 126	gai m2	1W0 (2) 150,000 gai tanks
Activated Siddge Basili Volume (V)	1,130	1115	
Diffuser depth (Dr)	13.20	π	
Diffuser Depth	4.02	m	
Diffuser Efficiency (E)	0.35	-	assumed
Operating DO	2.00	mg/L	assumed
Correction Factors			
alpha	1	-	Aeration system
beta	0.95	-	Aeration system
Fouling factor (F)	0.90	-	Aeration system
Mid depth correction factor (d _e)	0.40	-	
Flovation			
Elevation	307	π	reet above sea level
Elevation	93.57	m	
Secondary Clarifier Parameters			
Total number of clarifiers	2	ea	
SWD	10	ft	
Area, per clarifier	550	sf	
Total Clarifier Area	1,100	sf	
Default Kinetic and Stoichiometric Paramete	ers		
OHOs (COD oxidation)	value	units	temperature-adjusted value
li	6.00	1/d	2 6640718
Pmax .	0.00	±/u	2.0040/10
Ks	8.00	mg COD/L	8
Y	0.45	g VSS/g COD	0.45
b	0.12	1/d	0.074951646
f _d	0.15	-	0.15
κο	0.20	mg O ₂ /L	0.2
θ (umax)	1.07	-	
θ (b)	1.0/	-	
AOBs (NH, oxidation)	1.04	-	
μ _{max}	0.90	1/d	0.390755487
Ks	0.50	mg NH ₄ -N/L	0.5
Y	0.15	g VSS/g NH4-N	0.15
b	0.17	1/d	0.120632527
f	0.15	-	0.15
v	0.15	mg 0 //	0.5
K _O	0.50	mg U ₂ /L	0.5
θ (μ _{max})	1.07	-	
θ (b)	1.03		
NOBs (NO ₂ oxidation)			
H	1.00	1/d	0.480397596
r max	2.00	1/u	0.0000000
n _s	0.20	mg NO ₂ -N/L	0.2
Y	0.05	g VSS/g NO ₂	0.05

Influent Nutrient Loading

τ b f_d Κ₀ θ (μ_{max}) θ (b)

Influent TSS Concentration	422	mg/L	
Influent TSS Load	782	lb/day	
Influent BOD Concentration	324	mg/L	
Influent BOD Load	600	lb/day	
Influent TKN Concentration	50	mg/L	
Influent TKN Load	92	lb/day	

1/d

mg O₂/L

0.05 0.120632527

0.15

0.05 0.17

0.15

0.90 1.06 1.03

Reference Data

Constituent	Value	Units	Notes	
MLSS - Actual	5993	mg/L	from averaging sampling data 10/11/2023	
WAS	0.008	MGD	historial MAC flow, adjusted for an antitud flow.	
WAS	30.64	m ³ /d	nistoricui was jiow, uujusteu joi permitteu jiow	

Maintaining MLSS

Mass of Total Solids in Aeration Basin	5002	kg	kg TSS/d* SRT
MLSS - Calculated	4405	mg/L	calculated
Minimum Aerobic SRT Required	10.98	d	SRT = 10.98 days = 1.5x safety factor
Minimum Total SRT Required	11.42	d	including anoxic selectors
Design Aerobic SRT	15.52	d	SRT = 10.98 days = 1.5x safety factor
Design Total SRT	16.14	d	including anoxic selectors
Total HRT	1.351	d	HRT = V/Q

Nitrate Concentrations

Estimate NO3-N	33.04	mg/L	Adjust this number to match the calculated effluent NO3-N below. For fully aerated, this value is the same as the effluent NO3-N (But this is not true for cases where denirtification is evaluated)
Calculated Effluent NO3-N	33.04	mg/L	$NO_3 = TKN_{inf} - NH_{3eff} - NH_{3Biomass}$

COD Concentrations

bCOD	518.2	mg bCOD/L	Influent biodegradable COD, assumed 1.6*primary effluent BOD	
S _{bCOD}	0.43	mg bCOD/L	Effluent biodegradable COD	
SBOD	0.3	mg BOD/L	Effluent BOD	

Ammonium Concentrations

S _{NH4-N}	1.62	mg NH ₄ -N/L	Effluent NH4-N
S _{NH4-N, Ox}	33.04	mg NH ₄ -N/L	Nitrogen oxidized by AOBs and NOBs

Microbial Activity

Х _{оно,а}	1,259	mg/L	Active OHO Biomass	$X_{OHO,a} = \frac{SRT_{Total}}{HRT} \frac{V_{OHO}(S_0 - S)}{1 + b_{OHO}(SRT_{Total})}$
Х _{оно,і}	844	mg/L	Assumes all Xi in influent is Xi,oho	$X_{OHO,i} = \frac{SRT_{Total}}{HRT} \Big(X_{OHO,i,0} + X_{OHO,a} f_d b_{OHO} (HRT) \Big)$
X _{AOB,a}	19	mg/L	Active AOB biomass	$V = SRT_{Aerobic} V_{AOB}(NH_{3,ox})$
X _{AOB,i}	6	mg/L	Assumes no Xi,aob in influent	$\frac{A_{AOB,a} - \frac{1}{HRT} + b_{AOB}(SRT_{Total})}{SRT_{Total}(u_{transp})}$
X _{NOB,a}	7	mg/L	Activate NOB biomass	$X_{AOB,i} = \frac{1}{HRT} \left(X_{AOB,a} f_{a} b_{AOB} (HRT) \right)$ $SRT_{aarobic} = Y_{NOB} (NH_{3,ov})$
X _{NOB,i}	2	mg/L	Inactive NOB biomass	$X_{NOB,a} = \frac{1}{HRT} \frac{1}{1 + b_{NOB}(SRT_{Total})}$
X _{total}	2,137	mg/L	Sum of all above	$X_{NOB,i} = \frac{SHATOTAL}{HRT} \left(X_{NOB,a} f_d b_{NOB} (HRT) \right)$

Sludge Production

PX_BIO_PartA	89	kg VS/d	Active OHO Biomass				
PX_BIO_PartB	16.1	kg VS/d	Inactive OHO Biomass				
PX_BIO_PartC1	1.11	kg VS/d	Active AOB Biomass (NOT M&E Part C)				
PX_BIO_PartC2	0.37	kg VS/d	Active NOB Biomass (NOT M&E Part C)				
PX_BIO_Total	106	kg VS/d	Total VS produced per day				
Aeration Basin Influent TSS	422	mg/L					
Aeration Basin Influent VSS	312	mg/L	calculated based on MLSS data input (TSS/VSS)				
Aeration Basin Influent nbVSS	103	mg/L	assuming 33% of VSS is nbVSS				
PX_nbVSS	87	kg VS/d	Total non-biodegradable volatile suspended solids flowing through aeration basin (Part C in M&E)				
PX_VSS	193	kg VS/d	$P_{X,YSS} = \frac{QY_{0ABO}(S_0 - S)}{1 + b_{0HO}(SRT_T)} + \frac{f_{a}b_{0HO}(SRT_T)QY_{0HO}(S_0 - S)}{1 + b_{0HO}(SRT_T)} + \frac{QY_{AOB}(NH_{3,av})}{1 + b_{AOB}(SRT_T)} + \frac{QY_{NOB}(NH_{3,av})}{1 + b_{AOB}(SRT_T)} + \frac{QY_{NOB}(NH_{3,av})}{1 + b_{AOB}(SRT_T)}$ $(Part A) \qquad (Part C1) \qquad (Part C2)$				
PX_TSS	322	kg TSS/d	Total suspended solids flowing through aeration basin				
PX_VSS/PX_TSS	0.60	-	Should be between 65 and 80%				
NH3-N Assimilated into biomass	15.16	mg/L	Assumes 12% of new VS is Nitrogen - SUMO, ASM2d, M&E				
Sludge Production	322	kg TS/d					
Sludge Production (lb/d)	711	lb TS/d	converts kg to lb				
F:M	0.11		Food to microorganism ratio				

Oxygen Requirements

Oxygen required for BOD removal	435	kg O2/d	M&E Equation 7-61, assuming 1 kg O2/kg BOD removed
Oxygen offest for BOD assimilated into biomass	-151	kg O2/d	$R_o = Q(S_0 - S) - 1.42 P_{X,BIO}$
			$M_{O2,Nitr(fication} = 4.57 \frac{g}{g} \frac{O_2}{N} Q(NH_{3.ex})$
Oxygen required for nitrification	127	kg O2/d	M&E Equation 8.23, assuming 4.57 kg O2/kg nitrogen oxidized to nitrate
Total Oxygen Required	357	kg O2/d	$R_o = Q(S_0 - S) - 1.42P_{X,BIO} + 4.57 * Q * NO_x - 2.86 * NO_X,$
Oxygen offset for Denitrification	-54	kg O2/d	$bsCOD_{ox} = -2.86 * NO_{\chi},$ M&E Equation 7-124, 2.86 kg O2/kg NO3-N reduced

Air Flow

Standard pressure at sea level	10.33	m	
Cs,20	8.99	mg/L	DO saturation concentration at 20 deg C
C,temp	11.72	mg/L	DO saturation concentration at design temperature
C*∞ ₂₀	10.4	mg/L	$C^*_{oo20} = C^*_{o20} \left[1 + d_{\sigma} \left(\frac{D_f}{P_{\alpha}} \right) \right]$
Density of Air (p _a)	1.293	kg/m ³	$ \begin{aligned} \rho_{\alpha} = \frac{PM}{RT} & P = atmospheric pressure = 1.01325 * 10^{5} \frac{N}{m^{2}} \\ R = \text{universal gas constant, } 8314 \frac{N * m}{mole atr * K} \\ M = \text{molecular weight of air, } 28.97 \text{g/g mol} \\ T = \text{temperature, K} \end{aligned} $
kg O ₂ per m ³ air	0.2996	kg O ₂ /m ³ air	
Pb/Pa	0.989	-	$\frac{P_b}{P_a} = \exp\left[-\frac{gM(z_b - z_a)}{RT}\right]$
SOTE	1.022	kgQ2/d	$SOTR = \left(\frac{AOTR}{\alpha F}\right) \left[\frac{C_{w20}^*}{\beta\left(\frac{C_{g1}^*}{C_{g20}}\right)\left(\frac{F_b}{F_a}\right)C_{w20}^* - C}\right] \left[(1.024)^{20-T}\right]$
	1,022	N502/0	ka .
Required Standard Air Flow Rate	9,748	m3/d	$Airflow Rate, \frac{m^3}{day} = \frac{(SOTR, \frac{m^2}{day})}{(E)(\frac{kg \ O_2}{m^3 air})}$
Required standard air flow rate	6.77	sm3/min	
Required Standard Air Flow Rate	239	scfm	

Secondary Clarifiers

Total Secondary Clarifier Area	1,100	ft2	
Total Influent Flow (Q+Qr)	0.37	MGD	
Qr/A (2 clarifiers)	138	gpd/sf	
Qr/A (1 clarifier)	276	gpd/sf	
Solids Loading to Secondary Clarifiers	13,750	lb/d	
Solids Loading Rate (2 clarifiers)	12.50	lb/d/ft2	
Solids Loading Rate (1 clarifier)	25.00	lb/d/ft2	solving model based on SLR of 25 lb/day/sf with one (1) clarifier in service
Surface Overflow Rate (2 clarifiers)	202	gpd/sf	
Surface Overflow Rate (1 clarifier)	404	gpd/sf	

Nitrification

uAOB	0.1183	g/g day	$\mu_{AOB} = \mu_{max,AOB,T} \left(\frac{NH_{3 eff}}{NH_{3 eff} + K_{NH3}} \right) \left(\frac{DO}{DO + K_{O2}} \right) - b_{AOB,T}$
Minimum SRT for AOB growth	8.455	days	$SRT_{min,AOB} = \frac{1}{\mu_{AOB}} \label{eq:srtmin,AOB}$ based on eff NH4 target and DO concentration
AOB Safety Factor	1.836		$SF_{AOB} = \frac{SRT_{Aero}}{SRT_{min,AOB}}$
SNH4	1.621	mgNH4-N/L	$NH_{3,eff} = \frac{K_{NH3}[1 + b_{AOB}(SRT)]}{(SRT)(\mu_{AOB}) - 1}$
SNH4	1.621	mgNH4-N/L	Solver is needed here. This is because the ammonia concentration determines the growth rate of AOBs, but the ammonia concentration depends on this as well as the uptake by organisms for growth.
Square Error	9.81E-10		This is minimized using solver to force the calculated SNH4 to be the same as the SNH4 used for calculating the growth rate of AOBs.

APPENDIX E

MANHOLE INSPECTION REPORTS

Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 10:50 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID BK 01

Location Baker Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>				
7		8		AC		104		
	Manhole I	Details			Manhole Condition			
Where is the Roadway		What is the cover condition?	Servi	cable				
manhole located?				Other notes on the cover?				
What material is the manhole?	Concrete	2		What is the ring and frame condition?	Servi	ceable		
What cover size is the	26"			Other notes on the ring and frame?				
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable		
manhole? Other general				Other notes on the cone and riser?				
notes on the manhole				What is the barrel condition?	Servi	ceable		
details?	Hydrau	ilics		What is the ladder condition?	Servi	ceable		
Indication of	None			What is the bench condition?	Servi	ceable		
surcharge? Issues related to				Other notes on the bench?				
Level of infiltration?				What is the channel condition?	Servi	ceable, Obstructed		
				Other notes on the channel?	Some chan	e sewage blocking nel		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	Yes			
Standing Water	Yes			
Flow	Trickle			
Maintenance Needed	No			

Notes					
Structural Notes					
General Notes	Needs cleaning sewage in channel blocking flow at the outlet.				



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 12:35 PM

Attributes



Powered by Esri

Asset ID BO 01

Location **Browns Court**



<u>Outlet Clock</u> (Facing N	Outlet Clock PositionOutlet Pipe(Facing North)Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
6		8		AC		89
	Manhole	Details			Manho	le Condition
Where is the Grass		What is the cover condition?	Servi	cable		
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
	Hydra	ulics		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	Yes			
Flow	None			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 12:28 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID BO 02

Location Browns Court

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
12	8	AC	127
3	8	AC	128

<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet</u> Diamete	<u>Pipe</u> er (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
7		8		AC		128
	Manhole [Details			Manho	le Condition
Where is the manhole located?	Roadway			What is the cover condition? Other notes on the	Servi	cable
What material is the manhole?	Concrete			cover? What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?	Hydrau	lics		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 12:44 PM

Attributes



Asset ID BO 03

Location **Browns Court**



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
9		8		AC		124
Manhole Details				Manhole Condition		
Where is the Roadway manhole		What is the cover condition? Other notes	Servi	cable		
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"		Other notes on the ring and frame?	Rusted		
What barrel size is the	48"		What is the cone and riser condition?	Serviceable		
manhole? Other general	2? 1 2			Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
	Lludro	ulice		What is the ladder	Servi	ceable
Indication of	None	uncs		What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?	,			What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 12:52 PM

Attributes



Asset ID BO 04

Location **Browns Court**



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)			<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
9		8	_	AC		116	
Manhole Details				Manhole Condition			
Where is the manhole located?	Roadway e ? Il is Concrete e?		What is the cover condition? Other notes on the	Servi	cable		
What material is the manhole?			cover? What is the ring and frame condition?	e Serviceable			
What cover size is the	26"		Other notes on the ring and frame?				
What barrel size is the	48"			What is the cone and riser condition?	Serviceable		
manhole? Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
	Hydra	ulics		What is the ladder condition?	Servi	ceable	
Indication of	None			What is the bench condition?	Servi	ceable	
Issues related to				Other notes on the bench?			
Level of infiltration?	,			What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 1:01 PM

Attributes



Asset ID BO 05

Location **Browns Court**



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
10 8		AC 124		124		
	Manhole	Details			Manhc	le Condition
Where is the Roadway			What is the cover condition?	Servi	cable	
manhole located?				Other notes on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"		Other notes on the ring and frame?	Rusted		
What barrel size is the	48"		What is the cone and riser condition?	Serviceable		
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?				What is the ladder	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	Slight	
Cleaning Needed	No	
Standing Water	No	
Flow	None	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 2:09 PM

Attributes



Asset ID BO 06

Location **Browns Court**



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		Outlet Pipe MaterialOutlet Pipe DepthAC90		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
10 8				90		
Manhole Details			Manhole Condition			
Where is the Roadway		What is the cover condition?	Servi	cable		
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
	Hydra	ulics		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	Slight		
Cleaning Needed	No		
Standing Water	Yes		
Flow	Trickle		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 1:57 PM

Attributes



Asset ID BO 07

Location Browns Court



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)			<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
5 8		AC		70		
Manhole Details			Manhole Condition			
Where is the Grass manhole located?		What is the cover condition? Other notes on the	Servi	cable		
What material is the manhole?	Concret	e		cover? What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Manhole? Other general			_	Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?	Hydro	ulice		What is the ladder condition?	Servi	ceable
Indication of	None	uncs		What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	None		
Cleaning Needed	No		
Standing Water	No		
Flow	Trickle		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 2:02 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID BO 08

Location Browns Court



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
8 8		AC		99		
	Manhole	Details			Manhc	le Condition
Where is the Woods manhole located?			What is the cover condition? Other notes on the	Servi	cable	
What material is the manhole?	Concret	e		cover? What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?	Hydra	ulics		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	None		
Cleaning Needed	No		
Standing Water	No		
Flow	Trickle		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	


Richmond Sewer Collection System Manhole Inspection Surveyed: October 13, 2023 9:33 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID BO 09

Location **Browns Court**



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
7			8	AC		81	
Manhole Details			Manhole Condition				
Where is the Woods			What is the cover condition?	cable			
located?				on the cover?			
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable	
Manhole? Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
details?				What is the ladder	Servi	ceable	
Indication of	None	uncs		What is the bench condition?	Servi	ceable	
surcharge? Issues related to				Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Steady	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 12:26 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID BR 01

Location Bridge Street



<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
2		8		AC		110	
Manhole Details				Manhole Condition			
Where is the manhole	Where is the Roadway manhole		What is the cover condition? Other notes	Servi	cable		
located?				on the cover?			
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?	Ruste	ed	
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable	
manhole? Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
details?	Hydra	ulice		What is the ladder	Servi	ceable	
Indication of	None			What is the bench condition?	Servi	ceable	
Issues related to				Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Steady	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 12:41 PM

Attributes



Asset ID BR 02

Location Bridge Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
9	8	AC	114
3	8	PVC	118

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)			<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
11					
	Manhole D	etails		Manho	ole Condition
Where is the Roadway manhole		What is the cover condition? Other notes	cable		
What material is the manhole?	Concrete		cover? What is the ring and frame condition?	Servi	ceable
What cover size is the	26"		Other notes on the ring and frame?		
What barrel size is the	48"		What is the cone and riser condition?	Servi	ceable
manhole? Other general			Other notes on the cone and riser?		
notes on the manhole			What is the barrel condition?	Servi	ceable
uetails!	Hydrau	lics	What is the ladder condition?	Servi	ceable
Indication of	None		What is the bench condition?	Servi	ceable
Issues related to			Other notes on the bench?		
Level of infiltration?			What is the channel condition?	Servi	ceable
			Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	Moderate	
Cleaning Needed	Yes	
Standing Water	No	
Flow	Неаvy	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 12:49 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID BR 03

Location Bridge Street



Outlet Clock Position (Facing North)		<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
7		8		PVC		136	
	Manhole	Details		Manhole Condition			
Where is the manhole	/here is ne Roadway nanhole			What is the cover condition? Other notes	Servi	cable	
located?				on the cover?			
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?			
What barrel size is the	48"		-	What is the cone and riser condition?	Serviceable		
Other general	Drop inlet has concrete walls in a built up channel		_	Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
details?			What is the ladder condition?	Servi	ceable		
Indication of	None			What is the bench condition?	Servi	ceable	
Issues related to			-	Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	None	
Maintenance Needed	No	



	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 12:11 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID BR 04

Location Bridge Street



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
7		12	PVC		82	
Manhole Details			Manhole Condition			
Where is the manhole located?	Roadwa	У	What is the cover condition? Other notes on the	Servi	icable	
What material is the manhole?	Concret	e	cover? What is the ring and frame condition?	Servi	iceable	
What cover size is the	26"		Other notes on the ring and frame?			
What barrel size is the	48"		What is the cone and riser condition?	Servi	iceable	
manhole? Other general			Other notes on the cone and riser?			
notes on the manhole			What is the barrel condition?	Servi	iceable	
detalls?	Hydra	ulics	What is the ladder condition?	Servi	iceable	
Indication of	None		What is the bench condition?	Servi	iceable	
Issues related to			Other notes on the bench?	Debr	is on bench	
Level of infiltration?			What is the channel condition?	Servi	iceable	
			Other notes on the channel?			

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	Slight			
Cleaning Needed	No			
Standing Water	No			
Flow	Steady			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 12:41 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID BR 05

Location Bridge Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
7		12	AC		93	
	Manhole	Details		Manhole Condition		
Where is the Roadway		у	What is the cover condition?	Servi	cable	
located?			Other notes on the cover?			
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"		Other notes on the ring and frame?			
What barrel size is the	48"		What is the cone and riser condition?	Servi	ceable	
Other general			Other notes on the cone and riser?			
notes on the manhole			What is the barrel condition?	Servi	ceable	
	Hydra	ulics	What is the ladder condition?	Servi	ceable	
Indication of	None		What is the bench condition?	Servi	ceable	
Issues related to			Other notes on the bench?			
Level of infiltration?			What is the channel condition?	Servi	ceable	
			Other notes on the channel?			

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Steady			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 1:31 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID BR 06

Location Bridge Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
1	8	AC	105
9	8	AC	96
3	8	AC	102

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
6		12		AC		105
Manhole Details		Manhole Condition				
Where is the Roadway			What is the cover condition?	Servi	cable	
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"		_	What is the cone and riser condition?	Servi	ceable
Other general			_	Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
	Hydra	ulics]	What is the ladder condition?	Servi	ceable
Indication of	None		-	What is the bench condition?	Servi	ceable
Issues related to			-	Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	Moderate			
Cleaning Needed	Yes			
Standing Water	No			
Flow	Steady			
Maintenance Needed	No			

Notes				
Structural Notes				
General Notes	Signs of previous infiltration			



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 1:53 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID BR 07

Location Bridge Street



<u>Outlet Clock</u> (Facing N	Iet Clock PositionOutlet Pipe(Facing North)Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
8		12	AC		88	
	Manhole	Details	Manhole Condition			
Where is the Roadway		What is the cover condition?	Servi	cable		
located?			on the cover?			
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"		Other notes on the ring and frame?			
What barrel size is the	48"		What is the cone and riser condition?	Servi	ceable	
Other general			Other notes on the cone and riser?			
notes on the manhole			What is the barrel condition?	Servi	ceable	
detalls?			What is the ladder	Servi	ceable	
	Hydra	ulics	condition? What is the			
Indication of	None		bench condition?	Servi	ceable	
Issues related to			Other notes on the bench?			
Level of infiltration?			What is the channel condition?	Servi	ceable	
			Other notes on the channel?			

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	Slight		
Cleaning Needed	No		
Standing Water	No		
Flow	Steady		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 11:10 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID BR 08

Location North Main Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
2	8	AC	102
4	8	AC	102

<u>Outlet Clock</u> (Facing N	utlet Clock PositionOutlet Pipe(Facing North)Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
9		8	AC		104	
	Manhole	Details		Manhole Condition		
Where is the manhole	Where is the Roadway manhole		What is the cover condition? Other notes	Servi	cable	
located?			on the cover?			
What material is the manhole?	Brick		What is the ring and frame condition?	Servi	ceable	
What cover size is the manhole?	26"		Other notes on the ring and frame?			
What barrel size is the	Other		What is the cone and riser condition?			
Other general	e? Brick straight style		Other notes on the cone and riser?	N/A		
notes on the manhole			What is the barrel condition?	Servi	ceable	
details?			What is the	What is the N/A		
	Hydra	nulics	condition?			
Indication of	Minor		what is the bench condition?			
Issues related to	Silt		Other notes on the bench?	N/A		
Level of infiltration?			What is the channel condition?			
			Other notes on the channel?	N/A		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	None		
Cleaning Needed	No		
Standing Water	Yes		
Flow	Steady		
Maintenance Needed	No		



	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 12:14 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID BU 01

Location Burnett Court

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
10	6	AC	82
9	6	AC	74

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outl</u> Diame	<u>et Pipe</u> eter (in.)	<u>Outlet Pipe</u> <u>Material</u>		Outlet Pipe Invert Depth (in.)
3			8	AC		84
	Manhole	Details		Manhole Condition		
Where is the manhole	Roadwa	у		What is the cover condition? Other notes	Servi	cable
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?				What is the ladder	Servi	ceable
	Hydra	ulics		condition?		
Indication of	None			bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable, Obstructed
				Other notes on the channel?	Need reduc	to remove the detached cer.

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	None		
Cleaning Needed	No		
Standing Water	No		
Flow	None		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 12:05 PM

Attributes



Asset ID BU 02

Location **Burnette Court**

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
9	8	AC	145
3	8	AC	125

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
6			8	AC		147	
	Manhole	Details			Manhole Condition		
Where is the Roadway		What is the cover condition?	Servi	cable			
located?				on the cover?			
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable	
Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
detalls?	Hydro	ulice		What is the ladder condition?	Servi	ceable	
Indication of	None			What is the bench condition?	Servi	ceable	
Issues related to				Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	None	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 11:42 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID CH 01

Location Esplanade Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
12	12	AC	114
3	8	PVC	101

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
6		12	AC		113
	Manhole	Details		Manho	ole Condition
Where is the Grass, Row/Easement		What is the cover condition?			
manhole located?	Other notes on the cover?				
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	ceable
What cover size is the	26"		Other notes on the ring and frame?		
What barrel size is the	48"		What is the cone and riser condition?	Servi	ceable
Manhole? Other general			Other notes on the cone and riser?		
notes on the manhole			What is the barrel condition?	Servi	ceable
detalls?			What is the ladder	Servi	ceable
	Hydra	ulics	condition? What is the		
Indication of	Minor		bench condition?	Servi	ceable
Issues related to			Other notes on the bench?	Debr inlet	is buildup on bench from at 3 oclock
Level of infiltration?			What is the channel condition?	Servi	ceable
			Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	Moderate	
Cleaning Needed	Yes	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	



	Notes
Structural Notes	
General Notes	


Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 12:22 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID CH 02

Location Church Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
9	12	AC	94
6	8	AC	89
12	8	PVC	92

<u>Outlet Clock</u> (Facing N	Outlet Clock PositionOutlet Pipe(Facing North)Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
3		12		AC		93	
	Manhole	Details			Manho	le Condition	
Where is the Roadway		What is the cover condition?	Servi	cable			
located?				Other notes on the cover?			
What material is the manhole?	Concrete			What is the ring and frame condition?	Serviceable		
What cover size is the	26"		Other notes on the ring and frame?				
What barrel size is the	48"	48"		What is the cone and riser condition?	ceable		
Other general	nole? ral s on nole			Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	ceable		
details?				What is the ladder	Servi	ceable	
	Hydra	ulics		condition?			
Indication of	None	one		bench condition?	Serviceable		
Issues related to				Other notes on the bench?			
Level of infiltration?			What is the channel condition?	ceable			
				Other notes on the channel?			

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	Steady				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 30, 2023 8:34 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID CH 03

Location Church Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
11		8		PVC		100	
	Manhole	Details		Manhole Condition			
Where is the Woods		What is the cover condition?	Servi	cable			
located?			on the cover?				
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable	
Manhole? Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
	Hydra	ulics]	What is the ladder condition?	ceable		
Indication of	Minor			What is the bench condition?	Servi	ceable	
Issues related to	Debris,	Silt		Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	Slight				
Cleaning Needed	No				
Standing Water	No				
Flow	None				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 12:32 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID CH 04

Location Church Street



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
9		6		PVC		51
	Manhole	Details			Manhc	le Condition
Where is the Roadway, Grass		What is the cover condition?	Servi	cable		
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?	Hvdra	ulics		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?	Debr	is on bench
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?	Stand	ding water and debris

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	Slight				
Cleaning Needed	No				
Standing Water	Yes				
Flow	None				
Maintenance Needed	No				







Richmond Sewer Collection System Manhole Inspection Surveyed: June 30, 2023 8:51 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID CH 05

Location Church Street



<u>Outlet Clock</u> (Facing N	Outlet Clock PositionOutlet Pipe(Facing North)Diameter (in.)			<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
5			8		AC		57		
	Manhole	Deta	iils		Manhole Condition				
Where is the manhole located?	Grass, Does Grade Slope Away			What is the cover condition? Other notes on the	Servi	icable			
What material is the manhole?	Concret	e		-	cover? What is the ring and frame condition?	Servi	iceable		
What cover size is the	26"				Other notes on the ring and frame?	Some	e rust		
What barrel size is the	48"				What is the cone and riser condition?	Servi	iceable		
manhole? Other general					Other notes on the cone and riser?				
notes on the manhole					What is the barrel condition?	Servi	iceable		
details?	Hydra	ulics]	What is the ladder condition?	Servi	iceable		
Indication of	None				What is the bench condition?	Servi	iceable		
Issues related to					Other notes on the bench?	Heav	y debris		
Level of infiltration?					What is the channel condition?	Servi	iceable		
					Other notes on the channel?				

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	Heavy				
Cleaning Needed	Yes				
Standing Water	Yes				
Flow	None				
Maintenance Needed	No				







Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 12:04 PM



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID CO 01

Location Cochran Road



<u>Outlet Clock</u> (Facing N	Outlet Clock PositionOutlet Pipe(Facing North)Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
10		8		AC		95	
	Manhole	Details			Manho	ble Condition	
Where is the manhole located?	Roadway			What is the cover condition? Other notes on the	Servi	cable	
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?			
What barrel size is the	48"	3"		What is the cone and riser condition?	Servi	ceable	
manhole? Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
details?	Hydra	ulics		What is the ladder condition?	Servi	ceable	
Indication of	Minor			What is the bench condition?	Servi	ceable	
Issues related to	Debris,	Silt		Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?	Debr	is	

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	Slight			
Cleaning Needed	No			
Standing Water	No			
Flow	None			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 11:56 AM



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID CO 02

Location Cochran Road



<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
8		8		AC		89	
	Manhole	Details			Manho	le Condition	
Where is the Roadway		What is the cover condition?	Servi	cable, Damaged			
manhole located?				Other notes on the cover?	Rusting		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?	Rusti	ng	
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable	
manhole? Other general			_	Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
	Hydra	ulics		What is the ladder condition?	Servi	ceable	
Indication of	None		_	What is the bench condition?	Servi	ceable	
Issues related to				Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?	Slow	moving	

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	Slight			
Cleaning Needed	No			
Standing Water	Yes			
Flow	Trickle			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 11:48 AM



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID CO 03

Location Cochran Road



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
12		8		AC		103
	Manhole	Details			Manho	ole Condition
Where is the Roadway		What is the cover condition?	Servicable			
manhole located?				Other notes on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	iceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	iceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	iceable, Infiltration
				What is the ladder	Servi	iceable
Indication of	None	ulics		What is the bench condition?	Servi	iceable
surcharge? Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	iceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Trickle			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 9:55 AM





Asset ID DP 01

Location North Main Street

Inlet Clock Position (Facing North)	Inlet Pipe Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
11	8	PVC	106
11	8	PVC	89

<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
6		8		PVC		110
	Manhole	Details			Manho	ble Condition
Where is the Grass		What is the cover condition?	Servi	cable		
manhole located?				Other notes on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable, Missing Grout
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?	Hydra	ulics		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	Slight			
Cleaning Needed	No			
Standing Water	No			
Flow	None			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	





Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 9:49 AM

Attributes



Asset ID DP 02

Location Depot Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
6		8		AC		91	
	Manhole	Details			Manhole Condition		
Where is the Grass manhole		What is the cover condition? Other notes	Servi	cable			
located?				on the cover?			
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the manhole?	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable	
Manhole? Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
	Hydra	ulics		What is the ladder condition?	Servi	ceable	
Indication of	Minor			What is the bench condition?	Servi	ceable	
Issues related to	Grease	, Debris		Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	None		
Cleaning Needed	No		
Standing Water	No		
Flow	Trickle		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 9:44 AM

Attributes



Asset ID DP 03

Location Depot Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
5		8	8	AC		108
	Manhole	Details			Manhc	le Condition
Where is the Grass			What is the cover condition?		cable	
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole	2			What is the barrel condition?	Servi	ceable
details?		ulies		What is the ladder	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	None		
Cleaning Needed	No		
Standing Water	No		
Flow	Trickle		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 9:38 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID DP 04

Location Depot Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
11	8	AC	113
1	8	AC	112
2	8	AC	73

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
4		8		AC		115
	Manhole	Details			Manhc	le Condition
Where is the Grass				What is the cover condition?		cable
located?				Other notes on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?	Some	e rust
What barrel size is the	60"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?	Lludra	ulice		What is the ladder	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	Slight		
Cleaning Needed	No		
Standing Water	No		
Flow	Steady		
Maintenance Needed	No		







Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 9:17 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID DP 05

Location Depot Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
12	8	AC	127
5	8	AC	129
5	8	AC	87

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>)</u> 1.)	<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
8		12		AC		129
	Manhole	Details		Manhole Condition		
Where is the	Grass, (Other		What is the cover condition? Servicable		cable
located?				Other notes on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the manhole?	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general	manhole? Other general notes on the manhole			Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
detalls?				What is the ladder	Servi	ceable
	Hydra	iulics		condition? What is the		
Indication of surcharge?	None			bench condition?	Servi	ceable
Issues related to				Other notes on the bench?	Some	e debris
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	Slight		
Cleaning Needed	No		
Standing Water	No		
Flow	Steady		
Maintenance Needed	No		

Notes	
Structural Notes	
General Notes	


Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 12:30 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID DP 06

Location Depot Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
1	8	PVC	131
4	8	PVC	132

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
10		8		PVC		135
Manhole Details		Manhole Condition				
Where is the manhole	Where is the Roadway manhole		What is the cover condition? Other notes	cable		
located?				on the cover?		
What material is the manhole?	Concret	ete		What is the ring and frame condition?	ceable	
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general	al		Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	ceable	
				What is the ladder	Servi	ceable
Indication of	None	UIICS		What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?	<u>?</u>]?		What is the channel condition?	Servi	ceable	
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Trickle			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 12:36 PM

Attributes



Asset ID DP 07

Location Depot Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
11		8		PVC		97
Manhole Details		Manhole Condition				
Where is the Roadway		What is the cover condition?	Servi	cable		
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?	Some	e rust
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable, Misaligned
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	ceable	
details?	Hydra	ulice		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
surcharge? Issues related to				Other notes on the bench?		
Level of infiltration?	el of tration?			What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Trickle			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 12:43 PM

Attributes



Asset ID DP 08

Location Depot Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
2	8	AC	83
1	8	PVC	75

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>				
10		8		AC		93		
	Manhole	Details			Manhole Condition			
Where is the Roadway		What is the cover Servicable						
located?				Other notes on the cover?				
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable		
What cover size is the	26"			Other notes on the ring and frame?				
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable		
Other general				Other notes on the cone and riser?				
notes on the manhole			What is the barrel condition?	ceable				
	Hydro	ulice		What is the ladder condition?	Servi	ceable		
Indication of	None			What is the bench condition?	Servi	ceable		
Issues related to				Other notes on the bench?				
Level of infiltration?	on?		What is the channel condition?	Servi	ceable			
				Other notes on the channel?				

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Steady			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 12:04 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID ES 01

Location Esplanade Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
6		12		AC		110	
	Manhole	Details		Manhole Condition			
Where is the Roadway		What is the cover condition?	Servicable				
manhole located?				Other notes on the cover?			
What material is the manhole?	S Concrete		What is the ring and frame condition?	Loose, Displaced			
What cover size is the	26"			Other notes on the ring and frame?	Cover is stuck inside ring.		
What barrel size is the	48"	3"		What is the cone and riser condition?	Serviceable		
Other general	manhole? Other general notes on the manhole			Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
detalls?	Hydro	ulics]	What is the ladder condition?	Servi	ceable	
Indication of	None		_	What is the bench condition?	Servi	ceable	
Issues related to				Other notes on the bench?			
Level of infiltration?	evel of nfiltration?			What is the channel condition?	Servi	ceable	
				Other notes on the channel?	Smal chan	l amount of debris in nel	

General				
Structural Condition	Fair			
Operational Condition	Good			
Debris Amount	Slight			
Cleaning Needed	No			
Standing Water	No			
Flow	Trickle			
Maintenance Needed	Yes			



	Notes
Structural Notes	Loose frame and ring
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 11:53 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID ES 02

Location Esplanade Street



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
4		4		PVC		93
	Manhole	Details			Manho	ole Condition
Where is the Roadway		What is the cover condition?	Servicable			
manhole located?				Other notes on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	iceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	iceable, Misaligned
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	iceable
details?	Hydra	ulics		What is the ladder condition?	Servi	iceable
Indication of	None			What is the bench condition?	Servi	iceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	iceable
				Other notes on the channel?		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	Slight		
Cleaning Needed	No		
Standing Water	No		
Flow	None		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 11:30 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID ES 03

Location Esplanade Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
12	12	AC	159
10	8	AC	156
3	8	AC	158

<u>Outlet Clock</u> (Facing N	utlet Clock PositionOutlet Pipe(Facing North)Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
6		12	AC		160
	Manhole	Details		Manhc	le Condition
Where is the Roadway		What is the cover condition?	Servicable		
manhole located?			Other notes on the cover?		
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	ceable
What cover size is the manholo?	26"		Other notes on the ring and frame?		
What barrel size is the	48"		What is the cone and riser condition?	Servi	ceable
Other general			Other notes on the cone and riser?		
notes on the manhole			What is the barrel condition?	Servi	ceable
details?	Hydra	ulics	What is the ladder condition?	Servi	ceable
Indication of	None		What is the bench condition?	Servi	ceable
Issues related to			Other notes on the bench?		
Level of infiltration?			What is the channel condition?	Servi	ceable
			Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Trickle			
Maintenance Needed	No			







Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 8:20 AM



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID ES 04

Location Esplanade Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
5	8	AC	148
2	8	AC	133

<u>Outlet Clock</u> (Facing N	<u>C Position</u> <u>Outlet Pipe</u> <u>North)</u> <u>Diameter (in.)</u>		Pipe r (in.)	<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
10		8		AC		148
	Manhole	Details			Manhc	le Condition
Where is the Grass		What is the cover condition?	Servi	cable		
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the manhole?	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
	Lludra	ulice		What is the ladder	Servi	ceable
Indication of	None	uncs		What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?	Some Flow	e rags in the channel. isn't obstructed.

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	Slight		
Cleaning Needed	No		
Standing Water	No		
Flow	Steady		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 8:31 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID ES 05

Location Esplanade Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
9			8		AC		165
	Manhole	Deta	ils			Manho	ole Condition
Where is the	Grass			What is the cover condition?	Servicable		
located?					on the cover?		
What material is the manhole?	Concret	Concrete			What is the ring and frame condition?	Servi	ceable
What cover size is the manhole?	26"				Other notes on the ring and frame?		
What barrel size is the	48"				What is the cone and riser condition?	Servi	ceable
Other general	nole? r ral s on nole			Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
detalls?					What is the ladder	Serviceable	
Indication	Hydra	ulics			condition? What is the	Sanvisaabla	
of surcharge?	None	None			bench condition?		
Issues related to					Other notes on the bench?		
Level of infiltration?	Level of infiltration?			What is the channel condition?	Servi	ceable	
					Other notes on the channel?		

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	Slight				
Cleaning Needed	No				
Standing Water	No				
Flow	Steady				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 10:06 AM



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID HU 01

Location Huntington Road

Inlet Clock Position (Facing North)	Inlet Pipe Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
2	8	AC	71
5	2	PVC	50

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>e</u> n.)	<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
9		8		AC		72
	Manhole	Details			Manho	le Condition
Where is the manhole	ere is Roadway			What is the cover condition? Other notes	Servi	cable
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
uetans!	Hydra	ulics		What is the ladder condition?	Servi	ceable
Indication of	None	None		What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?	?			What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	Trickle				
Maintenance Needed	No				

Notes					
Structural Notes					
General Notes					







Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 10:12 AM



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID HU 02

Location Huntington Road



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)			<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>				
3			8		AC		120		
	Manhole	De	etails		Manhole Condition				
Where is the	Roadwa	adway			What is the cover condition?	Servicable			
located?					Other notes on the cover?				
What material is the manhole?	Concret	e			What is the ring and frame condition?	Serv	iceable		
What cover size is the	26"				Other notes on the ring and frame?				
What barrel size is the	48"			-	What is the cone and riser condition?	Serv	iceable		
manhole? Other general					Other notes on the cone and riser?				
notes on the manhole					What is the barrel condition?	Serv	iceable		
details?	Hydra	uli	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~]	What is the ladder condition?	Serv	iceable		
Indication of	None			-	What is the bench condition?	Serv	iceable		
Issues related to					Other notes on the bench?				
Level of infiltration?					What is the channel condition?	Serv	iceable		
					Other notes on the channel?	Bit o	f debris looks like rags		

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	Slight				
Cleaning Needed	No				
Standing Water	No				
Flow	Steady				
Maintenance Needed	No				



	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 1:44 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID JO 01

Location Jolina Court

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
6	8	PVC	114
8	8	AC	114

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
11 12		PVC		117			
	Manhole	Details		Manho	ole Condition		
Where is the	Roadwa	y	What is the cover condition?	Servi	Servicable		
manhole located?			Other notes on the cover?				
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	ceable		
What cover size is the manhole?	30"		Other notes on the ring and frame?				
What barrel size is the	48"		What is the cone and riser condition?	Servi	ceable		
Other general	manhole? Other general notes on the manhole		Other notes on the cone and riser?				
notes on the manhole			What is the barrel condition?	Servi	ceable		
detalls?	Hydro	ulics	What is the ladder condition?	Servi	ceable		
Indication of	None		What is the bench condition?	Servi	ceable		
Issues related to			Other notes on the bench?				
Level of infiltration?	of ation?		What is the channel condition?	Servi	ceable, Obstructed		
			Other notes on the channel?				

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	Slight			
Cleaning Needed	No			
Standing Water	No			
Flow	Trickle			
Maintenance Needed	No			

Notes				
Structural Notes				
General Notes				



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 2:31 PM



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID JR 01

Location Jericho Road



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	D	<u>Outlet Pipe</u> iameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
6			8		AC		101		
Manhole Details				Manhole Condition					
Where is the Roadway					What is the cover condition?	Servicable			
manhole located?					Other notes on the cover?				
What material is the manhole?	Concret	e			What is the ring and frame condition?	at is the g and Serviceable me ndition?			
What cover size is the manhole?	26"				Other notes on the ring and frame?				
What barrel size is the	48"			-	What is the cone and riser condition?	Servi	ceable		
Other general					Other notes on the cone and riser?				
notes on the manhole				What is the barrel condition?	Serviceable				
details?	Lludro]	What is the ladder	Servi	ceable		
Indication of	None			_	What is the bench condition?	Servi	ceable		
Issues related to					Other notes on the bench?				
Level of infiltration?					What is the channel condition?	Servi	ceable		
					Other notes on the channel?				

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	None		
Cleaning Needed	No		
Standing Water	No		
Flow	None		
Maintenance Needed	No		

Notes				
Structural Notes				
General Notes				


Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 2:26 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID JR 02

Location Jericho Road



Outlet Clock Position(Facing North)D		<u>Outle</u> Diame	<u>et Pipe</u> eter (in.)	<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
9			8	AC		107	
Manhole Details				Manhole Condition			
Where is the Roadway, Grass				What is the cover condition?	Servi	cable	
manhole located?	manhole located?			Other notes on the cover?			
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the manholo?	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Servi	Serviceable	
Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
detalls?				What is the ladder	Servi	ceable	
	Hydra	ulics		condition?			
Indication of surcharge?	None			bench condition?	Servi	ceable	
Issues related to				Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	None			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 2:19 PM

Attributes



Asset ID JR 03

Location Jericho Road

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
	Manhole D	etails		Manhc	le Condition		
Where is the manhole located?			What is the cover condition? Other notes on the				
What material is the manhole?			What is the ring and frame condition?				
What cover size is the			Other notes on the ring and frame?				
What barrel size is the			What is the cone and riser condition?				
Other general	Couldn't c	open. Cover	Other notes on the cone and riser?				
the manhole details?	stuck.		What is the barrel condition?				
Hydraulics			ladder condition?				
Indication of surcharge?			bench condition?				
Issues related to			Other notes on the bench?				
Level of infiltration?			What is the channel condition?				
			Other notes on the channel?				

General				
Structural Condition				
Operational Condition				
Debris Amount				
Cleaning Needed				
Standing Water				
Flow				
Maintenance Needed				

	Notes
Structural Notes	
General Notes	

Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 2:38 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID JR 04

Location Jericho Road



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>)utlet Pipe</u> ameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
9			8		AC		109	
Manhole Details					Manhole Condition			
Where is the Roadway		у			What is the cover condition?		icable	
located?					Other notes on the cover?			
What material is the manhole?	Concret	Concrete			What is the ring and frame condition?	Servi	iceable	
What cover size is the manhole?	26"				Other notes on the ring and frame?			
What barrel size is the	48"			-	What is the cone and riser condition?	Servi	iceable	
Other general	manhole? Other general notes on the manhole details?				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?		Servi	iceable	
details?]	What is the ladder	Servi	iceable	
Indication of	None			_	What is the bench condition?	Servi	iceable	
Issues related to surcharge?				-	Other notes on the bench?			
Level of infiltration?					What is the channel condition?	Servi	iceable	
					Other notes on the channel?			

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	None			
Maintenance Needed	No			

Notes					
Structural Notes					
General Notes					



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 2:44 PM

Attributes



Asset ID JR 05

Location Jericho Road



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)			<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
6			8		AC		92
Manhole Details				Manhole Condition			
Where is the Roadway					What is the cover condition?	Servi	cable
manhole located?					Other notes on the cover?		
What material is the manhole?	Concret	oncrete			What is the ring and frame condition?	Servi	ceable
What cover size is the manhole?	26"				Other notes on the ring and frame?		
What barrel size is the	48"				What is the cone and riser condition?	ceable	
Other general					Other notes on the cone and riser?		
notes on the manhole	notes on the manhole details?			What is the barrel condition?	Servi	ceable	
detalls?				What is the ladder condition?	Servi	ceable	
Indication of	None				What is the bench condition?	Servi	ceable
Issues related to					Other notes on the bench?		
Level of infiltration?					What is the channel condition?	Servi	ceable
					Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	None			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 2:51 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID JR 06

Location Jericho Road



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	D	<u>Outlet Pipe</u> iameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
6			8		AC		106
	Manhole	Deta	ils			Manho	ole Condition
Where is the Roadway				What is the cover condition?	Servi	cable	
manhole located?		Other notes on the cover?					
What material is the manhole?	Concret	e			What is the ring and frame condition?	Servi	ceable
What cover size is the manhole?	26"				Other notes on the ring and frame?		
What barrel size is the	48"			-	What is the cone and riser condition?	Servi	ceable
Other general	Other general			Other notes on the cone and riser?			
notes on the manhole	es on nhole				What is the barrel condition?	ceable	
details?	Hydro	ulice]	What is the ladder	Servi	ceable
Indication of	None			-	What is the bench condition?	Servi	ceable
Issues related to					Other notes on the bench?		
Level of infiltration?					What is the channel condition?	Servi	ceable
					Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	None			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 2:57 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID JR 07

Location Jericho Road

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
12	8	AC	88
12	8	AC	115

<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	[<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
6			8		AC		116
	Manhole	Deta	ails			Manho	ole Condition
Where is the Roadway				What is the cover condition?	Servi	cable	
located?					Other notes on the cover?		
What material is the manhole?	Concret	e			What is the ring and frame condition?	Servi	ceable
What cover size is the manhole?	26"				Other notes on the ring and frame?		
What barrel size is the	48"				What is the cone and riser condition?	Servi	ceable
Other general	nanhole? Other general notes on the manhole				Other notes on the cone and riser?		
notes on the manhole					What is the barrel condition?	ceable	
details?]	What is the ladder	Servi	ceable
Indication of	None				What is the bench condition?	Servi	ceable
Issues related to					Other notes on the bench?		
Level of infiltration?					What is the channel condition?	Servi	ceable
					Other notes on the channel?		

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	None				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 3:04 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID JR 08

Location Jericho Road

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
12	8	PVC	77
12	8	PVC	136
9	6	PVC	126

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)		<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
6			8	AC		140
	Manhole	Def	ails		Manho	ole Condition
Where is the Roadway			What is the cover condition?	Servi	cable	
manhole located?				Other notes on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the manhole?	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				 Other notes on the cone and riser?		
notes on the manhole	on			What is the barrel condition?	Servi	ceable
details?				What is the ladder	Servi	ceable
Indication of	None		5	 What is the bench condition?	Servi	ceable
Issues related to				 Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	None				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 3:10 PM

Attributes



Asset ID JR 09

Location Jericho Road



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
6		8		PVC		89	
Manhole Details				Manhole Condition			
Where is the Grass manhole located?		What is the cover condition?	cable				
		Other notes on the cover?					
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the manhole?	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable	
Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
details?	Hydra	ulics		What is the ladder condition?	Servi	ceable	
Indication of	None			What is the bench condition?	Servi	ceable	
Issues related to				Other notes on the bench?	Some	e debris on bench	
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	Slight				
Cleaning Needed	No				
Standing Water	No				
Flow	Trickle				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 3:15 PM



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID JR 10

Location Jericho Road



<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Ou</u> Diar	<u>tlet Pipe</u> neter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
6			8		PVC		81	
Manhole Details				Manhole Condition				
Where is the Grass, Sidewalk					What is the cover condition?	Servi	cable	
manhole located?		Other notes on the cover?						
What material is the manhole?	Concret	e			What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"				Other notes on the ring and frame?			
What barrel size is the	48"				What is the cone and riser condition?	Servi	ceable	
Other general	nanhole? other eneral otes on ne nanhole			Other notes on the cone and riser?				
notes on the manhole					What is the barrel condition?	Servi	ceable	
details?	Hydra	ulics			What is the ladder condition?	Servi	ceable	
Indication of	None				What is the bench condition?	Servi	ceable	
Issues related to					Other notes on the bench?			
Level of infiltration?					What is the channel condition?	Servi	ceable	
					Other notes on the channel?			

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	Trickle				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 7, 2023 8:57 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID JR 11

Location Jericho Road



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
6		8		PVC		99		
Manhole Details				Manhole Condition				
Where is the Roadway				What is the cover condition?	Servi	icable		
manhole located?				Other notes on the cover?				
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	iceable		
What cover size is the	26"			Other notes on the ring and frame?	Some fram	e infiltration around the e and cover.		
What barrel size is the	48"			What is the cone and riser condition?	Servi	iceable		
Other general				Other notes on the cone and riser?				
notes on the manhole				What is the barrel condition?	Servi	iceable		
detalls?	Hydra	ulics		What is the ladder condition?	Servi	iceable		
Indication of	None			What is the bench condition?	Servi	iceable		
Issues related to				Other notes on the bench?				
Level of infiltration?				What is the channel condition?	Servi	iceable		
				Other notes on the channel?				

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	Slight				
Cleaning Needed	No				
Standing Water	No				
Flow	Trickle				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 1:26 PM

Attributes



Asset ID LM 01

Location Lemroy Court



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)		<u>Outlet Pipe</u> <u>Diameter (in.)</u>		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
7			8		AC		124		
Manhole Details					Manhole Condition What is the cover condition?				
Where is the Other									
located?					on the cover?				
What material is the manhole?	Concret	e			What is the ring and frame condition?	Serv	iceable		
What cover size is the	26"				Other notes on the ring and frame?				
What barrel size is the	48"			-	What is the cone and riser condition?	Serv	iceable		
manhole? Other general					Other notes on the cone and riser?				
notes on the manhole					What is the barrel condition?	Serv	iceable		
details?	Hydra	ulic	c]	What is the ladder condition?	Serv	iceable		
Indication of	None		5		What is the bench condition?	Serv	iceable		
Issues related to					Other notes on the bench?				
Level of infiltration?					What is the channel condition?	Serv	iceable		
					Other notes on the channel?				

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	Steady				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 2:46 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID LM 02

Location Lemroy Court



<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outle</u> Diame	<u>et Pipe</u> eter (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
9			8	AC		104
Manhole Details			Manhole Condition			
Where is the	Grass			What is the cover condition?	Servicable	
located?				Other notes on the cover?		
What material is the manhole?	Concrete			What is the ring and frame condition?	Serviceable	
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Serviceable, Misaligned	
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?	Lludra	ulice		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Trickle			
Maintenance Needed	No			

Notes				
Structural Notes				
General Notes				


Richmond Sewer Collection System Manhole Inspection Surveyed: June 30, 2023 9:09 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID LM 03

Location Lemroy Court



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pir</u> <u>Diameter (</u>	<u>be</u> (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
10		8		AC		87
Manhole Details				Manhc	le Condition	
Where is the Roadway			What is the cover condition?	cable		
located?				Other notes on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?	Some	e rust
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
				What is the ladder	Servi	ceable
Indication of	None	uncs		What is the bench condition?	Servi	ceable
surcharge? Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 30, 2023 9:21 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID LM 04

Location Lemroy Court

Inlet Clock Position (Facing North)	Inlet Pipe Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
5	8	AC	155
12	8	PVC	152
12	8	PVC	36

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pip</u> <u>Diameter (</u> i	<u>e</u> in.)	<u>Outlet Pipe</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
10		8		AC		155
Manhole Details				Manho	le Condition	
Where is the Grass		What is the cover condition?	Servi	cable		
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
	Lludra	ulice		What is the ladder	Servi	ceable
Indication of	None	uncs		What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	None	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 30, 2023 9:31 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID LM 05

Location Lemroy Court



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
7		8		AC		158
	Manhole D	etails			Manho	ole Condition
Where is the manhole located?	Roadway			What is the cover condition? Other notes	Serv	icable
			-	cover?		
What material is the manhole?	Concrete			What is the ring and frame condition?	Serv	iceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"		_	What is the cone and riser condition?	Serv	iceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Serv	iceable
detalls?	Hydraul	ics]	What is the ladder condition?	Serv	iceable
Indication of	None			What is the bench condition?	Serv	iceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Serv	iceable
				Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 7, 2023 10:37 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID LM 06

Location Lemroy Court



<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (ir	<u>e</u> n.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
9		8		AC		141
Manhole Details			Manhole Condition			
Where is the Grass		What is the cover condition?ServicableOther notes				
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the manhole?	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
				What is the ladder	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 10:20 AM

Attributes



Chittenden County RPC, VCGI, Esri Ganada, Esri, HERE, Garmi.... Powered by Esri

Asset ID ML 01

Location Millet Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
7		8	AC		93		
	Manhole D	etails	Manhole Condition				
Where is the manhole located?	Roadway		What is the cover condition? Other notes on the	Servi	icable		
What material is the manhole?	Concrete		cover? What is the ring and frame condition?	Servi	iceable		
What cover size is the	26"		Other notes on the ring and frame?				
What barrel size is the	48"		What is the cone and riser condition?	Servi	iceable		
manhole? Other general			Other notes on the cone and riser?				
notes on the manhole			What is the barrel condition?	Servi	iceable		
details?	Hydraul	ics	What is the ladder condition?	Servi	iceable		
Indication of	None		What is the bench condition?	Servi	iceable		
Issues related to			Other notes on the bench?				
Level of infiltration?			What is the channel condition?	Servi	iceable, Obstructed		
			Other notes on the channel?	Sewa	age in channel		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	Yes			
Standing Water	Yes			
Flow	None			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 10:30 AM

Attributes



Asset ID ML 02

Location Millet Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
2	8	AC	75
6	8	PVC	72

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
8		8	3	AC		79	
Manhole Details		Manhole Condition					
Where is the manhole	Roadway		What is the cover condition? Other notes	Servi	cable		
What material is the manhole?	Concret	e		What is the ring and frame condition?	ceable		
What cover size is the	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Serviceable		
manhole? Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
uetans!	Hydra	ulics		What is the ladder condition?	Servi	ceable	
Indication of	None			What is the bench condition?	Servi	ceable	
Issues related to				Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Trickle			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 10:35 AM

Attributes



Asset ID ML 03

Location Millet Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
6			8	AC		87	
Manhole Details			Manhole Condition				
Where is the Roadway		What is the cover condition?	Servi	cable			
located?				Other notes on the cover?			
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Serviceable		
Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
details?	Hydro	ulice		What is the ladder	Servi	ceable	
Indication of	None	uncs		What is the bench condition?	Servi	ceable	
Issues related to				Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Trickle			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: October 13, 2023 9:48 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID NM 01

Location North Main Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
9			8	AC		118	
Manhole Details			Manhole Condition				
Where is the Woods		What is the cover condition?	Servi	cable			
located?				on the cover?			
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Serviceable		
Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
details?	Lludro	ulico		What is the ladder	Servi	ceable	
Indication of	None	uiics		What is the bench condition?	Servi	ceable	
Issues related to				Other notes on the bench?			
Level of infiltration?				What is the channel condition?	ceable		
				Other notes on the channel?			

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Trickle			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 10:10 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID NM 02

Location North Main Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>				
4			8	AC		95		
	Manhole	Details			Manhole Condition			
Where is the Roadway		What is the cover condition?	Servi	Servicable				
located?				Other notes on the cover?				
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable		
What cover size is the	26"			Other notes on the ring and frame?	Ruste	ed but functional		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable		
Other general				Other notes on the cone and riser?				
notes on the manhole				What is the barrel condition?	Servi	ceable		
details?	Hydro	ulice		What is the ladder	Servi	ceable		
Indication of	None			What is the bench condition?	Servi	ceable		
Issues related to				Other notes on the bench?				
Level of infiltration?				What is the channel condition?	Servi	ceable		
				Other notes on the channel?				

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	Slight			
Cleaning Needed	No			
Standing Water	No			
Flow	Trickle			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 10:03 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID NM 03

Location North Main Street



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
3		8	PVC		52	
	Manhole	Details	Manhole Condition			
Where is the	Grass		What is the cover condition? Servicable			
located?			on the cover?			
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"		Other notes on the ring and frame?			
What barrel size is the	48"		What is the cone and riser condition?	Servi	ceable	
Other general			Other notes on the cone and riser?			
notes on the manhole			What is the barrel condition?	Servi	ceable	
detalls?	Hydro	ulice	What is the ladder condition?	Servi	ceable	
Indication of	None		What is the bench condition?	Servi	ceable	
Issues related to			 Other notes on the bench?			
Level of infiltration?			What is the channel condition?	Servi	ceable	
			Other notes on the channel?			

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	Slight			
Cleaning Needed	No			
Standing Water	No			
Flow	None			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 10:20 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi Powered by Esri

Asset ID NM 04

Location North Main Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
11	8	AC	118
1	8	AC	112

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)			<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
4		8		AC		119	
	Manhole	Details		Manhole Condition			
Where is the Roadway		What is the cover condition? Other notes	Servicable				
located?				on the cover?			
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	iceable	
What cover size is the	26"			Other notes on the ring and frame?	Ruste	ed	
manhole? What barrel size is the	48"		-	What is the cone and riser condition?	Servi	iceable	
manhole? Other general			-	Other notes on the cone and riser?	Missi	ng bricks	
notes on the manhole				What is the barrel condition?	Servi	iceable	
details?	Hydra	ulics		What is the ladder condition?	Servi	iceable	
Indication of	Minor		-	What is the bench condition?	Servi	iceable	
Issues related to	Debris,	Infiltration		Other notes on the bench?			
Level of infiltration?	Some			What is the channel condition?		iceable	
				Other notes on the channel?			

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	Moderate			
Cleaning Needed	No			
Standing Water	No			
Flow	Steady			
Maintenance Needed	Yes			

Notes				
Structural Notes	Missing bricks in riser and bricks/debris in the channel			
General Notes				



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 10:29 AM

Attributes



Asset ID NM 05

Location North Main Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
10	8	AC	95
2	8	AC	99

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
5		8	AC		99
Manhole Details		Manhole Condition			
Where is the manhole located?	Roadwa	У	What is the cover condition? Other notes	Servi	icable
What material is the manhole?	Concret	e	cover? What is the ring and frame condition?	Servi	iceable
What cover size is the	26"		Other notes on the ring and frame?		
What barrel size is the	48"		What is the cone and riser condition?	Servi	iceable, Misaligned
manhole? Other general			Other notes on the cone and riser?	Missi	ng some bricks
notes on the manhole			What is the barrel condition?	Servi	iceable
	Hvdra	ulics	What is the ladder condition?	Servi	iceable
Indication of	None		What is the bench condition?	Servi	iceable
Issues related to			Other notes on the bench?		
Level of infiltration?			What is the channel condition?	Servi	iceable, Obstructed
			Other notes on the channel?		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	Moderate		
Cleaning Needed	Yes		
Standing Water	No		
Flow	Steady		
Maintenance Needed	Yes		

Notes				
Structural Notes	Need to replace bricks in riser			
General Notes	Need to clean and remove debris in channel			


Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 10:36 AM

Attributes



Asset ID NM 06

Location North Main Street



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet</u> Diamete	<u>Pipe</u> er (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
6		8		AC		95	
Manhole Details				Manho	le Condition		
Where is the Sidewalk			What is the cover condition?	Servicable			
manhole located?				Other notes on the cover?			
What material is the manhole?	Concrete	Concrete		What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable	
Other general				Other notes on the cone and riser?	Missi	ng bricks in riser	
notes on the manhole				What is the barrel condition?	Servi	ceable	
	Hydra	ulics		What is the ladder condition?	Servi	ceable	
Indication of	None			What is the bench condition?	Servi	ceable, Cracked/Broken	
Issues related to				Other notes on the bench?	Some does	e missing pieces but n't affect operation	
Level of infiltration?				What is the channel condition?	Servi	ceable, Obstructed	
				Other notes on the channel?	Brick chan	s from the riser in the nel	

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	Slight	
Cleaning Needed	No	
Standing Water	No	
Flow	Steady	
Maintenance Needed	No	

	Notes
Structural Notes	Some missing bricks in riser but okay for now
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 10:45 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmin Powered by Esri

Asset ID NM 07

Location North Main Street



<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pi</u> Diameter	<u>pe</u> (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
11		8		AC		78
Manhole Details			Manhole Condition			
Where is the manhole located?	Roadwa	vay		What is the cover condition? Other notes on the cover?	Servi	cable
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
	Hydra	ulics		What is the ladder condition?	Servi	ceable
Indication of	Minor			What is the bench condition?	Servi	ceable
Issues related to	Debris,	Silt		Other notes on the bench?		
Level of infiltration?				What is the channel condition?		
				Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	Slight	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 7, 2023 10:01 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID NM 08

Location North Main Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
12	8	PVC	62
2	8	PVC	62

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
7		8		PVC		62
Manhole Details					ble Condition	
Where is the Sump/Depression, Other				What is the cover condition?	Servi	cable
located?				on the cover?		
What material is the manhole?	Concrete			What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?	Hydraul	lics		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?	A lot	of debris on bench.
Level of infiltration?				What is the channel condition?	Servi	ceable, Obstructed
				Other notes on the channel?	Full c clear	of sewage. Needs ning.

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	Yes	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 10:52 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID NM 09

Location North Main Street



<u>Outlet Clock</u> (Facing N	Position orth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> Depth (in.)
6		8		PVC		47
Manhole Details			Manhole Condition			
Where is the Grass manhole			What is the cover condition? Other notes	Servi	cable	
What material is the manhole?	Concret	e	-	What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general			-	Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?	Hydra	ulics]	What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?			-	What is the channel condition?	Servi	ceable, Obstructed
				Other notes on the channel?	Chan	nel full of sewage

General		
Structural Condition	Good	
Operational Condition	Poor	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	Yes	
Flow	None	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 12:59 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID NM 10

Location North Main Street



<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
5		8		PVC		70
	Manhole	Details			Manho	le Condition
Where is the manhole located?	Grass			What is the cover condition? Other notes	Servi	cable
What material is the manhole?	Concret	e		what is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
	Hydra	ulics]	What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Trickle			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 12:52 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID NM 11

Location North Main Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
10	8	PVC	76
12	6	PVC	75
2	6	PVC	74

<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
7		8		PVC		77
	Manhole	Details			Manho	ole Condition
Where is the Grass		What is the cover condition?	Servicable			
manhole located?				Other notes on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	iceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"		_	What is the cone and riser condition?	Servi	iceable, Cracked/Broken
manhole? Other general			-	Other notes on the cone and riser?	Some	e crack in the grout
notes on the manhole				What is the barrel condition?	Servi	iceable
details?	Hydra	ulics]	What is the ladder condition?	Servi	iceable
Indication of	None			What is the bench condition?	Servi	iceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	iceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	None			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 11:00 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID NM 12

Location North Main Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
1	8	PVC	72
4	8	PVC	70

<u>Outlet Clock</u> (Facing N	Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
11		8		PVC		72
	Manhole	Details			Manhc	le Condition
Where is the Grass		What is the cover condition?	Servi	cable		
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?				What is the ladder	Servi	ceable
Indication of	Hydra None	ulics		What is the bench	Servi	ceable
surcharge? Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	None		
Cleaning Needed	No		
Standing Water	No		
Flow	Trickle		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 7, 2023 10:44 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID PL 01

Location Pleasant Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
10 8		AC		139			
	Manhole	Details			Manho	le Condition	
Where is the	Roadway, Grass			What is the cover condition?	Servicable		
manhole located?			Other notes on the cover?				
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Serviceable		
Other general	nole? r ral s on nole			Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
details?				What is the ladder	Servi	ceable	
Indication	Hydra None	iulics		Condition? What is the bench	Servi	ceable	
surcharge? Issues related to				Other notes on the bench?			
surcharge? Level of infiltration?	urcharge? .evel of nfiltration?		What is the channel condition?	Serviceable			
				Other notes on the channel?			

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	Trickle				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 1:22 PM





Asset ID

PL 02

Location Pleasant Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
3	8	AC	135
12	8	AC	133
12	8	AC	73

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>				
9 8			AC		136			
	Manhole	Deta	ils			Manho	ole Condition	
Where is the Roadway			What is the cover condition?	Servicable				
located?					on the cover?			
What material is the manhole?	Concret	e			What is the ring and frame condition?	Servi	ceable	
What cover size is the manhole?	26"				Other notes on the ring and frame?	Rusting and will need replacing soon		
What barrel size is the	48"				What is the cone and riser condition?	Serviceable		
Other general	ral on ole			Other notes on the cone and riser?				
notes on the manhole				What is the barrel condition?	Servi	ceable		
			What is the ladder condition?	Servi	ceable			
Indication of	ion None			What is the bench condition?	Servi	ceable		
Issues related to					Other notes on the bench?			
Level of infiltration?			What is the channel condition?	Servi	ceable			
					Other notes on the channel?			

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	Trickle				
Maintenance Needed	Yes				

	Notes
Structural Notes	Frame needs replacing
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 1:41 PM

Attributes



Asset ID PL 03

Location Pleasant Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
11	8	PVC	70
3	8	PVC	69

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
8		8		PVC		71	
Manhole Details					Manhc	le Condition	
Where is the Roadway			What is the cover condition?	Servi	Servicable		
manhole located?				Other notes on the cover?			
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the	30"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Serviceable		
Other general	nhole? er ieral es on nhole			Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
detalls?	Hydro	ulice		What is the ladder condition?	Servi	ceable	
Indication of	None			What is the bench condition?	Servi	ceable	
Issues related to surcharge?				Other notes on the bench?			
Level of infiltration?	iltration?		What is the channel condition?	Servi	ceable		
				Other notes on the channel?			

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	Trickle				
Maintenance Needed	No				



	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 1:11 PM

Attributes



Asset ID PL 04

Location Pleasant Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
10 8		AC		147			
	Manhole	Details			Manho	ole Condition	
Where is the Roadway			What is the cover condition?	Servi	cable		
located?				on the cover?			
What material is the manhole?	Concreto	2		What is the ring and frame condition?	Servi	ceable	
What cover size is the manhole?	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Serviceable		
Other general	ole?			Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
			What is the ladder condition?	e Serviceable			
Indication of	None			What is the bench condition?	Servi	ceable	
Issues related to				Other notes on the bench?			
Level of infiltration?	Level of infiltration?		What is the channel condition?	Serviceable			
				Other notes on the channel?			

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

Notes		
Structural Notes		
General Notes		



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 12:13 PM



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID RC 01

Location Round Church Road



<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet</u> Diamete	<u>Pipe</u> er (in.)	<u>Outlet Pipe</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
1		8		AC		93	
Manhole Details			Manhole Condition				
Where is the manhole	ere is Roadway		What is the cover condition?ServicableOther notes on the				
What material is the manhole?	Concret	e		What is the ring and frame condition?	Serviceable		
What cover size is the	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable	
manhole? Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
uetans!	Hydra	ulics		What is the ladder condition?	Servi	ceable	
Indication of	None			What is the bench condition?	Servi	ceable	
Issues related to				Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

Notes		
Structural Notes		
General Notes		


Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 12:53 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID RR 01

Location Railroad Street

Inlet Clock Position (Facing North)	Inlet Pipe Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
2	12	AC	98
11	12	PVC	86
10	8	PVC	84

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)			<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
6		12		AC		98
Manhole Details			Manho	le Condition		
Where is the Roadway manhole		What is the cover condition? Other notes	Servi	cable		
			-	on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the manhole?	26"			Other notes on the ring and frame?		
What barrel size is the	48"		-	What is the cone and riser condition?	Servi	ceable
Other general			-	Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?]	What is the ladder	Servi	ceable
	Hydra	ulics		condition?		
Indication of surcharge?	None			bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Steady			
Maintenance Needed	No			



	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 1:05 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID RR 02

Location Railroad Street



<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
6		12		PVC		94
Manhole Details		Manhole Condition				
Where is the manhole located?	Grass			What is the cover condition? Other notes on the	Servi	cable
What material is the manhole?	Concret	e		cover? What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?	Hvdra	ulics		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
surcharge? Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Obst	ructed
				Other notes on the channel?	Debr to be	is in the channel. Needs cleaned.

General				
Structural Condition	Good			
Operational Condition	Fair			
Debris Amount	Moderate			
Cleaning Needed	Yes			
Standing Water	Yes			
Flow	Trickle			
Maintenance Needed	No			

Notes					
Structural Notes					
General Notes	Needs cleaning. Water backing up.				



Richmond Sewer Collection System Manhole Inspection Surveyed: July 28, 2023 9:17 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID RR 03

Location Railroad Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
3	12	AC	106
5	8	AC	107

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
9			12	AC		126
	Manhole	Details	;		Manho	le Condition
Where is the Roadway, Other		What is the cover condition?	Servi	cable		
located?				Other notes on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?				What is the ladder	Servi	ceable
	Hydra	ulics		condition? What is the		
Indication of surcharge?	None			bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Steady			
Maintenance Needed	No			

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 1:16 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID RR 04

Location **Railroad Street**

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)

<u>Outlet Clock</u> (Facing N	<u>Position</u> <u>lorth)</u> <u>D</u>	<u>Outlet Pipe</u> Viameter (in.)	<u>Outlet Pipe</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
	Manhole Deta	iils		Manho	le Condition
Where is the manhole located?			What is the cover condition? Other notes on the cover?		
What material is the manhole?			What is the ring and frame condition?		
What cover size is the			Other notes on the ring and frame?		
What barrel size is the			What is the cone and riser condition?		
Manhole? Other general			Other notes on the cone and riser?		
the manhole details?		en	What is the barrel condition?		
	Hydraulics		ladder condition?		
Indication of surcharge?			what is the bench condition?		
Issues related to			Other notes on the bench?		
Level of infiltration?			What is the channel condition?		
			Other notes on the channel?		

General				
Structural Condition				
Operational Condition				
Debris Amount				
Cleaning Needed				
Standing Water				
Flow				
Maintenance Needed				

Notes				
Structural Notes				
General Notes	Unable to open			



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 1:24 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID RR 05

Location Railroad Street



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)			<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
12		8		PVC		97
Manhole Details		Manhole Condition				
Where is the Roadwa		У		What is the cover condition?	Servicable	
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	24"			Other notes on the ring and frame?		
What barrel size is the	48"		_	What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?]	What is the ladder	Servi	ceable
	Hydra	ulics		condition?		
Indication of surcharge?	None			bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	None		
Cleaning Needed	No		
Standing Water	No		
Flow	None		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 11:53 AM

Attributes



Asset ID SC 01

Location School Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
12	8	PVC	97
12	8	PVC	136

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		.)	<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> Depth (in.)			
7		8		AC		151		
	Manhole I	Details		Manhole Condition				
Where is the	Roadway	oadway		What is the cover condition?	Servi	cable		
manhole located?				Other notes on the cover?				
What material is the manhole?	Concrete			What is the ring and frame condition?	Servi	ceable		
What cover size is the	26"			Other notes on the ring and frame?				
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable		
Manhole? Other general				Other notes on the cone and riser?				
notes on the manhole				What is the barrel condition?	Servi	ceable		
details?	Hvdrau	llics		What is the ladder condition?	Servi	ceable		
Indication of	None			What is the bench condition?	Servi	ceable		
surcharge? Issues related to				Other notes on the bench?				
Level of infiltration?				What is the channel condition?	Servi	ceable		
				Other notes on the channel?				

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	None		
Cleaning Needed	No		
Standing Water	No		
Flow	None		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 7, 2023 10:22 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID SM 01

Location South Main Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
12	8	AC	70
9	2	PVC	43

Outlet Clock Position (Facing North)Outlet Diame		<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
9		8		AC		70
Manhole Details		Manhole Condition				
Where is the manhole located?	Woods			What is the cover condition? Other notes on the	Servi	cable
What material is the manhole?	Concret	e		cover? What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
	Hydra	ulics		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable, Obstructed
				Other notes on the channel?	Rags	in the channel

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	Yes	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 2:32 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID SM 02

Location South Main Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
12	8	PVC	48
6	8	AC	158

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)		<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
3			8		AC		158
	Manhole	Det	ails			Manho	ole Condition
Where is the	Roadwa	у			What is the cover condition?	Serv	icable
located?					on the cover?		
What material is the manhole?	Concret	e			What is the ring and frame condition?	Servi	iceable
What cover size is the	26"				Other notes on the ring and frame?		
What barrel size is the	48"			_	What is the cone and riser condition?	Servi	iceable, Cracked/Broken
Manhole? Other general				_	Other notes on the cone and riser?		
notes on the manhole					What is the barrel condition?	Servi	iceable
details?					What is the ladder	Serv	iceable
Indication of	Hydra None	ulic	S	-	What is the bench	Servi	iceable
surcharge? Issues related to					Other notes on the bench?		
Level of infiltration?					What is the channel condition?	Servi	iceable
					Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	Slight	
Cleaning Needed	No	
Standing Water	No	
Flow	None	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 2:22 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID SM 03

Location South Main Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
1	6	AC	64
2	4	PVC	58

<u>Outlet Clock</u> (Facing N	<u>Position</u> orth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
7		8		AC		63
	Manhole D	Details			Manho	le Condition
Where is the manhole	Roadway			What is the cover condition? Other notes	Servi	cable
iocateu:				cover?		
What material is the manhole?	Concrete			What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable, Misaligned
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?]	What is the ladder	Servi	ceable
	Hydrau	lics		condition?		
Indication of	None			bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	Slight	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 2:16 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID SM 04

Location South Main Street



<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
10		8		AC		79
	Manhole	Details			Manho	ole Condition
Where is the manhole located?	Roadwa	у		What is the cover condition? Other notes on the	Servi	icable
What material is the manhole?	Concret	e	_	cover? What is the ring and frame condition?	Servi	iceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	iceable, Misaligned
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	iceable
	Hvdra	ulics		What is the ladder condition?	Servi	iceable
Indication of	None			What is the bench condition?	Servi	iceable
Issues related to				Other notes on the bench?	Some	e debris
Level of infiltration?				What is the channel condition?	Servi	iceable
				Other notes on the channel?	Sewa	age in the channel

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	Moderate	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 2:08 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID SM 05

Location South Main Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
4	8	AC	85
1	8	PVC	80

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
11		8		AC		87
	Manhole	Details			Manho	ble Condition
Where is the Roadway		What is the cover condition?	Servi	cable		
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
	Hydra	ulics		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 29, 2023 1:55 PM

Attributes



Asset ID SM 06

Location South Main Street

Inlet Clock Position (Facing North)	Inlet Pipe Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
4	8	PVC	97
1	8	PVC	96

<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
11		8		PVC		99
	Manhole	Details			Manho	ole Condition
Where is the Roadway		What is the cover condition?	Servi	icable		
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	iceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	iceable, Cracked/Broken
Manhole? Other general				Other notes on the cone and riser?	Missi	ng bricks
notes on the manhole				What is the barrel condition?	Servi	iceable
	Hydra	ulics		What is the ladder condition?	Servi	iceable
Indication of	None			What is the bench condition?	Servi	iceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	iceable
				Other notes on the channel?		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	None		
Cleaning Needed	No		
Standing Water	No		
Flow	Trickle		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	


Richmond Sewer Collection System Manhole Inspection Surveyed: October 13, 2023 9:08 AM

Attributes



Asset ID SM 07

Location South Main Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
9	6	PVC	67
6	6	PVC	67

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)			<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
3		6		PVC		68
Manhole Details		Manhole Condition				
Where is the manhole located?	Grass		What is the cover condition? Other notes on the	Servi	cable	
What material is the manhole?	Concret	e		cover? What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
	S?]	What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	None			
Maintenance Needed	No			

Notes						
Structural Notes						
General Notes	Brand new manhole. Installed in 2023.					



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 8:35 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID TD 01

Location Tilden Avenue



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)			<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>				
6			8		AC		83		
	Manhole	Deta	ails		Manhole Condition				
Where is the manhole	Where is the Roadway, Other				What is the cover condition?	Servi	icable		
located?					on the cover?				
What material is the manhole?	Concret	te			What is the ring and frame condition?	Servi	iceable		
What cover size is the	26"				Other notes on the ring and frame?				
What barrel size is the	48"			-	What is the cone and riser condition?	Serviceable			
manhole? Other general					Other notes on the cone and riser?				
notes on the manhole		vdraulics			What is the barrel condition?	iceable			
details?	Hvdra				What is the ladder condition?	Servi	iceable		
Indication of	None		·		What is the bench condition?	Servi	iceable		
Issues related to					Other notes on the bench?				
Level of infiltration?					What is the channel condition?	Servi	iceable		
					Other notes on the channel?				

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	Steady				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 8:54 AM

Attributes



Powered by Esri

Asset ID TD 02

Location Tilden Avenue

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
12	8	AC	87
12	8	AC	133
10	2	PVC	105
5	8	PVC	110
5	8	PVC	139

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>				
6			8	AC		143		
Manhole Details					Manhole Condition			
Where is the Roadway			What is the cover condition?	Servi	cable			
located?				on the cover?				
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable		
What cover size is the	26"			Other notes on the ring and frame?				
What barrel size is the	60"			What is the cone and riser condition?	ition?			
Other general				Other notes on the cone and riser?				
notes on the manhole				What is the barrel condition?	Servi	ceable		
details?	Hydro	ulice		What is the ladder condition?	Servi	ceable		
Indication of	None	unes		What is the bench condition?	Servi	ceable		
Issues related to				Other notes on the bench?				
Level of infiltration?			What is the channel condition?	Servi	ceable			
				Other notes on the channel?				

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	Steady				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: October 13, 2023 9:40 AM

Attributes



Powered by Esri

Asset ID TD 03

Location Tilden Avenue



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
6			8	AC		104	
Manhole Details				Manhole Condition			
Where is the Woods			What is the cover condition?	Servi	cable		
located?				on the cover?			
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?			
What barrel size is the	48"			What is the cone and riser condition?	Serviceable		
Other general				Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
details?	Lludro	ulico		What is the ladder	Servi	ceable	
Indication of	None	uncs		What is the bench condition?	Servi	ceable	
Issues related to				Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Steady	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 9:19 AM

Attributes



Asset ID TD 04

Location Tilden Avenue

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
3	6	PVC	72
6	4	PVC	72
2	4	PVC	53

<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
12		8		PVC		75		
	Manhole	Details		Manhole Condition				
Where is the manhole	Grass			What is the cover condition? Other notes	Servi	cable		
What material is	Concret	e	_	on the cover? What is the ring and	Servi	ceable		
the manhole?				frame condition?	Jervi			
What cover size is the	26"			Other notes on the ring and frame?	Ruste	ed but still servicable		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable		
manhole? Other general			-	Other notes on the cone and riser?				
notes on the manhole				What is the barrel condition?	Servi	ceable		
	Hydra	ulics		What is the ladder condition?	Servi	ceable		
Indication of	None		-	What is the bench condition?	Servi	ceable		
Issues related to			-	Other notes on the bench?				
Level of infiltration?				What is the channel condition?	Servi	ceable, Obstructed		
				Other notes on the channel?	Some	e rags in channel		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 9:06 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID TD 05

Location Tilden Avenue

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet P</u> Diameter	i <u>pe</u> (in.)	<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
9		6		PVC		63		
	Manhole	Details		Manhole Condition				
Where is the manhole located?	Grass			What is the cover condition? Other notes on the	Servi	cable		
What material is the manhole?	Concret	e		cover? What is the ring and frame condition?	Servi	ceable		
What cover size is the	26"			Other notes on the ring and frame?				
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable		
manhole? Other general				Other notes on the cone and riser?				
notes on the manhole				What is the barrel condition?	Servi	ceable		
details?	Hydra	ulics		What is the ladder condition?	Servi	ceable		
Indication of	None			What is the bench condition?	Servi	ceable		
Issues related to				Other notes on the bench?				
Level of infiltration?				What is the channel condition?	Servi	ceable		
				Other notes on the channel?				

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	None	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 9:43 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi.... Powered by Esri

Asset ID TD 06

Location Tilden Avenue



<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
6		8	AC		102	
	Manhole	Details	Manhole Condition			
Where is the manhole located?	Roadwa	У	What is the cover condition? Other notes on the	Servi	cable	
What material is the manhole?	Concret	e	cover? What is the ring and frame condition?	Loose	e, Displaced	
What cover size is the	26"		Other notes on the ring and frame?	Ring	and frame broken.	
What barrel size is the	48"		What is the cone and riser condition?	Servi	ceable	
manhole? Other general notes on the	Ring col from fra cover.	mpletely broken me and stuck to	Other notes on the cone and riser? What is the barrel	Servi	ceable	
manhole details?			condition? What is the ladder	Servi	ceable	
Indication of	Hydra None	ulics	condition? What is the bench condition?	Servi	ceable	
surcharge? Issues related to			Other notes on the bench?			
Level of infiltration?			What is the channel condition?	Obst	ructed	
			Other notes on the channel?	A lot	of debris in channel.	

General		
Structural Condition	Fair	
Operational Condition	Good	
Debris Amount	Heavy	
Cleaning Needed	Yes	
Standing Water	No	
Flow	None	
Maintenance Needed	Yes	

	Notes
Structural Notes	Ring and frame needs replacing
General Notes	Lots of debris in channel.



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 9:51 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi ... Powered by Esri

Asset ID TD 07

Location Tilden Avenue



<u>Outlet Clock</u> (Facing N	Outlet Clock PositionOutlet Pipe(Facing North)Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
4		8		AC		105
	Manhole	Details			Manhc	le Condition
Where is the manhole	Roadwa	у		What is the cover condition? Other notes	Servi	cable
				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the manholo?	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?				What is the ladder	Servi	ceable
Indication of	Hydra None	ulics		condition? What is the bench condition?	Servi	ceable
surcharge? Issues related to				Other notes on the bench?	Some	e debris
Level of infiltration?				What is the channel condition?	Servi	ceable, Obstructed
				Other notes on the channel?	Some	e rags in the channel.

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	Slight				
Cleaning Needed	No				
Standing Water	No				
Flow	Trickle				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 5, 2023 10:12 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi ... Powered by Esri

Asset ID TD 08

Location Tilden Avenue



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
6		8		AC		115
	Manhole	Details			Manho	le Condition
Where is the manhole located?	Grass			What is the cover condition? Other notes	Servi	cable
What material is the manhole?	Concret	e		cover? What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?	Hydra	ulics		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?	Some	e debris
Level of infiltration?				What is the channel condition?	Servi	ceable, Obstructed
				Other notes on the channel?	Debr	is blocking channel

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	Moderate				
Cleaning Needed	Yes				
Standing Water	Yes				
Flow	Trickle				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 8:58 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID TH 01

Location Thompson Road



<u>Outlet Clock</u> (Facing N	<u>Position</u> orth)	<u>O</u> Dia	<u>utlet Pipe</u> meter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
3			6		AC		95
	Manhole	Details	5			Manho	ole Condition
Where is the manhole	Roadwa	у			What is the cover condition? Other notes	Servi	icable
What material is the manhole?	Concret	e			on the cover? What is the ring and frame condition?	Servi	iceable
What cover size is the	26"				Other notes on the ring and frame?		
What barrel size is the	48"				What is the cone and riser condition?	Servi	iceable
Manhole? Other general					Other notes on the cone and riser?		
notes on the manhole					What is the barrel condition?	Servi	iceable
details?	Hydra	ulics]	What is the ladder condition?	Servi	iceable
Indication of	None				What is the bench condition?	Servi	iceable
Issues related to					Other notes on the bench?		
Level of infiltration?					What is the channel condition?	Servi	iceable
					Other notes on the channel?		

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	Trickle				
Maintenance Needed	No				









Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 9:21 AM

Attributes



Asset ID TH 02

Location Thompson Road



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
4			8		AC		106
	Manhole	Deta	ails			Manho	ole Condition
Where is the manhole	Roadway		What is the cover condition? Other notes	Servi	icable		
What				_	on the cover?		
material is the manhole?	Concret	e			ring and frame condition?	Servi	iceable
What cover size is the manhole?	26"				Other notes on the ring and frame?		
What barrel size is the	48"				What is the cone and riser condition?	Servi	iceable
Manhole? Other general				_	Other notes on the cone and riser?		
notes on the manhole					What is the barrel condition?	Servi	iceable
detalls?					What is the ladder	Servi	iceable
	Hydra	ulics			condition?		
Indication of surcharge?	Minor				bench condition?	Servi	iceable
Issues related to	Debris				Other notes on the bench?		
Level of infiltration?					What is the channel condition?	Servi	iceable
					Other notes on the channel?		

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	Slight				
Cleaning Needed	No				
Standing Water	No				
Flow	Trickle				
Maintenance Needed	No				



	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 9:31 AM



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID TH 03

Location Thompson Road



<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in	<u>e</u> n.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
3		8		AC		144
Manhole Details				Manhole Condition		
Where is the manhole located?	Roadway			What is the cover condition? Other notes on the	Servi	cable
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
	Hydra	ulics		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Trickle			
Maintenance Needed	No			



Notes				
Structural Notes				
General Notes				


Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 9:41 AM





Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID TH 04

Location Thompson Road



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
4		8		AC		92
Manhole Details				Manho	le Condition	
Where is the Roadway manhole			What is the cover condition?ServicableOther notes			
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?				What is the ladder	Servi	ceable
	Hydra	ulics		condition?		
Indication of	None			bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	None			
Cleaning Needed	No			
Standing Water	No			
Flow	Steady			
Maintenance Needed	No			



	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 11:40 AM





Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID TH 05

Location Thompson Road



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
1		8		PVC		135
	Manhole	Details			Manhc	le Condition
Where is the manhole located?	Grass			What is the cover condition? Other notes on the	Servi	cable
What material is the manhole?	Concret	e		Cover? What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?	Hydra	ulics		What is the ladder condition?	Servi	ceable
Indication of	None			What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	Slight			
Cleaning Needed	No			
Standing Water	No			
Flow	None			
Maintenance Needed	No			



Notes					
Structural Notes					
General Notes	Some dirt from opening manhole				



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 9:51 AM





Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID TH 06

Location Thompson Road

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
7	8	AC	132
6	8	AC	115

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)			<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> Depth (in.)		
3			8		AC		131
	Manhole	De	etails			Manho	ole Condition
Where is the	Roadwa	У			What is the cover condition?ServicableOther notes		
located?					on the cover?		
What material is the manhole?	Concret	e			What is the ring and frame condition?	Serv	iceable
What cover size is the	26"				Other notes on the ring and frame?		
What barrel size is the	48"	8"			What is the cone and riser condition?	Serv	iceable
manhole? Other general					Other notes on the cone and riser?		
notes on the manhole					What is the barrel condition?	Serv	iceable
details?	Hydra	uli	cs]	What is the ladder condition?	Serv	iceable
Indication of	Minor				What is the bench condition?	Serv	iceable
Issues related to	Debris			_	Other notes on the bench?		
Level of infiltration?					What is the channel condition?	Obst	ructed
					Other notes on the channel?	Rags	in channel

General				
Structural Condition	Good			
Operational Condition	Good			
Debris Amount	Slight			
Cleaning Needed	Yes			
Standing Water	No			
Flow	Steady			
Maintenance Needed	No			



Notes					
Structural Notes					
General Notes	Rags on ladder rung and in channel				



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 10:22 AM



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID TH 07

Location Bridge Street

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
7	8	AC	159
9	8	AC	142

Outlet Clock Position (Facing North)Outlet Pip Diameter (i		<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>)e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
2		8	AC		160		
Manhole Details				Manhole Condition			
Where is the manhole located?	Roadwa	у	What is the cover condition?ServicableOther notes on the				
What material is the manhole?	Concret	e	cover? What is the ring and frame condition?	Servi	iceable		
What cover size is the	26"		Other notes on the ring and frame?				
What barrel size is the	48"		What is the cone and riser condition?	Servi	iceable		
Other general			Other notes on the cone and riser?				
notes on the manhole			What is the barrel condition?	Servi	iceable		
details?			What is the ladder	Servi	iceable		
Indication of	None	ulics	What is the bench condition?	Servi	iceable		
Issues related to			Other notes on the bench?				
Level of infiltration?			What is the channel condition?	Servi	iceable		
			Other notes on the channel?				

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Steady	
Maintenance Needed	No	







Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 8:31 AM

Attributes



Asset ID VG 01

Location Volunteers Green

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
11	4	AC	105
3	8	AC	82

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
11		8	AC		113	
	Manhole	Details		Manhole Condition		
Where is the Grass		What is the cover condition?	Servi	cable		
located?			on the cover?			
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	ceable	
What cover size is the manhole?	26"		Other notes on the ring and frame?			
What barrel size is the	Other		What is the cone and riser condition?	Servi	ceable	
Other general	manhole?Other general notes on theOpening is offset to inlet 1 and outlet		Other notes on the cone and riser?	Not a	pplicable precast vault	
notes on the manhole			What is the barrel condition?	Servi	ceable	
details?			What is the ladder	Servi	ceable	
Indication	Hydra	ulics	condition? What is the	Soni	caphla	
of	None		bench condition?	Servi		
Issues related to			Other notes on the bench?			
Level of infiltration?			What is the channel condition?	Servi	ceable	
			Other notes on the channel?			

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	Slight	
Cleaning Needed	Yes	
Standing Water	No	
Flow	Steady	
Maintenance Needed	No	



	Notes
Structural Notes	
General Notes	





Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 1:07 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID VG 02

Location Volunteers Green Access Road

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
6	8	AC	92
3	8	PVC	76

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> <u>Diameter (in.)</u>	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
12		8	AC		94	
	Manhole	Details		Manhole Condition		
Where is the Roadway		What is the cover condition?	Servi	cable, Damaged		
manhole located?			Other notes on the cover?	Ruste	ed	
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	ceable	
What cover size is the manholo?	26"		Other notes on the ring and frame?			
What barrel size is the	48"		What is the cone and riser condition?	Servi	ceable, Misaligned	
Other general	manhole? Other general notes on blocked overflow to river the manhole		Other notes on the cone and riser?			
notes on the manhole			What is the barrel condition?	Servi	ceable	
details?	Hydra	ulics	What is the ladder condition?	Servi	ceable	
Indication of	None		What is the bench condition?	Servi	ceable	
Issues related to			Other notes on the bench?	Could	d use some work	
Level of infiltration?			What is the channel condition?	Servi	ceable	
			Other notes on the channel?			

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Steady	
Maintenance Needed	Yes	



Notes			
Structural Notes Frame and cover misaligned			
General Notes	Bricked off overflow to the river		



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 1:16 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID VG 03

Location Volunteers Green Access Road

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
4	8	AC	107
1	4	AC	108

<u>Outlet Clock</u> (Facing N	Position orth)	<u>Ou</u> Diar	<u>itlet Pipe</u> neter (in.)	<u>Outlet Pi</u> <u>Materia</u>	<u>pe</u> I	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
10			8	AC		109		
	Manhole	Details			Manhole Condition			
Where is the Grass		What is the cover condition?	Servi	cable				
manhole located?				Other notes on the cover?	Rusti	ng		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable		
What cover size is the	26"			Other notes on the ring and frame?	Rusti	ng		
What barrel size is the	60"			What is the cone and riser condition?	Servi	ceable		
Other general	Deized			Other notes on the cone and riser?				
the manhole details?	Raiseu	Jover		What is the barrel condition?	Servi	ceable		
	Hydra	ulics		What is the ladder condition?	Servi	ceable		
Indication of	None			What is the bench condition?	Servi	ceable		
Issues related to				Other notes on the bench?				
Level of infiltration?				What is the channel condition?	Servi	ceable		
				Other notes on the channel?	Debr	is		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	Slight	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	



	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 20, 2023 1:24 PM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID VG 04

Location Volunteers Green Access Road



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)			<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
10		8		AC		104
	Manhole	Details			Manho	ole Condition
Where is the manhole located?	Grass		-	What is the cover condition? Other notes	Servi	cable
What material is the manhole?	Concret	e	-	what is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"		-	What is the cone and riser condition?	Servi	ceable
manhole? Other general			-	Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
details?	Hydra	ulics]	What is the ladder condition?	Servi	ceable
Indication of	Minor		-	What is the bench condition?	Servi	ceable
Issues related to	Debris,	Silt	-	Other notes on the bench?	Debr	is from overflow
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	Slight	
Cleaning Needed	No	
Standing Water	No	
Flow	Trickle	
Maintenance Needed	No	



	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: October 13, 2023 8:20 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID VG 05

Location WWTF



<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
5		20		AC		83
	Manhole	Details			Manho	ble Condition
Where is the manhole	Other			What is the cover condition? Other notes	Servi	cable
located?				on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
	Hydra	ulics		What is the ladder condition?	Servi	ceable
Indication of	None		_	What is the bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	Slight	
Cleaning Needed	No	
Standing Water	No	
Flow	Steady	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: July 7, 2023 8:14 AM



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi.... Powered by Esri

Asset ID VG 06

Location Volunteers Green



Outlet Clock Position	<u>Outlet Pipe</u>
(Facing North)	Diameter (in.)
7	12

Outlet Pipe Material AC Outlet Pipe Invert Depth (in.)

Manhole Details		
Where is the manhole located?	Woods	
What material is the manhole?	Concrete	
What cover size is the manhole?	26"	
What barrel size is the manhole?	48"	
Other general notes on the manhole details?		

Hydraulics		
Indication of surcharge?	None	
lssues related to surcharge?		
Level of infiltration?		

Manhole Condition		
What is the cover condition?	Servicable	
Other notes on the cover?		
What is the ring and frame condition?	Serviceable	
Other notes on the ring and frame?		
What is the cone and riser condition?	Serviceable, Misaligned	
Other notes on the cone and riser?	Frame and cover are slighly off but it would be difficult to reset due to presence of trees.	
What is the barrel condition?	Serviceable	
What is the ladder condition?	Serviceable	
What is the bench condition?	Serviceable	
Other notes on the bench?	N/A	
What is the channel condition?	Serviceable	
Other notes on the channel?		

General		
Structural Condition	Good	
Operational Condition	Good	
Debris Amount	None	
Cleaning Needed	No	
Standing Water	No	
Flow	Steady	
Maintenance Needed	No	

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 8:57 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID WW 01

Location WWTF

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> <u>Diameter (in.)</u>	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
	Manhole D	etails		Manhc	le Condition
Where is the manhole	ere is Roadway hole ted?		What is the cover condition? Other notes	Servi	cable
located?			on the cover?		
What material is the manhole?	Concrete		What is the ring and frame condition?	Servi	ceable
What cover size is the manhole?	26"		Other notes on the ring and frame?		
What barrel size is the	48"		What is the cone and riser condition?	Servi	ceable
Other general	Abandone	d. No inlets or	Other notes on the cone and riser?		
the manhole details?	outlets.		What is the barrel condition?	Servi	ceable
			What is the ladder	Servi	ceable
Hydraulics		condition? What is the			
Indication of	None		bench condition?		
Issues related to			Other notes on the bench?	N/A	
Level of infiltration?			What is the channel condition?		
			Other notes on the channel?	N/A	

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	Moderate		
Cleaning Needed			
Standing Water	Yes		
Flow	None		
Maintenance Needed			

Notes				
Structural Notes				
General Notes	Standing water. Manhole abandoned. No inlets or outlets.			



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 9:05 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID WW 02

Location WWTF

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
3	12	AC	228
6	4	AC	215

<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
9		12	AC		227
Manhole Details		Manhole Condition			
Where is the Roadway, Grass manhole located?		What is the cover condition?	Servi	icable	
		Other notes on the cover?			
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	iceable
What cover size is the manhole?	26"		Other notes on the ring and frame?		
What barrel size is the	48"		What is the cone and riser condition?	Serviceable	
Other general			Other notes on the cone and riser?		
notes on the manhole			What is the barrel condition?	Servi	iceable
detalls?	Lludra	ulice	What is the ladder	Servi	iceable
Indication of	None		What is the bench condition?	Servi	iceable
Issues related to			Other notes on the bench?		
Level of infiltration?			What is the channel condition?	Servi	iceable
			Other notes on the channel?		
General					
--------------------------	--------	--	--		
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	Steady				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 9:22 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID WW 03

Location WWTF

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
3	12	AC	200
4	8	AC	157
6	8	AC	178

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
9		12	AC		202	
	Manhole	Details		Manhole Condition		
Where is the Grass, In Sump/Depression		What is the cover condition?	Servi	cable		
manhole located?			Other notes on the cover?			
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"		Other notes on the ring and frame?			
What barrel size is the	48"		What is the cone and riser condition?	Servi	ceable	
Other general			Other notes on the cone and riser?			
notes on the manhole			What is the barrel condition?	Servi	ceable	
details?			What is the ladder	Servi	ceable	
	Hydra	oulics	condition? What is the			
Indication of surcharge?	Minor		bench condition?	Servi	ceable	
Issues related to	Debris		Other notes on the bench?			
Level of infiltration?			What is the channel condition?	Servi	ceable	
			Other notes on the channel?			

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	Slight		
Cleaning Needed	No		
Standing Water	No		
Flow	Steady		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 9:31 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID WW 04

Location WWTF

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
3	12	AC	201
9	12	AC	201

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>)e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
6		12	AC		202	
	Manhole	Details		Manhole Condition		
Where is the	Where is Grass, In Sump/Depression, Does		What is the cover condition?	Servi	cable	
located?	Grade 5	юре Амау	on the cover?			
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"		Other notes on the ring and frame?			
What barrel size is the	48"		What is the cone and riser condition?	Servi	ceable	
manhole? Other general			Other notes on the cone and riser?			
notes on the manhole			What is the barrel condition?	Servi	ceable	
	Hydra	ulics	What is the ladder condition?	Servi	ceable	
Indication of	Minor		What is the bench condition?	Servi	ceable	
Issues related to	Debris,	Silt	Other notes on the bench?			
Level of infiltration?			What is the channel condition?	Servi	ceable	
			Other notes on the channel?			

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	Slight		
Cleaning Needed	No		
Standing Water	No		
Flow	Steady		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 9:43 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID WW 05

Location WWTF

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
9	12	AC	202
6	4	AC	84

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)			<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>		
3		12		AC		205	
	Manhole	Details		Manhole Condition			
Where is the maphala Grass, Does Grade Slope			What is the cover condition?	Servi	cable		
located?				on the cover?			
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable	
What cover size is the	26"			Other notes on the ring and frame?			
What barrel size is the	48"		-	What is the cone and riser condition?	Servi	ceable	
manhole? Other general			-	Other notes on the cone and riser?			
notes on the manhole				What is the barrel condition?	Servi	ceable	
details?]	What is the ladder	Servi	ceable	
	Hydra	ulics		condition?			
Indication of surcharge?	Minor			bench condition?	Servi	ceable	
Issues related to	Silt			Other notes on the bench?			
Level of infiltration?				What is the channel condition?	Servi	ceable	
				Other notes on the channel?			

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	Slight		
Cleaning Needed	No		
Standing Water	No		
Flow	Steady		
Maintenance Needed	No		

	Notes
Structural Notes	
General Notes	



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 9:53 AM



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID WW 06

Location WWTF

Inlet Clock Position (Facing North)	Inlet Pipe Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
9	12	AC	201
11	8	PVC	51
8	4	AC	29

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outl</u> Diam	<u>et Pipe</u> eter (in.)	<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
3			12	AC		204
	Manhole	Details		Manhole Condition		
Where is the	Where is he Grass, Does Grade Slope		What is the cover condition?	Servi	cable	
manhole located?	, may			Other notes on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable
What cover size is the	26"			Other notes on the ring and frame?		
What barrel size is the	48"			What is the cone and riser condition?	Servi	ceable
Manhole? Other general				Other notes on the cone and riser?		
notes on the manhole				What is the barrel condition?	Servi	ceable
detalls?				What is the ladder	Servi	ceable
	Hydra	ulics		condition?		
Indication of surcharge?	None			bench condition?	Servi	ceable
Issues related to				Other notes on the bench?		
Level of infiltration?				What is the channel condition?	Servi	ceable
				Other notes on the channel?		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	None		
Cleaning Needed	No		
Standing Water	Yes		
Flow	Trickle		
Maintenance Needed	No		



Notes				
Structural Notes				
General Notes	Slow moving			



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 10:11 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID WW 07

Location WWTF

Inlet Clock Position (Facing North)	Inlet Pipe Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
6	4	AC	186
6	8	PVC	33

<u>Outlet Clock</u> (Facing N	<u>Position</u> Iorth)	<u>Outlet Pipe</u> Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>
3		12		AC		189
	Manhole	Details		Manhole Condition		
Where is the	Where is the Away		What is the cover condition?	Servi	icable	
located?	_			on the cover?		
What material is the manhole?	Concret	e		What is the ring and frame condition?		
What cover size is the	26"			Other notes on the ring and frame?	Corro	oded
What barrel size is the	48"			What is the cone and riser condition?	Servi	iceable
manhole? Other general				Other notes on the cone and riser?	Debr	is covering cone
notes on the manhole				What is the barrel condition?	Servi	iceable
detalls?				What is the ladder	Servi	iceable
	Hydra	iulics		condition? What is the		
Indication of	None			bench condition?	Servi	iceable
Issues related to				Other notes on the bench?	Debr	is covering bench
Level of infiltration?				What is the channel condition?	Servi	iceable
				Other notes on the channel?		

General			
Structural Condition	Good		
Operational Condition	Good		
Debris Amount	Heavy		
Cleaning Needed	Yes		
Standing Water	No		
Flow	Неаvy		
Maintenance Needed	No		



	Notes
Structural Notes	
General Notes	Needs cleaning. Manhole coated in debris



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 10:11 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID WW 08

Location WWTF

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)
6	12	AC	180
7	8	AC	166

<u>Outlet Clock</u> (Facing N	<u>Position</u> lorth)	<u>Outlet Pipe</u> Diameter (in.)	<u>Outlet Pip</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>	
12		12	AC		182	
	Manhole	Details		Manhole Condition		
Where is the	Where is the Roadway		What is the cover condition?	Holes	5	
manhole located?			Other notes on the cover?	Catch basin style cover		
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	ceable	
What cover size is the manhole?	26"		Other notes on the ring and frame?			
What barrel size is the	48"		cone and riser condition?	Serviceable		
Other general	Open se	ewer manhole	Other notes on the cone and riser?			
notes on the manhole	connect	ed with storm	What is the barrel condition?	Servi	ceable	
detalls?	Hydro	ulice	What is the ladder	Servi	ceable	
Indication of	None		What is the bench condition?	Servi	ceable	
Issues related to			Other notes on the bench?			
Level of infiltration?			What is the channel condition?	Servi	ceable	
			Other notes on the channel?			

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	Moderate				
Cleaning Needed	No				
Standing Water	Yes				
Flow	Trickle				
Maintenance Needed	No				



	Notes
Structural Notes	
General Notes	Needs cover replacement



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 10:22 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID WW 09

Location WWTF

Inlet Clock Position (Facing North)	<u>Inlet Pipe</u> Diameter (in.)	<u>Inlet Pipe</u> <u>Material</u>	Inlet Pipe Invert Depth (in.)

Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)				<u>Outlet Pipe</u> <u>Material</u>	<u>e</u>	<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>			
3		12		AC		128			
	Manhole	Details		Manhole Condition					
Where is the	Roadwa	Roadway		What is the cover condition?	Servicable, Holes				
manhole located?				Other notes on the cover?	Catch basin style cover				
What material is the manhole?	Concret	e		What is the ring and frame condition?	Servi	ceable			
What cover size is the manhole?	'hat over size 26" the			Other notes on the ring and frame?					
What barrel size is the	What barrel size 48" is the			what is the cone and riser condition?	Servi	ceable			
Other general				Other notes on the cone and riser?					
the manhole	No met	5		What is the barrel condition?	Serviceable				
	Hydra	nulics		What is the ladder condition?	Servi	ceable			
Indication of	Indication of surcharge? Issues related to			What is the bench condition?					
Issues related to				Other notes on the bench?	N/A				
Level of infiltration?				What is the channel condition?					
				Other notes on the channel?	N/A				

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	Moderate				
Cleaning Needed	No				
Standing Water	Yes				
Flow	None				
Maintenance Needed	No				

Notes							
Structural Notes							
General Notes	Catch basin style cover						



Richmond Sewer Collection System Manhole Inspection Surveyed: June 21, 2023 8:39 AM

Attributes



Chittenden County RPC, VCGI, Esri Canada, Esri, HERE, Garmi... Powered by Esri

Asset ID WW 10

Location WWTF



Outlet Clock Position (Facing North)Outlet Pipe Diameter (in.)		<u>Outlet Pipe</u> <u>Material</u>		<u>Outlet Pipe Invert</u> <u>Depth (in.)</u>				
6		12	AC		114			
	Manhole	Details		Manhole Condition				
Where is the Grass			What is the cover condition?	Servicable				
manhole located?			Other notes on the cover?					
What material is the manhole?	Concret	e	What is the ring and frame condition?	Servi	ceable			
What cover size is the manholo?	26"		Other notes on the ring and frame?					
What barrel size is the	<u>?</u> 2e 48"		What is the cone and riser condition?	What is the cone and Serviceable riser condition?				
Other general			Other notes on the cone and riser?					
notes on the manhole			What is the barrel condition?	Servi	ceable			
details?	Hydra	ulics	What is the ladder Serviceable		ceable			
Indication of	Indication of None		What is the bench condition?	Servi	ceable			
Issues related to			Other notes on the bench?					
Level of infiltration?		What is the channel condition?	Servi	ceable				
		Other notes on the channel?						

General					
Structural Condition	Good				
Operational Condition	Good				
Debris Amount	None				
Cleaning Needed	No				
Standing Water	No				
Flow	Steady				
Maintenance Needed	No				

	Notes
Structural Notes	
General Notes	



APPENDIX F COST ESTIMATES

Hoyle, Tanner	Town of Richmond, VT		Project No.:	102	501
125 College St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrade		By:	KDW	1
Burlington, VT 05401	Engineer's Opinion of Probable Project Costs		CK By:	JO	
802-860-1331	Total Project Cost	Date: 1/11/2024			
		Co	sts ¹ ENR	Pro	jected Costs ²
Process Area			= 13514.76	E	NR = 15800
			(Dec 2023)	(/	April 2027)
Collection System Upgrades	5	Ē			
Collection System Rehabili	itation and Maintenance	\$	65,000	\$	76,000
	Collection System Upgrades Subtotal	\$	65,000	\$	76,000
				<u> </u>	
WWTF Upgrade		<u>ج</u>	499.000	~	571.000
Influent Pumping Opgrade	Multivelys Carpon	ې د	488,000	ې د	1 024 000
Apovic Selector Ungrade A	Willflake Screen	ې د	72 000	ې د	1,954,000
Riological Process Ungrade		ې د	462 000	ې د	541 000
RAS System Ungrade		Ś	57.000	ې د	67,000
WAS Pumping System Upg	zrade	Ś	95,000	Ś	112,000
Filtration System Upgrade		\$	217,000	\$	254,000
UV System Upgrade		\$	230,000	\$	269,000
Effluent Flow Measuremen	nt	\$	15,000	\$	18,000
Operations Building		\$	686,000	\$	802,000
Site		\$	545,000	\$	638,000
	WWTF Upgrade Subtotal	\$	4,521,000	\$	5,291,000
Flood Mitigation Upgrade		Ļ		Ļ	
Bridge Street Pump Station	n Upgrade	\$	1,068,000	\$	1,249,000
Effluent Pump Station		Ş	514,000	Ş	601,000
FIOOD DOORS	Eland Mitigation Ungrado Subtotal	ې د	26,000	ې د	31,000
	Flood Mitigation Opgrade Subtotal	Ş	1,608,000	Ş	1,881,000
Septage Receiving Facilities	: Upgrade				
Septage Receiving Unit Up	perade	\$	871,000	\$	1.019,000
Septage & Sludge Storage	Alt. 2 - Compressed Gas Mixing, Transfer Pumps, Odor Control	\$	821,000	\$	960,000
Dewatering Facilities Upgr	ade	\$	2,137,000	\$	2,499,000
	Septage Receiving Facilities Upgrade Subtotal	\$	3,829,000	\$	4,478,000
	Construction Cost Subtotal ¹	\$	10,023,000	\$	11,726,000
Engineering & Construction	Contingency @ 30%	\$	3,007,000	\$	3,518,000
	Total Construction Cost ⁴	\$	13,030,000	\$	15,244,000
Engineering Costs					
Preliminary Engineering	- Step I ³		\$154,000	L	\$154,000
Pre-Design Phase (Surve	y, Geotechnical, Pre-Procurement, Funding Assistance)	└──	\$87,000		\$87,000
WWTF Upgrade Final De	WWTF Upgrade Final Design - Step II ⁴				\$385,000
Flood Mitigation Final De	Flood Mitigation Final Design - Step II ⁴				\$210,000
Septage Upgrade Final D		\$326,000		\$326,000	
Bid, Construction Admin		\$1,245,000		\$1,456,000	
Legal, Administrative, Pe		\$65,000		\$76,000	
	Total Project Cost		\$15,502,000		\$17,938,000
Notes:					
1) ENR Construction Cost Inc	dex = 13514.76 (December 2023)				
ENR Construction Cost Inc	dex projected for April 2027 = 15800				

3) Executed contract dated 3/21/2023
4) Engineering Fee is calculated based on percentage of construction value and past project experience

Hoyle, Tanne	Tanner Town of Richmond, VT					Project No.:	102601
125 College S	it., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrade					KDW
Burlington, V	T 05401	Engineer's Opinion of Probable Project Cos	Engineer's Opinion of Probable Project Costs				
802-860-1331	1	Influent Pumps Replacement				Date:	12/30/2023
	-						
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost
Influent Pum	ps Replaceme	nt					
	Site/Civil						
		N/A					
	Structural						
		N/A					
	Process Mech	nanical					
		Existing Pump & Piping Demolition	1	LS	\$10,000		\$10,000
		New Influent Pumps (3 - 17 HP dry-pit submersible non-clog pumps)		LS	\$150,000	30%	\$195,000
		Influent Pump Main Control Panel			\$53,300	30%	\$69,300
	Process Piping and Valves Modifications Allowance		1	LS	\$50,000		\$50,000
	Buidling Mec	dling Mechanical					
		N/A					
	Electrical/I&C						
		Influent Pump Variable Frequency Drives (3)	1	LS	\$25,100		\$25,100
		Magnetic Flow Meter	1	LS	\$10,000		\$10,000
		Intermediate Pump Level Control System with back-up floats	1	LS	\$15,000		\$15,000
		Suction and Discharge Pressure Sensors	6	EA	\$1,000		\$6,000
		Pump Process Electrical	1	LS	\$10,000		\$10,000
		I&C Equipment and Integration Allowance	1	LS	\$10,000		\$10,000
		Influent Pumps Replacement - Subtotal					\$400,400
				Cons	truction Subto	tal (Rounded)	\$400,000
Contractor N	larkups						
		Contractor Overhead & Profit	15%				\$60,000
		Mobilization/Demobilization	5%				\$20,000
		Bonds & Insurance	2%				\$8,000
					Total Cons	struction Cost	\$488,000
Notes: 1.	ENR Construc	tion Cost Index = 13514.76 (December 2023)					

Hoyle, Tanner	·	Town of Richmond, VT				Project No.:	102601
125 College St	t., 4th Floor	Richmond WWTF - 20 Year Evaluation	Upgrade	_		By:	KDW
Burlington, VI	05401	Engineer's Opinion of Probable Project	ct Costs			CK By:	
802-860-1331		Headworks Alternative 1 - Replace-i	n-Kind			Date:	12/30/2023
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost
Headworks -	Replacement	In-Kind					
	Site/Civil						
		N/A					
	Structural						
		Building modifications to install replacement equipment	1	LS	\$50,000		\$50,000
	Process Mec	hanical					
		Existing Screen/Grit Package Equipment Demolition	1	LS	\$10,000		\$10,000
		Lakeside Headworks Acceptance Plant	1	LS	\$505,000	30%	\$656,500
		Process piping and valve modifications	1	LS	\$25,000		\$25,000
	Buidling Med	hanical					
		HVAC Upgrade	1	LS	\$75,000		\$75,000
	Electrical/I&0						
		Gas Detection System	1	LS	\$15,000		\$15,000
		Process Electrical & Instrumentation	1	LS	\$15,000		\$15,000
		Headworks Replacement In-Kind - Subtotal				Subtotal	\$846,500
				Constr	uction Subtot	al (Rounded)	\$847,000
Contractor M	arkups						
		Contractor Overhead & Profit	15%				\$127,000
		Mobilization/Demobilization	5%				\$42,000
		Bonds & Insurance	2%				\$17,000
					Total Cons	truction Cost	\$1,033,000
Notes: 1.	ENR Construe	ction Cost Index = 13514.76 (December 2023)					

Hoyle, Tanner		Town of Richmond, VT				Project No.:	102601
125 College St.,	4th Floor	Richmond WWTF - 20 Year Evaluation Upg	rade			By:	KDW
Burlington, VT (05401	Engineer's Opinion of Probable Project Co	osts			CK By:	
802-860-1331		Headworks Alternative 2 - Multi-Rake Screen Ahead of Influent	t Pumping &	Grit Class	sifier	Date:	12/30/2023
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost
New Headwork	s Building at	Influent Wet Well					
S	Site/Civil						
		Potable Water Service Extension to Building	1	LS	\$5,000		\$5,000
S	Structural						
		New Headworks Building (29' x 13.5')	392	SF	\$250		\$98,000
		Upper Level Concrete Slab	15	CY	\$3,000		\$45,000
		Lower Level Channel Modifications	1	LS	\$20,000		\$20,000
P	Process Mech	anical					
		Multi-Rake Bar Screen & Wash Compactor (includes control panels)	1	LS	\$235,000	30%	\$305,500
		Process Piping and Valves	1	LS	\$10,000		\$10,000
В	Buidling Mech	nanical					
		HVAC & Plumbing	1	LS	\$75,000		\$75,000
E	Electrical/I&C						
		Gas Detection System	1	LS	\$15,000		\$15,000
		Building Electrical Panels	1	LS	\$15,000		\$15,000
		Building Basic Electrical	1	LS	\$75,000		\$75,000
		Operations Building PLC Panel & Wiring	1	LS	\$15,000		\$15,000
		New Headworks Building at Influent Wet Well - Subtotal				Subtotal	\$678,500
Grit Removal Sy	ystem Replac	ement					
S	lite/Civil						
		N/A					
5	structural		4	10	¢50.000		¢50.000
		Building modifications to install replacement equipment	1	LS	\$50,000		\$50,000
P	rocess ween	anical	1	10	¢10.000		¢10.000
		Existing Screen/Grit Package Equipment Demolition	1	LS	\$10,000	200/	\$10,000
		Lakeside Headworks Acceptance Plant	1	LS	\$375,000	30%	\$487,500
P	uidling Mach		1	LS	\$25,000		\$25,000
B	suidling ween		1	15	¢75.000		Ś75 000
	loctrical/18.C	nvac opgrade	1	LS	\$75,000		\$75,000
		Gas Datastian System	1	15	\$15,000		\$15,000
		Process Electrical & Instrumentation	1	15	\$15,000		\$15,000
		Grit Removal System Penlacement - Subtotal	1	LJ	\$15,000	Subtotal	\$677 500
		Grit Kemoval System Replacement - Subtotal		Conc	truction Subto	Fal (Roundod)	\$1,256,000
Contractor Mar	dunc			Cons		lai (Kouliueu)	\$1,330,000
	rups	Contractor Overhead & Profit	15%				\$202.000
		Mohilization /Demohilization	5%				203,000 \$68,000
		Ronds & Insurance	3% 2%				\$00,000 \$27 000
			2/0		Total Con	struction Cost	\$1 654 000
Notos:							91,094,000
1. E	NR Construct	tion Cost Index = 13514.76 (December 2023)					

Hoyle, Ta	nner	Town of Richmond, VT				Project No.:	102601
125 Colle	ge St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrade				By:	KDW
Burlingto	on, VT 05401	Engineer's Opinion of Probable Project Costs				CK By:	
802-860-	1331	Anoxic Selectors - Alternative #1 Submersible Mixers				Date:	12/30/2023
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost
Anoxic S	Anoxic Selector Submersible Mixers						
	Site/Civil						
		N/A		CY			
	Structural						
		N/A		EA			
	Process Mechan	ical					
		Submersible Mixers	3	EA	\$8,051	30%	\$31,400
	Buidling Mechar	nical					
		N/A					
	Electrical/I&C						
		Process Electrical & Instrumentation	1	LS	\$10,000		\$10,000
		Anoxic Selector Submersible Mixers - Subtotal					\$41,400
			Construction Subt		tion Subto	tal (Rounded)	\$41,000
Contract	or Markups						
		Contractor Overhead & Profit	15%				\$6,000
		Mobilization/Demobilization	5%				\$2,000
		Bonds & Insurance	2%				\$1,000
					Total Cons	truction Cost	\$50,000
Notes: 1	. ENR Constructio	n Cost Index = 13514.76 (December 2023)					

Hoyle, Ta	nner	Town of Richmond, VT				Project No.: 1			
125 Colle	ge St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrade				By:	KDW		
Burlingto	n, VT 05401	Engineer's Opinion of Probable Project Costs				CK By:			
802-860-	1331	Anoxic Selectors - Alternative #2 Compressed Gas Mixing System	n			Date:	12/30/2023		
Process Area	Division/ Discipline	Description No. Of Unit Unit Cost					Total Cost		
Anoxic Se	Anoxic Selector Submersible Mixers								
	Site/Civil								
		N/A		CY					
	Structural								
		N/A		EA					
	Process Mechan	ical							
		Compressed Gas Mixing System (includes all valves, headers, valve module control panel)	1	EA	\$34,000	30%	\$44,200		
	Buidling Mechar	ical							
		N/A							
	Electrical/I&C								
		Process Electrical & Instrumentation	1	LS	\$15,000		\$15,000		
		Anoxic Selector Submersible Mixers - Subtotal					\$59,200		
				Const	ruction Subt	otal (Rounded)	\$59,000		
Contracto	or Markups								
		Contractor Overhead & Profit	15%				\$9,000		
		Mobilization/Demobilization	5%				\$3,000		
		Bonds & Insurance	2%				\$1,000		
					Total Con	struction Cost	\$72,000		
Notes: 1.	ENR Constructio	n Cost Index = 13514.76 (December 2023)							

Hoyle, Tanner		Town of Richmond, VT				Project No.:	102601
125 Colle	ge St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgra	de			KDW	
Burlingto	n, VT 05401	Engineer's Opinion of Probable Project Cost	ts			CK By:	
802-860-	1331	Biological Process Upgrades				Date:	1/11/2024
	-						
Process	Division/	Description	No. Of	Unit	Unit Cost	Install	Total Cost
Area	Discipline		Units	0	onit cost	motan	Total cost
Biologica	l Process Upgrade	s					
	Site/Civil						
		6" Stainless Steel Air Piping (independent air header to aeration tanks	120	LF	\$200		\$24,000
	Structural						
		N/A					
	Process Mechani	cal					
		Demolition of existing blowers and air intake piping	1	LS	\$15,000		\$15,000
		New Blowers (Positive Displacment, includes VFD and sound enclosures)	2	EA	\$70,000	30%	\$182,000
		New Air Intake Silencer/Filter	1	EA	\$15,000		\$15,000
		Air Piping and Valves Modifications (indoors)	1	LS	\$25,000		\$25,000
		Demolition of existing diffused aeration system	1	LS	\$15,000		\$15,000
		New Diffusers	1	LS	\$41,000	30%	\$53,300
	Building Mechan	ical					
		N/A					
	Electrical/I&C						
		New Airflow Meter	1	LS	\$15,000		\$15,000
		D.O. Probes	2	EA	\$4,000		\$8,000
		Process Electrical & Instrumentation	1	LS	\$25,000		\$25,000
		Biological Process Modifications - Subtotal					\$377,300
				Co	nstruction Subto	otal (Rounded)	\$378,000
Contract	or Markups						
		Contractor Overhead & Profit	15%				\$57,000
		Mobilization/Demobilization	5%				\$19,000
		Bonds & Insurance	2%				\$8,000
	Total Co				Total Con	struction Cost	\$462,000
Notes: 1	. ENR Constructior	n Cost Index = 13514.76 (December 2023)					

Hoyle, Tanner		Town of Richmond, VT				Project No.:	102601
125 Colle	ge St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrade				By:	KDW
Burlingto	n, VT 05401	Engineer's Opinion of Probable Project Costs				CK By:	
802-860-	1331	Chemical Storage and Feed Improvements				Date:	1/11/2024
Process	Division/	Description	No. Of	Unit	Unit Cost	Install	Total Cost
Area	Discipline	Description	Units	onne	onit cost	motum	Total Cost
Existing	Chemical Storage	and Feed System Improvements					
	Site/Civil	N/A					
	Structural						
		Concrete - Raised containment wall (8" height)	1	CY	\$1,400		\$1,400
	Process Mechan	ical					
		Biological Process Coagulant Pump Skid	1	EA	\$13,000	30%	\$16,900
		Chemical Feed Piping and Valves Allowance	1	EA	\$5,000		\$5,000
	Electrical/I&C						
		Electrical, Instrumentation, & Controls	1	LS	\$5,000		\$5,000
		Existing Chemical Storage and Feed System - Subtotal					\$28,300
					Subt	total (Rounded)	\$28,000
Contract	or Markups						
		Contractor Overhead & Profit	15%				\$4,000
		Mobilization/Demobilization	5%				\$1,000
		Bonds & Insurance	2%				\$1,000
					Total Co	nstruction Cost	\$34,000
Notes: 1	. ENR Constructio	n Cost Index = 13514.76 (December 2023)					

Hoyle, Tanner		Town of Richmond, VT				Project No.:	102601
125 College St., 4th Floor		Richmond WWTF - 20 Year Evaluation Upgrad	de			By:	KDW
Burlington, VT 05401		Engineer's Opinion of Probable Project Costs	5			CK By:	
802-860-	1331	RAS System Replacement				Date:	1/11/2023
Process	Division/	Description	No. Of	Unit	Unit Cost	Install	Total Cost
Area	Discipline		Units	•	0		
RAS Syst	em Replacement						
	Site/Civil						
		N/A					
	Structural						
		N/A					
	Process Mechanical						
		Existing Pinch Valve & Flow Meter Demolition	1	LS	\$1,000		\$1,000
		Pinch Valves & Electric Actuators	1	EA	\$25,000	30%	\$32,500
	Electrical/I&C						
		Magnetic Flow Meter for RAS Discharge	1	EA	\$10,000	30%	\$13,000
		RAS System Replacement - Subtotal					\$46,500
					Subto	tal (Rounded)	\$47,000
Contract	or Markups						
		Contractor Overhead & Profit	15%				\$7,000
		Mobilization/Demobilization	5%				\$2,000
		Bonds & Insurance	2%				\$1,000
					Total Cons	struction Cost	\$57,000
Notes:							
1.	ENR Constructio	n Cost Index = 13514.76 (December 2023)					

Hoyle, Tanner		Town of Richmond, VT				Project No.:	102601	
125 Colle	ge St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgra	ade			By:	KDW	
Burlington, VT 05401		Engineer's Opinion of Probable Project Cos	ts			CK By:		
802-860-	1331	WAS Pump System - Replace In-Kind				Date:	1/11/2024	
		·						
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost	
WAS Pur	np Replacement							
	Site/Civil							
		N/A						
	Structural							
		N/A						
	Process Mechan	ical						
		Existing Pump Demolition	1	LS	\$5,000		\$5,000	
		Double Disc WAS Pump	1	EA	\$34,700	30%	\$45,200	
		Suction Pressure Sensing Assembly	1	LS	\$1,400		\$1,400	
		Discharge Pressure Switch Assembly	1	LS	\$1,400		\$1,400	
		Valve Replacements	6	EA	\$4,000		\$24,000	
	Building Mechar	ical						
		N/A						
	Electrical/I&C							
		N/A						
		WAS System Rehabilitation - Subtotal					\$77,000	
					Su	Subtotal (Rounded)		
Contract	or Markups							
		Contractor Overhead & Profit	15%				\$12,000	
		Mobilization/Demobilization	5%				\$4,000	
		Bonds & Insurance	2%				\$2,000	
			Total Construction Cost			\$95,000		
Notes: 1	. ENR Constructio	n Cost Index = 13514.76 (December 2023)						
Hoyle, Tanner Town of Richmond, VT						Project No.:	102601	
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125 Colle	ge St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgra	de			By:	KDW	
Burlingto	n, VT 05401	Engineer's Opinion of Probable Project Cos	ts			CK By:		
802-860-	1331	Filter System Improvements				Date:	1/11/2024	
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost	
Cloth Dis	k Filter Replacem	ent						
	Site/Civil							
		N/A						
	Structural							
		Structural Assessment of Steel Tanks	1	LS	\$15,000		\$15,000	
		Structural Repair of Tanks	1	LS	\$25,000		\$25,000	
	Process Mechan	ical						
		Filter Motor Drive Replacements	2	EA	\$10,000	30%	\$26,000	
		Sludge Removal Pump Replacements	2	EA	\$8,368	30%	\$21,800	
		Backwash Pump Replacements	2	EA	\$8,238	30%	\$21,500	
		Filter Cloth Replacements	24	EA	\$350	30%	\$11,000	
		Valve actuator replacement	2	EA	\$1,424	30%	\$3,800	
		Misc. Piping and Valves	1	LS	\$25,000	30%	\$32,500	
	Electrical/I&C							
		Instrumentation Allowance	1	LS	\$10,000		\$10,000	
		SCADA Programming	1	LS	\$10,000		\$10,000	
		Cloth Disk Filter Replacement - Subtotal					\$176,600	
					Subtot	al (Rounded)	\$177,000	
Contract	or Markups							
		Contractor Overhead & Profit	15%				\$27,000	
		Mobilization/Demobilization	5%				\$9,000	
		Bonds & Insurance	2%				\$4,000	
					Total Const	ruction Cost	\$217,000	

Hoyle, Ta	inner	Town of Richmond, VT				Project No.:	102601
125 Colle	ge St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrade				By:	KDW
Burlingto	n, VT 05401	Engineer's Opinion of Probable Project Costs				CK By:	
802-860-	1331	UV System Replacement				Date:	1/11/2024
	1		-				
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost
UV Disin	fection System Re	placement					
	Site/Civil						
		N/A					
L	Structural						
		N/A					
	Process Mechan						
		New UV Disinfection System (lamp modules, ballasts, instrumentation, controls)	1	LS	\$145,000	30%	\$188,500
	Buidling Mechar	lical					
		N/A					
	Electrical/I&C						
		N/A		LS			
		UV System Replacement - Subtotal					\$188,500
					Subto	tal (Rounded)	\$189,000
Contract	or Markups						
		Contractor Overhead & Profit	15%				\$28,000
		Mobilization/Demobilization	5%				\$9,000
		Bonds & Insurance	2%				\$4,000
					Total Cons	truction Cost	\$230,000
Notes: 1	. ENR Constructio	n Cost Index = 13514.76 (December 2023)					

Hoyle, Tanner Town of Richmond, VT						Project No.:	102601
125 Colle	ege St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrade	9			By:	KDW
Burlingto	on, VT 05401	Engineer's Opinion of Probable Project Costs				CK By:	
802-860	1331	Effluent Flow Measurement Improvements	Date:	1/11/2024			
Process	Division/	Description	No. Of	Unit	Unit Cost	Install	Total Cost
Area	Discipline		Units	onne	Onit Cost	mstan	Total Cost
Effluent	Flow Measureme	nt	Units Unit cost Image: Stress of the stres of the stress of the stress of the stress of the st				
	Site/Civil						
		N/A					
	Structural						
		SS 90-Degree Weir Plate	1	LS	\$5,000		\$5,000
	Process Mechan	ical					
		N/A					
	Buidling Mechar	ical					
		N/A					
	Electrical/I&C						
		Radar Flow Meter	1	LS	\$5,000		\$5,000
		Electrical & SCADA Controls	1	LS	\$2,000		\$2,000
		Effluent Flow Measurement - Subtotal					\$12,000
					Subto	tal (Rounded)	\$12,000
Contract	or Markups						
		Contractor Overhead & Profit	15%				\$2,000
		Mobilization/Demobilization	5%				\$1,000
		Bonds & Insurance	2%				\$0
					Total Conc	truction Cost	¢1E 000

125 Colleg		Hoyle, Tanner Town of Richmond, VT					102601
	ge St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrade	1			By:	KDW
Burlingtor	n, VT 05401	Engineer's Opinion of Probable Project Costs				CK By:	
802-860-1	1331	Effluent Pump Station				Date:	1/11/2024
							_
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost
Effluent F	low Measureme	nt					
	Site/Civil						
		18" Backflow Preventer	1	EA	\$7,000		\$7,000
		Effluent Pipe Segments	20	LF	\$100		\$2,000
		Excavation for Structure	142	CY	\$25		\$3,600
		Gravel Subbase (24")	5	CY	\$60		\$400
		Structural Backfill	57	CY	\$25		\$1,500
		Site Dewatering	1	LS	\$2,000		\$2,000
	Structural						
		Wetwell Concrete (9' x 7' x 15' inside dimensions)	26	CY	\$2,000		\$52,700
		Access Hatch	1	EA	\$6,000		\$6,000
		Building Enclosure	90	SF	\$600		\$54,000
	Process Mechan	ical					
		Effluent Pumps - Vertical Turbine	2	EA	\$65,000	30%	\$169,000
		Process Piping and Valves	1	LS	\$25,000		\$25,000
	Buidling Mechar	ical					
		Building HVAC	1	LS	\$20,000		\$20,000
	Electrical/I&C						
		Building Electrical	1	LS	\$20,000		\$20,000
		Pump VFDs and Control Panel	1	LS	\$23,800		\$23,800
		Level Control System	1	LS	\$15,000		\$15,000
		Electrical, Instrumentation & Controls	1	LS	\$20,000		\$20,000
		Effluent Flow Measurement - Subtotal					\$422,000
					Subto	tal (Rounded)	\$422,000
Contracto	or Markups						
		Contractor Overhead & Profit	15%				\$63,000
		Mobilization/Demobilization	5%				\$21,000
		Bonds & Insurance	2%				\$8,000
					Total Cons	truction Cost	\$514,000

Hoyle, Ta	nner	Town of Richmond, VT				Project No.:	102601
125 Colle	ge St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrade				By:	KDW
Burlingto	n, VT 05401	Engineer's Opinion of Probable Project Costs				CK By:	
802-860-3	1331	Septage Receiving Facilities - Replace in Kind & Building Modification	Date:	1/11/2024			
		·					
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost
Septage F	Receiving Facilities	s					
	Site/Civil						
		N/A					
	Structural						
		CMU Block Wall	400	SF	\$50		\$20,000
		6' Double Door	1	LS	\$10,000		\$10,000
		Painting and Specialty Coatings	1	LS	\$10,000		\$10,000
	Process Mechani	cal					
		Septage Acceptance Plant (incl. screening unit, control panel, hauler card reader, flow meter)	1	LS	\$306,700	30%	\$399,000
		Process Piping and Valve Replacements	1	LS	\$50,000		\$50,000
	Buidling Mechan	ical					
		New HVAC System for Garage and Septage Room	1	LS	\$150,000		\$150,000
	Electrical/I&C						
		Building Electrical Allowance	1	LS	\$50,000		\$50,000
		Process Electrical, Instrumentation & Control Allowance	1	LS	\$25,000		\$25,000
		Septage Receiving Facilities - Subtotal					\$714,000
					Subto	tal (Rounded)	\$714,000
Contracto	or Markups						
		Contractor Overhead & Profit	15%				\$107,000
		Mobilization/Demobilization	5%				\$36,000
		Bonds & Insurance	2%				\$14,000
					Total Cons	struction Cost	\$871,000
Notes: 1.	ENR Construction	n Cost Index = 13514.76 (December 2023)					

Hoyle, Ta	inner	Town of Richmond, VT	Town of Richmond, VT					
125 Colle	ge St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrad	e			By:	KDW	
Burlingto	n, VT 05401	Engineer's Opinion of Probable Project Costs				CK By:		
802-860-	1331	Septage and Sludge Holding - Alternative 1	and Sludge Holding - Alternative 1				1/11/2024	
					·			
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost	
Septage	and Sludge Holdi	ng - Alternative 1						
	Site/Civil							
		Air yard piping from Aeration Tank Blowers to Outdoor Sludge Holding	60	LF	\$200		\$12,000	
	Structural							
		Concrete Cover for Outdoor Aerobic Sludge Holding Basin	10	CY	\$2,000		\$20,000	
		Access Hatches	1	EA	\$10,000		\$10,000	
	Process Mechar	nical						
		Coarse Bubble Diffusers for all Tanks	1	EA	\$50,000	30%	\$65,000	
		Septage and Sludge Holding Tank Blower w/VFD	1	EA	\$51,000	30%	\$66,300	
		Aerobic Sludge Holding Tank Blower w/VFD	1	EA	\$51,000	30%	\$66,300	
		Transfer Pumps (2)	2	EA	\$19,000	30%	\$49,400	
		Modifications to Existing Pump Suction and Discharge Piping	1	LS	\$25,000		\$25,000	
		Valve Replacements	1	LS	\$25,000		\$25,000	
	Electrical							
		Air flow meters	2	EA	\$10,000		\$20,000	
		Magnetic flow meters	1	EA	\$10,000		\$10,000	
		Level Detection System for Holding Tanks	3	EA	\$5,000		\$15,000	
		Process Electrical, Instrumentation & Control Allowance	1	LS	\$25,000		\$25,000	
		Septage & Sludge Holding Alternative 1- Subtotal					\$409,000	
Odor Cor	ntrol							
		Vapex Odor Control System	1	EA	\$186,000	30%	\$241,800	
		Vapex piping and valves (sch 80 PVC)	1	EA	\$25,000		\$25,000	
		Odor Control Sub-Total					\$266,800	
						Subtotal	\$675,800	
					Subt	total (Rounded)	\$676,000	
Contract	or Markups							
		Contractor Overhead & Profit	15%				\$101,000	
		Mobilization/Demobilization	5%				\$34,000	
		Bonds & Insurance	2%				\$14,000	
	Î				Total Co	nstruction Cost	\$825,000	
Notes: 1	. ENR Constructio	on Cost Index = 13514.76 (December 2023)						

Hoyle, Ta	nner	Town of Richmond, VT				Project No.:	102601
125 Colle	ege St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgra	ade			By:	KDW
Burlingto	on, VT 05401	Engineer's Opinion of Probable Project Cos	sts			СК Ву:	
802-860-	1331	Septage and Sludge Holding - Alternative	2			Date:	1/11/2024
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost
Septage	and Sludge Holdi	ng - Alternative 2					
	Site/Civil						
		Compressed gas yard piping	210	LF	\$50		\$10,500
	Structural						
		Concrete Cover for Outdoor Aerobic Sludge Holding Basin	10	CY	\$2,000		\$20,000
		Access Hatches	1	EA	\$10,000		\$10,000
	Process Mechan	ical					
		Compressed Gas Mixing System for all Tanks	1	EA	\$166,000	30%	\$215,800
		Transfer Pumps (2)	2	EA	\$19,000	30%	\$49,400
		Modifications to Existing Pump Suction and Discharge Piping	1	LS	\$25,000		\$25,000
		Valve Replacements	1	LS	\$25,000		\$25,000
	Electrical						
		Magnetic flow meters	1	EA	\$10,000		\$10,000
		Level Detection System for Holding Tanks	3	EA	\$5,000		\$15,000
		Process Electrical, Instrumentation & Control Allowance	1	LS	\$25,000		\$25,000
		Septage & Sludge Holding Alternative 2- Subtotal					\$405,700
Odor Cor	ntrol						
		Vapex Odor Control System (for all holding tanks)	1	EA	\$186,000	30%	\$241,800
		Vapex piping and valves (sch 80 PVC)	1	EA	\$25 <i>,</i> 000		\$25,000
		Odor Control Sub-Total					\$266,800
						Subtotal	\$672,500
					Sub	total (Rounded)	\$673,000
Contract	or Markups						
		Contractor Overhead & Profit	15%				\$101,000
		Mobilization/Demobilization	5%				\$34,000
		Bonds & Insurance	2%				\$13,000
					Total Co	nstruction Cost	\$821,000
Notes: 1	. ENR Constructio	n Cost Index = 13514.76 (December 2023)					

Hoyle, Tan	ner	Town of Richmond, VT		Project No.: 102601			
125 Colleg	e St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrade				By:	KDW
Burlington	, VT 05401	Engineer's Opinion of Probable Project Costs		CK By:			
802-860-13	331	Dewatering Facilities				Date:	1/11/2024
Process	Division/	Description	No. Of	Unit	Unit Cost	Install	Total Cost
Area	Discipline	Description	Units	Unit	Unit Cost	instan	Total Cost
Dewaterin	g Facilities						
	Site/Civil						
		Excavation for Structures	256	CY	\$25		\$6,400
		Gravel Subbase (24")	93	CY	\$60		\$5,600
		Finish Grading & Site Restoration	1	LS	\$25,000		\$25,000
	Structural						
		Building Addition (approx. 32' x 41')	1258	SF	\$300		\$377,400
		Concrete Slab	47	CY	\$1,000		\$46,600
		Foundation Walls	29	CY	\$1,200		\$35,100
		Strip Footings	20	CY	\$800		\$15,600
	Process Mecha	nical					
		Sludge Transfer Pumps					-
		Sludge Transfer Pumps - Rotary Lobe, Positive Displacement, w/ VFD	2	EA	\$17,400	30%	\$45,300
		Sludge Feed Pumps					-
		Dewatering Sludge Feed Pump - Rotary Lobe, Positive Displacement, w/ VFD	2	EA	\$17,400	30%	\$45,300
		Dewatering Press					
		4-Channel Rotary Press (reuses 2 newly purchased channels)	1	EA	\$214,800	30%	\$279,300
		Polymer Feed System					
		Polymer Feed System	1	EA	\$24,000	30%	\$31,200
		Chemical Process Piping & Valves	1	ALL	\$20,000		\$20,000
		Conveyor System					
		Shaftless Conveyor System	1	EA	\$89,500	30%	\$116,400
		Sludge Day Tank					
		Mixer	1	EA	\$12,500	30%	\$16,300
		Dewatering Process Piping and Valves Allowance	1	ALL	\$100,000		\$100,000
	Building Mecha	anical					
		New HVAC System	1	LS	\$200,000		\$200,000
		Building Plumbing Allowance	1	ALL	\$75,000		\$75,000
	Electrical						
		Magnetic flow meter	2	EA	\$10,000		\$20,000
		Level Detection System for Day Tank	1	EA	\$5,000		\$5,000
		Building Electrical Allowance	1	ALL	\$150,000		\$150,000
		Process Electrical, Instrumentation & Control Allowance	1	ALL	\$100,000		\$100,000
		Dewatering Facilities - Subtotal					\$1.715.500
							, , ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
Dowatorin	g Pressate Cher	nical Storage and Feed System					
Dewaterin	Sito/Civil			-			
	Structural						
	Structural	Curbed Containment Area for Dewatering Chemical Storage Area (112 SE 9" curb)	2	CV	\$1.200		\$2,400
	Brocoss Mocha		2	CI	\$1,200		\$2,400
	Process Wecha	Nical	4	5.4	¢12.000	20%	¢16.000
			1	EA	\$13,000	30%	\$16,900
			1	EA	\$5,000		\$5,000
		Injection quill for pressate dosing point	1	EA	\$800		\$800
	Electrical/I&C				.		410.000
		Electrical, Instrumentation, & Controls	1	LS	\$10,000		\$10,000
		Dewatering Pressate Chemical Storage and Feed System - Subtotal					\$35,100
						Subtotal	\$1,750,600
					Subto	tal (Rounded)	\$1,751,000
Contractor	r Markups						
		Contractor Overhead & Profit	15%				\$263,000
L		Mobilization/Demobilization	5%				\$88,000
		Bonds & Insurance	2%	L			\$35,000
					Total Cons	truction Cost	\$2,137,000
Notes: 1.	ENR Constructi	on Cost Index = 13514.76 (December 2023)					

Hoyle, Ta	nner	Town of Richmond, VT	Town of Richmond, VT				
125 Colle	ge St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrade				By:	KDW
Burlingto	n, VT 05401	Engineer's Opinion of Probable Project Costs				CK By:	
802-860-	1331	Operations Building Renovations					1/11/2024
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost
Operatio	ns Building Reno	vations					
	Structural						
		Floor Access Door for Influent Pump Removal	1	LS	\$15,000	50%	\$22,500
		Floor Restoration	1	LS	\$10,000		\$10,000
		New Windows and Doors	1	LS	\$50,000	30%	\$65 <i>,</i> 000
		New Paint and Finishes	1	LS	\$20,000		\$20,000
	Process Mechai	nical					
		Laboratory Equipment Allowance	1	LS	\$15,000		\$15,000
	Buidling Mecha	nical					
		New Effluent Heat Recovery System Allowance	1	LS	\$300,000		\$300,000
		New Boiler/HVAC improvements	1	LS	\$100,000		\$100,000
		Plumbing Upgrades	1	LS	\$30,000		\$30,000
				LS			
		Operations Building Renovations - Subtotal					\$562,500
					Subtot	al (Rounded)	\$563,000
Contract	or Markups						
		Contractor Overhead & Profit	15%				\$84,000
		Mobilization/Demobilization	5%				\$28,000
		Bonds & Insurance	2%				\$11,000
					Total Const	ruction Cost	\$686,000
Notes: 1	ENR Construction	on Cost Index =					

			102001			
., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrade				By:	KDW
05401	Engineer's Opinion of Probable Project Costs				CK By:	
	Site Improvements				Date:	1/11/2024
	·					
Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost
ion - Flood D	oors					
Site/Civil						
	Single Door Flood Barrier	1	EA	\$5,000	30%	\$6,500
	Garage Door Flood Barrier	1	EA	\$12,000	30%	\$15,600
	Flood Mitigation - Flood Doors - Subtotal					\$22,100
Site/Civil	Madifications to Chain Link Convity Conco	1	10	ć10.000		¢10.000
	Now Security Cote 24' wide	1		\$10,000		\$10,000
	Process Vard Pining & Valve Allowance	1		\$15,000		\$13,000
	Cold Planing Evisting Pavement	1 275	SV	\$30,000 \$10		\$12,800
	New Pavement (3")	220	TON	\$200		\$12,800
Electrical/I&	C	220	TON	<i>4</i> 200		Ç11,000
2.000.100.17.100	Facilility-Wide Electrical Assessment	1	LS	\$25,000		\$25,000
	New Generator	1	LS	\$40,000	30%	\$52,000
	SCADA Programming Allowance	1	LS	\$50,000		\$50,000
	Electrical Site Work - Conduit & Wire Allowance	1	LS	\$50,000		\$50,000
	Site - Subtotal					\$308,800
ystem						
Site/Civil						
	N/A					
Structural						
	N/A					
Process Med	hanical					
	Grundfos Hydro MPC 3CRE15-4 Pump Skid	1	LS	\$64,000	30%	\$83,200
	Two (2) 7.5 hp pumps - 150 gpm @ 70 psi					
	Control Panel with Integral VFDs			ć20.000		¢20.000
Duidling Ma	Piping, valves, Fittings, and Appurtenance	1	LS	\$20,000		\$20,000
Bulating wie						
Electrical/I&						
Electricaly lo	Plant Water Level Control System	1	15	\$10,000		\$10,000
	Electrical. Instrumentation & Control Allowance	1	LS	\$25.000		\$25.000
	Plant Water System - Subtotal	_		+/		\$138,200
				Subt	total (Rounded)	\$138.000
					(,	
					Subtotal	\$468,900
				Subt	total (Rounded)	\$469,000
arkups						
	Contractor Overhead & Profit	15%				\$70,000
	Mobilization/Demobilization	5%				\$23,000
	Bonds & Insurance	2%				\$9,000
				Total Co	nstruction Cost	\$571,000
	-	•	•	•		
	O5401 Division/ Discipline ion - Flood D Site/Civil Site/Civil Electrical/I& Structural Process Mec Buidling Me Electrical/I& Comparison Buidling Me Buidling Me Comparison Buidling Me Buidling	Object Engineer's Opinion of Probable Project Costs Site Improvements Division/ Discipline Description on - Flood Doors Single Door Flood Barrier Garage Door Flood Barrier Garage Door Flood Barrier Garage Door Flood Barrier Flood Mitigation - Flood Doors - Subtotal Site/Civil Nodifications to Chain-Link Security Fence New Security Gate - 24' wide Process Yard Piping & Valve Allowance Cold Planing Existing Pavement New Pavement (3') Electrical/I&C Facilitity. Wide Electrical Assessment New Generator Site - Subtotal Site/Civil N/A Process Mechanical Site - Subtotal ystem Site - Subtotal Site/Civil N/A Process Mechanical Grundfos Hydro MPC 3CRE15-4 Pump Skid Two (2) 7.5 hp pumps - 150 gpm @ 70 psi Control Panel with Integral VFDs Piping, Valves, Fittings, and Appurtenance Building Mechanical N/A Plant Water Level Control System Electrical, Instrumentation & Control Allowance Plant Water System - Subtotal Plant Water Level Control System Electrical, Ins	Observation Engineer's Opinion of Probable Project Costs Site Improvements No. Of Units Division/ Discipline Description No. Of Units on - Flood Doors	Object Engineer's Opinion of Probable Project Costs Site Improvements Division/ Disciplin Description No. of Units Unit Site Improvements Improvements Improvements Improvements Site/Civil Improvements Improvements Improvements New Security Gate - 24' wide Improvements Improvements Improvements Site/Civil Improvements Improvements Improvements Improvements New Recurity Gate - 24' wide Improvements Improvements Improvements Improvements Recurity Gate - 24' wide Improvements Improvements Improvements Improvements Recurity Recurity Gate - 24' wide	Observation Engineer's Opinion of Probable Project Costs Site Improvements No. of Units Unit Unit Unit Cost Division/ Discipline Description No. of Units Unit Unit Cost on - Flood Doors 1 EA \$5,000 Garage Door Flood Barrier 1 EA \$5,000 Garage Door Flood Barrier 1 EA \$51,000 Garage Door Flood Barrier 1 EA \$12,000 Modifications to Chain-Link Security Fence 1 LS \$11,000 New Security Gate - 24 Wale Aulevance 1 LS \$15,000 Cold Planing Existing Pavement 1,275 SY \$10 New Generator 1 LS \$250,000 Stectrical/I&C 1 LS \$550,000 Electrical Site Work - Conduit & Wire Allowance 1 LS \$550,000 Stectrical Site Work - Conduit & Wire Allowance 1 LS \$550,000 Stectrical Site Work - Conduit & Wire Allowance 1 LS \$550,000 Stectrical Site Work - Conduit & Wire Allowance 1 LS \$550,000	Obtain Engineer's Opinion of Probable Project Costs CK By; Site Improvements Date Division/ Discipline unit Cost Install Division/ Discipline No. Of Units Unit Unit Cost Install Single Door Flood Barrier 1 EA 55,000 30% Garage Door Flood Barrier 1 EA 551,000 30% Garage Door Flood Barrier 1 EA 551,000 30% Single Door Flood Barrier 1 EA 551,000 30% Garage Door Flood Barrier 1 LS \$10,000 30% Single/Civil Faci High Status 1 EA \$51,000 Modifications to Chain-Link Security Fence 1 LS \$51,000 500 Process Yard Piping & Valve Allowance 1 LS \$550,000 30% Cald Planing Existing Pavement 1,275 SY \$20 500 New Security Gate - 24' wide 1 LS \$50,000 30% Cald Planing Existing Pavement 1,25 \$550,000 30% Kext Barde Bedrical Assessment 1 LS \$50,000 30% Sice/Civil Site Subtotal I IS \$50,000

Hoyle, Tanne	r	Town of Richmond, VT				Project No.:	102601
125 College S	t., 4th Floor	Richmond WWTF - 20 Year Evaluation	Upgrade			By:	KDW
Burlington, V	T 05401	Engineer's Opinion of Probable Proje	ct Costs			CK By:	
802-860-1331	1	Collection System - Manhole Rehabilitation a	nd Mainte	nance		Date:	1/11/2024
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost
Manhole Reh	abilitation an	d Maintenance					
	Site/Civil						
		Manhole Cleaning	15	EA	\$2,500		\$37,500
		Manhole Ring and Frame Replacement	4	EA	\$2,000	\$2,000 30%	
		Manhole Refurbishment - brick repair in riser	2	EA	\$2,000	30%	\$5,200
		Manhole Rehabilitation and Maintenance - Subtotal					\$53,100
						Subtotal	\$53,100
					Subto	tal (Rounded)	\$53,000
Contractor M	larkups						
		Contractor Overhead & Profit	15%				\$8,000
		Mobilization/Demobilization	5%				\$3,000
		Bonds & Insurance	2%				\$1,000
					Total Cons	truction Cost	\$65,000
Notes: 1.	ENR Construc	tion Cost Index = 13514.76 (December 2023)					

Hoyle, Tann	er	Town of Richmond, VT		Project No.:	102601		
125 College	St., 4th Floor	Richmond WWTF - 20 Year Evaluation Upgrade				By:	KDW
Burlington	VT 05401	Engineer's Opinion of Probable Project Costs				, CK By:	
802-860-133	31	Bridge Street Pump Statin Replacment and Bridge Forcemain I	Replaceme	nt		Date:	1/11/2024
	-			-			_//
Process Area	Division/ Discipline	Description	No. Of Units	Unit	Unit Cost	Install	Total Cost
Bridge Stree	et Pump Statio	n					
	Site						
		Remove ex. wetwell and drywell	1	LS	\$20,000		\$20,000
		Dewatering	40	Day	\$300		\$12,000
		Site Restoration	1	LS	\$25,000		\$25,000
		Soil Erosion and Sediment Control	1	LS	\$10,000		\$10,000
		Bypass Pumping	6	WK	\$5,000		\$30,000
		Traffic Control	1	LS	\$5,000		\$5,000
		Flaggers (2 flaggers/day x 10 hrs/day x 40 days)	800	HR	\$25		\$20,000
		Site Subtotal					\$122,000
	Valve Vault						
		New 7'x10' precast concrete Valve Vault with extended top	1	LS	\$30,000		\$30,000
		Shoring	1	EA	\$30,000		\$30,000
		Valve Vault - installation (includes excavation, bedding, crane)	1	EA	\$30,000		\$30,000
		Valve Vault interior material	1	EA	\$12,000	50%	\$18,000
		Sump Alarm Float	1	EA	\$1,000		\$1,000
		Flow meter	1	LS	\$10,000		\$10,000
		Miscellaneous Concrete	5	CY	\$1,000		\$5,000
		Valve Vault Subtotal					\$124,000
	Wetwell & Pu	imping Equipment					
		Submersible Pumps, controls, slide rails, Mission RTU - material, startup	1	LS	\$125,000	50%	\$187,500
		Pump station piping - material	1	LS	\$50,000	50%	\$75,000
		Pump Station precast wet well 5' dia. x 15' deep - material, includes hatch	1	EA	\$50,000		\$50,000
		Shoring	1	EA	\$30,000		\$30,000
		Wet well - installation (includes excavation, bedding, crane)	1	EA	\$35,000		\$35,000
		Stainless Steel Vent (6" Sch 40)	15	LF	\$100		\$1,500
		Wetwell & Pumping Equipment Subtotal					\$379,000
	Electrical						
		Electrical Service with main disconnect switch	1	LS	\$35,000		\$35,000
		Generator & ATS	1	LS	\$50,000		\$50,000
		Alarm light and audible alarm with battery back-up	1	LS	\$5,000		\$5,000
		Mission Communications RTU	1	LS	\$10,000		\$10,000
		Pump Level Controls	1	LS	\$15,000		\$15,000
		Main Power Cabinet	1	LS	\$35,000		\$35,000
		Electrical to wetwell and valve vault	1	LS	\$10,000		\$10,000
		Electrical Subtotal					\$160,000
Bridge Cross	ssing Force Ma	in					
		Bridge Crossing Force Main - 4" HDPE	200	LF	\$150		\$30,000
		Carrier Pipe - 12" HDPE	200	LF	\$250		\$50,000
		Insulation	200	LF	\$50		\$10,000
		Bridge Crossing Subtotal					\$90,000
Ĺ.							
						Subtotal	\$875,000
Ĺ.					Subto	tal (Rounded)	\$875,000
Contractor I	Markups						
		Contractor Overhead & Profit	15%				\$131,000
		Mobilization/Demobilization	5%				\$44,000
		Bonds & Insurance	2%				\$18,000
					Total Cons	truction Cost	\$1,068,000
Notes:							
1.	ENR Construc	tion Cost Index = 13514.76 (December 2023)					

APPENDIX G LIFE CYCLE COSTS

Hoyle, Tanner & Associates, Inc.					Town of Richmond	, VT	Project No.:	102601
125 College St., 4th Floor				Richmond W	WTF - 20 Year Eva	luation Upgrade	By:	ACD
Burlington, VT 05401				Engineer's Opi	nion of Probable C	onstruction Costs	CK By:	KDW
802-860-1331				Life Cycle Cost	Estimate - Headw	orks Alternative 1	Date:	1/17/2024
Operation & Maintenance Cost Estimates								
Headworks Alternative 1 - Replace-in-Kind								
Electric	HP	kW	hrs/days	kwh/day	Annual KwH	Green Mountain Power Group Rate 65 PF	_	
Micro Strainer	2	1.4914	6	9	3,266	Customer Service Charge \$ 16.700 per day		
Horizontal Grit Screw	1	0.7457	4	3	1,089	\$ 0.14935 per kwh (this is bot	h peak and n	on-peak net rate)
Grit Dewatering Screw	2	1.4914	4	6	2,177			
Blower Package	2	1.4914	24	36	13,065			
				Total	19,597			
			Annı	ual Electrical Cost	\$ 9,022.21			
				Say	\$ 9,100.00			
	Froquency							
Ponlocomont Dorts	(vrs)	Unit Cost	Annual total					
Replacement Parts	(913)	c 151 500						
	20 20 April 20	3 151,500	\$ 7,575.00					
All								
		Jay	\$ 7,000.00					
Operation		Hours/week	Annual Hours					
Headworks - Operation & Maintenance 7 364			364					
Total 7 364								
	An	nual Labor cost	\$ 18,200.00					
		Tota	al Annual O&M	\$ 34,900.00				

Hoyle, Tanner & Associates, Inc.	Town of Richmon					ond, VT	Project No.: 102601
125 College St., 4th Floor				Richmond	l WWTF - 20 Year	Evaluation Upgrade	By: ACD
Burlington, VT 05401		le Construction Costs	CK By: KDW				
802-860-1331				Life Cycle Co	ost Estimate - Hea	adworks Alternative 2	Date: 1/17/2024
Operation & Maintenance Cost Estimates							
Headworks Alternative 2 - Multi-Rake Screen & Grit Remov	al Equipmer	nt					
Electric	НР	kW	hrs/days	kwh/day	Annual KwH	Green Mountain Power Group Rate 65 PF	
Multi-Rake Screen	3	2.2	6	13	4,899	Customer Service Charge \$ 16.70 per day	
Wash Press	5	3.7	6	22	8,165	\$ 0.15 per kwh (this is both	peak and non-peak net rate)
Horizontal Grit Screw	1	0.7	4	3	1,089		
Grit Dewatering Screw	2	1.5	4	6	2,177		
Blower Package	2	1.5	24	36	13,065		
				Total	29,395		
			Annual E	Electrical Cost	\$ 10,485.57		
				Say	\$ 10,500.00		
					-		
	Frequency						
Replacement Parts	(yrs)	Unit Cost	Annual total				
Screening Equipment Costs (1/3 of total cost)	20	\$ 70,500	\$ 3,525.00				
Grit Removal Equipment Costs (1/3 of total cost)	20	\$ 112,500	\$ 5,625.00				
An	nual Replacen	nent Parts Cost	\$ 3,525.00				
		Say	\$ 3,600.00				
Operation		Hours/week	Annual Hours				
Headworks - Operation & Maintenance		7	364				
	Total	7	364				
		Labor rate	\$ 50.00				
	Anr	ual Labor cost	\$ 18,200.00				
					_		
		Tota	al Annual O&M	\$32,300.00			
					-		

Hoyle, Tanner & Associates, Inc.	Town of Richmond	Project No.:	102601	
125 College St., 4th Floor	Richmond WWTF 20 Year Evaluation Upgrade	By:	ACD	
Burlington, VT 05401	Engineer's Opinion of Probable Construction Cost	СК Ву:	KDW	
802-860-1331	Headworks - Present Worth	Date:	1/17/2024	
		Alternative 1	Alternative 2	
	Description of Item	Replace-in-Kind	Multi-Rake Screen	
		& Grit Removal		
Alternative Project Cost				
Total Project Cost of A	Nternative ¹	\$1,033,000	\$1,654,000	
Alternative Annual O&M Cost		\$ 34,900	\$ 32,300	
Annual O&M Cost of A	Alternative (rounded)	\$35,000	\$32,000	
Present Worth of Alternatives				
Escalation rate, e (assu	umed)	3.0%	3.0%	
Discount rate, i (as per	EPA Fiscal Year 2023)	2.5%	2.5%	
Planning period, n (yea	ars)	20	20	
	Present Worth of Alternatives	\$1,784,000	\$2,341,000	
Notes:				
1 Total project costs are	inclusive of construction costs and contractor mark-up,	but do not include cont	ingency, engineering	
services, legal and adm	ninistrative costs.			

Hoyle, Tanner & Associates, Inc.					Town	of Richmond, VT		Project No.: 102601
125 College St., 4th Floor				Richm	ond WWTF -	20 Year Evaluation	on Upgrade	By: ACD
Burlington, VT 05401				Enginee	er's Opinion o	ruction Costs	CK By: KDW	
802-860-1331				Life Cycle (Cost Estimate	e - Anoxic Selecto	ors Alternative 1	Date: 1/17/2024
Operation & Maintenance Cost Estir	nates							
Anoxic Selectors Alternative 1 - Submersib	le Mixers							
Electric	Quantity	HP	kW	hrs/days	kwh/day	Annual KwH	Green Mountain Power Group Rate 65 PF	
Submersible Mixers	3	1.2	0.89484	24	21	7,839	Customer Service Charge \$ 16.700 per day	
		-			Total	7,839	\$ 0.149 per kwh (this is both	peak and non-peak net rate)
				Annual Ele	ectrical Cost	\$ 7,266.19		
					Say	\$ 7,300.00		
	Frequency							
Replacement Parts	(yrs)	Unit Cost	Annual total					
Mixing Equipment Costs (1/3 of total cost)	20	\$ 7,246	\$ 362.30					
Ann	ual Replacen	ent Parts Cost	\$ 362.30					
		Say	\$ 400.00					
		-	1	1				
Operation		Hours/week	Annual Hours					
Submersible Mixers - Operation & Maintenance	e	1	52					
	Total	1	52					
	_	Labor rate	\$ 50.00					
	Anr	ual Labor cost	\$ 2,600.00					
		-		¢ 40.000.00	-			
		Тс	otal Annual O&M	\$ 10,300.00				

Hoyle, Tanner & Associates, Inc.					Tow	n of Richmond, V	VT	Project No.: 102601
125 College St., 4th Floor			By: ACD					
Burlington, VT 05401			CK By: KDW					
802-860-1331				Life Cycle	e Cost Estima	ate - Anoxic Sele	ctors Alternative 2	Date: 1/17/2024
Operation & Maintenance Cost Estim	ates							
Anoxic Selectors Alternative 2 - Compressed	l Gas Mixing							
Electric	Quantity	HP	kW	hrs/days	kwh/day	Annual KwH	Notes	
Compressed Gas Mixing	3	0.80	0.60	24	14	5,219	Enviromix estimate a BHP of 4.7 HP. Estimating that	
					Total	5,219	17% of that is for anoxic selectors, other 83% is for	
				Annual Ele	ectrical Cost	\$ 6,874.98	sludge storage.	
					Say	\$ 6,900.00		
				_			Green Mountain Power Group Rate 65 PF	
Replacement Parts	Frequency (yrs)	Unit Cost	Annual total				Customer Service Charge \$ 16.700 per day	
Mixing Equipment Costs (1/3 of total cost)	20	\$ 10,200	\$ 510.00				\$ 0.149 per kwh (thi	is is both peak and non-peak net rate)
Anni	ual Replaceme	ent Parts Cost	\$ 510.00					
		Say	\$ 600.00]				
Operation		Hours/year	Annual Hours	Notes				
Compressed Gas Mixing - Operation & Maintena	nce	1	1	Enviromix e	estimates 4h	nr/yr, split b/w		
Total 1 1				anoxic & septage mixing				
Labor rate \$ 50.00								
	Anni	ual Labor cost	\$ 50.00					
		Tota	al Annual O&M					

Hoyle, Tanne	er & Associates, Inc.	Project No.:	107874.01	
125 College S	st., 4th Floor	Richmond WWTF 20 Year Evaluation Upgrade	By:	ACD
Burlington, V	/T 05401	Engineer's Opinion of Probable Construction Cost	СК Ву:	KDW
802-860-133	1	Anoxic Selectors - Present Worth	Date:	1/17/2024
			Alternative 1	Alternative 2
		Description of Item	Submersible	Compressed Gas
l			Mixers	Mixing
Alternative P	Project Cost			
۲	Total Project Cost of Al	ternative ¹	\$50,000	\$72,000
Alternative A	Annual O&M Cost		\$ 10,300	\$ 7,550
/	Annual O&M Cost of Al	ternative (rounded)	\$10,000	\$8,000
Present Wor	rth of Alternatives			
	Escalation rate, e (assur	ned)	3.0%	3.0%
l!	Discount rate, i (as per f	EPA Fiscal Year 2023)	2.5%	2.5%
F	Planning period, n (year	rs)	20	20
		Present Worth of Alternatives	\$265,000	\$244,000
Notes:				
1 7	Total project costs are in	nclusive of construction costs and contractor mark-up, but do n	ot include contingenc	y, engineering
5	services, legal and admi	nistrative costs.		

Hoyle, Tanner & Associates, Inc.					Т	own of Richmond	, VT	Project No.: 102601
125 College St., 4th Floor			By: ACD					
Burlington, VT 05401			CK By: KDW					
802-860-1331				Life C	ycle Cost Est	imate - Septage N	Mixing Alternative 1	Date: 1/17/2024
Operation & Maintenance Cost Estimates								
Septage Mixing Alternative 1 - Blowers & Diffuser	rs							
Electric	Quantity	bHP	kW	hrs/days	kwh/day	Annual KwH	Green Mountain Power Group Rate 65 PF	
Aerated Holding Basin Blower	1	7.1	5.29	24	127	46,380	Customer Service Charge \$ 16.700 per day	
Aerobic Digester Blower	1	8.6	6.41	24	154	56,178	\$ 0.149 per kwh (this is bot	n peak and non-peak net rate)
Aerobic Digester Blower	1	8.6	6.41	25	160	58,519		
					Tota	102,558		
				Annual Ele	ectrical Cost	\$ 21,411.97		
					Say	\$ 21,500.00		
	-	1						
Replacement Parts	Frequency (yrs)	Unit Cost	Annual total					
Blower & Diffuser Equipment Costs (1/3 of total cost)	20	\$ 22,246	\$ 1,112.30					
Annu	ual Replacem	ent Parts Cost	\$ 1,112.30					
		Say	\$ 1,200.00					
Operation		Hours/week	Annual Hours					
Blower & Diffuser - Operation & Maintenance		7	364					
Total 7 364								
Labor rate \$ 50.00								
	Ann	ual Labor cost						
		Tot	al Annual O&M	\$ 40,900.00				

Hoyle, Tanner & Associates, Inc.					To	wn of Richmond	, VT	Project No.:	102601	
125 College St., 4th Floor				Ric	hmond WW	TF - 20 Year Eva	uation Upgrade	By:	ACD	
Burlington, VT 05401				onstruction Costs	CK By:	KDW				
802-860-1331				Life Cyc	le Cost Estin	mate - Septage I	Mixing Alternative 2	Date:	1/17/2024	
Operation & Maintenance Cost Estimates										
Septage Mixing Alternative 2 - Compressed Gas I	Vixing									
Electric	Quantity	bHP	kW	hrs/days	kwh/day	Annual KwH	Notes			
Compressed Gas Mixing	3	3.90	2.91	24	70	25,483	Enviromix estimate a BHP of 4.7 HP. Estimating that			
					Total	25,483	17% of that is for anoxic selectors, other 83% is for			
				Annual Ele	ctrical Cost	\$ 9,901.20	sludge storage.			
					Say	\$10,000.00				
	ī									
Replacement Parts	Frequency (yrs)	Unit Cost	Annual total							
Mixing Equipment Costs (from Enviromix)	20	\$ 40,000	\$ 2,000.00							
Ann	ual Replacen	nent Parts Cost	\$ 2,000.00				Green Mountain Power Group Rate 65 PF			
		Say	\$ 2,000.00				Customer Service Charge \$ 16.700 per day			
							\$ 0.149 per kwh (this is both	peak and non-	peak net rate)	
Operation		Hours/year	Annual Hours							
Compressed Gas Mixing - Operation & Maintenance		4	4							
	Total	4	4	_						
		Labor rate	\$ 50.00							
	ual Labor cost									
	Tota									

Hoyle, Tanner 8	& Associat	es, Inc.	Project No.:	102601	
125 College St.	, 4th Floor		By:	ACD	
Burlington, VT	05401		Engineer's Opinion of Probable Construction Cost	СК Ву:	KDW
802-860-1331			Septage Mixing - Present Worth	Date:	1/17/2024
				Alternative 1	Alternative 2
			Description of Item	Blowers &	Compressed
		Diffusers	Gas Mixing		
Alternative Pro	oject Cost				
То	otal Projec	t Cost of Alte	ernative ¹	\$825,000	\$821,000
Alternative An	inual O&N	\$ 40,900	\$ 12,200		
An	nnual O&N	\$41,000	\$12,000		
Present Worth	of Altern	atives			
Ese	calation ra	ite, e (assum	ed)	3.0%	3.0%
Dis	scount rat	e, i (as per El	PA Fiscal Year 2023)	2.5%	2.5%
Pla	anning per	iod, n (years		20	20
			Present Worth of Alternatives	\$1,705,000	\$1,079,000
Notes:					
1 To	otal project	t costs are in	clusive of construction costs and contractor mark-up, but do not in	clude contingency, e	engineering
ser	rvices, leg	al and admin	istrative costs.		

APPENDIX H PROPOSED SITE PLAN





APPENDIX I PROPOSED HYDRAULIC PROFILE

