March 20, 2017

# **Preliminary Engineering Report**

West Main Street Water and Sewer Extensions for Town of Richmond, Vermont GME Project # 24-029



Prepared for:

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# PRELIMINARY ENGINEERING REPORT FOR WEST MAIN STREET WATER AND SEWER EXTENSIONS FOR TOWN OF RICHMOND, VERMONT

# March 20, 2017

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#### 1.0 PROJECT PLANNING

#### 1.1 Location

As shown on the Location Map (Figure No. 1 in Appendix A), the Town of Richmond is located in eastern Chittenden County, Vermont and is bordered to the north by Jericho, east by Bolton, South by Hinesburg and Huntington and to the west by Williston. The Town of Richmond provides municipal water and sewer service to the downtown portion of Richmond.

The Town wishes to expand its water and sewer service area to the Route 2 area from the Reap property west along Route 2 and then North along Route 117 to the Riverview Commons Mobile Home Park. The existing and proposed study areas are defined on an aerial map on Figure 2 (Aerial Study Area Map) in Appendix A. The existing and proposed study areas are defined on a topographical map on Figure 3 (Topographical Study Area Map) in Appendix A.

The water and sewer expansion project area is studied in three (3) phases including:

- 1. Phase I: Connection to existing water and sewer near the Camels Hump Middle School to Route 2 through the land trust and Reap development properties.
- Phase II: Route 2 from the Reap Development heading west to approximately 1151 West Main St.
- 3. Phase III: Route 2 @ approximately 1151 West Main St. (west end of Phase II) and Route 117 to the Riverview Commons Mobile Home Park.

# 1.2 Environmental Resources Present

#### 1.2.1 Water Bodies

As shown on the Hydrology Map (Figure 4 in Appendix A), there are four (4) unnamed streams that cross the proposed route. Stream alteration permits would be required for these crossings. These crossings could be done by directional drilling to minimize any impacts on these streams. The last stream is very deep and there is enough material over the culvert to go over the stream.



#### 1.2.2 Wetlands

As shown on the Wetlands Map (Figure No. 5 in Appendix A), there are class II and Class III wetlands located along the proposed route. These wetlands have been delineated by Gilman & Briggs as part of this project. The project will stay out of the wetlands by performing the work in the shoulder of the road. The project will be within 50 feet of the wetlands, so a State of Vermont, wetlands permit is needed. Installing erosion control measures and installing he pipelines by directional drilling will minimize impacts on these wetlands.

#### 1.2.3 Soils

A Soil Survey Map is provided as Figure No. 6 in Appendix A. A Custom Soils Report from the USDA Web Soil Survey for the proposed service area extension is provided in Appendix B. The soils report evaluated the area soils for suitability and limitations with regard to Vermont soilbased residential on-site wastewater disposal.

The ratings are represented by symbols for five interpretive groups and their subgroups. These groups and subgroups are described in the following paragraphs.

Group I soils are well suited to soil-based wastewater disposal systems. Good performance and low maintenance can be expected. The soils in this group are sandy and gravelly soils that have rapid permeability and well drained soils. These are suitable for conventional systems.

Group II soils are moderately suited to soil-based wastewater disposal systems. This group includes soils with moderately slow to very slow permeability; complexes in which one or more of the soils have bedrock at a moderate depth (20 to 40 inches); soils that would qualify for inclusion in group I but have slopes of more than 20 percent; and soils that have a seasonal high water table at a depth of 18 inches or more. These area encompassing these soils typically require a mound system.

Group III map units are marginally suited to soil-based wastewater disposal systems. Intensive onsite investigation may be needed to locate suitable areas, or special design, extra maintenance, or costly alteration may be needed to overcome the soil related limitations. In areas where the



water table is at a shallow depth, seasonal onsite monitoring of the water table may be needed to determine whether the site is suitable. These areas typically require a mound system along with a pre-treatment system, a hydrogeological study, mounding analysis, enhanced prescriptive or performance based system design. Some areas of any of the map units in group III may not be suitable for soil based wastewater disposal systems.

Group IV map units are generally not suited to soil-based wastewater disposal systems because of such limitations as wetness, depth to bedrock, restricted permeability, and slope.

Group V map units are not rated for soil-based wastewater disposal systems. This group includes miscellaneous areas that have been filled, excavated, regraded, or otherwise disturbed by human activities; areas that are mapped above the series level; and areas of water. The miscellaneous areas and the areas mapped above the series level have a wide range of soil properties. Onsite investigation is needed to determine the suitability of these areas for soil-based wastewater disposal.

Table 1 provides a summary of the percentage of soils within the study area by group.

Group No.	Septic Suitability Rating	Percent of Area
I	Well Suited	<1%
Π	Moderately Suited	16%
III	Marginally Suited	17%
IV	Not Suited	48%
V	Not Rated	18%

# Table 1Soil Septic Suitability Rating

An analysis of the soils, as shown in Table 1, reveals that soil conditions in the study area related to the effectiveness of on-site septic systems in the study area are primarily (48%) classified as no-suited. Less than 1% of the soil area is classified as well suited for on-site septic systems. Approximately 33% of the area is classified as moderately or marginally suited. The limitations of these soils generally cannot be overcome without major soil reclamation, special design, or expensive installation procedures. Poor performance and high maintenance can be expected.

#### 1.2.4 Important Farmland

As shown in Figure No. 7 in Appendix A, the project is located within prime and statewide agricultural soils. Portions of the proposed alignment would be located in soils designated as prime farmland or farmland of statewide importance. All but the proposed cross country portion would be located within existing roadway or ROWs that have been previously disturbed and converted. When construction is complete, all land will be returned to existing land use and grade. All pipes to be placed within prime farmland will be buried at sufficient depth to ensure that no land is irreversibly converted to nonagricultural use.

#### 1.2.5 Floodplain

As shown in Figure No. 8 in Appendix A, portions of the project are located within the 100 year floodplain. These portions only include buried piping with the land returned to existing conditions. Most of the project will be performed by directional drilling which does not disturb floodplain, therefore, floodplains will not be an issue.

#### 1.2.6 Historic Preservation

Refer to the Archeological Report in Appendix C. There are several houses on the historic record in the study area. The project area is located away from these structures as the project does not include services on private properties. There are some areas of archeological sensitivity that are recommended for a Phase IB survey.

#### 1.2.7 Rare, Threatened & Endangered Species

As shown in Figure No. 8 in Appendix A, there are no areas rare, threatened & endangered species within the study area. There are two areas of significant natural community (one animal and one natural community) within the trailer park; but these areas are not within any areas of proposed construction disturbance.



#### 1.3 Population Trends

The population of the Town is 4,081 (2010 Census) which includes both the village area served by municipal water and sewer and the rest of the Town. The population has been relatively constant over the last fifteen years. There were some significant increases in population from 1970 to 2000. US Census Data summarized since 1970 below:

1970:	2,249
1980:	3,159
1990:	3,729
2000:	4,090
2010:	4,081

The population is fairly constant year round as Richmond's population does not have significant seasonal influences.

Employment in the Town used to be centered on the dairy industry with a cheese plant as the main employer. The cheese plant has closed down and the Town would like to provide abilities for employment opportunities. These opportunities include extending municipal water and sewer to zoned growth areas of the Town that need the services to grow due to limited on-site water and sewer conditions.

# 1.4 Community Engagement

The Town of Richmond has actively engaged the community and elected officials in the proposed West Main Street sewer expansion project. These engagement activities included:

- A survey questionnaire sent to all property owners within the proposed expanded service area.
- Negotiations with the Riverview Commons Mobile Home Park.
- Completion of a Phase I Scoping Study of the proposed project.
- Two (2) public hearings on the results of the Scoping Study (11/17/14 and 12/1/14).
- Discussions of the project at regularly scheduled Water & Sewer Commission meetings in 2015, 2015 and 2016 which are open to the public.
- Rezoning of the Gateway zoning district and associated public meetings.
- Bond vote informational meeting on March 2, 2015



- Positive bond vote on March 4, 2015 in the amount of \$2.5 million.
- Income survey of the proposed expanded service area including the Route 2 area and residents within the Riverview Commons Mobile Home Park.

The Town of Richmond sent out a survey/questionnaire to all property owners within the study area. Eight (8) surveys were returned. All eight surveys returned were in favor of the water and wastewater utility extension. In addition to the 8 survey's the RCMHP is also interested and has been added to the study as Phase III. See Appendix C for copies of the surveys.





#### 2.0 EXISTING FACILITIES

#### 2.1 Location Map

Refer to Figure No. 1 in Appendix A for a location Map. This map shows the existing water and sewer service area as well as the proposed water and sewer service expansion area.

#### 2.2 History

Areas within the former boundaries of the Incorporated Village of Richmond are currently served by both municipal water and sewer.

The village is served by a municipal water system. It is a treated, gravel-packed well and tanks with a 250,000-gallon storage capacity which serves approximately 300 structures comprising 720 individual units. Waterhouse upgrades were completed in 1999 adding an aeration system to reduce lead and copper levels for improved water quality. Approximately 70,000 gallons are consumed daily, equaling less than 30% of the total capacity.

The village is also served by a municipal wastewater treatment facility (see Figure 8.3), located on Esplanade. The wastewater collection system was expanded in 1999 along Cochran road to cover the remainder of the homes in the service area. The plant was upgraded in 2005, when a \$3.9 million project to reduce phosphorous discharged to 0.8 mg/l was completed. The system lost its largest customer in 1999 with the closing of the Saputo Cheese Plant on Jolina Court. The plant provided 67% of the system revenue. Since that time, no significant new customer has connected to fill that void. Approximately 79,000 gallons are treated per day, equaling 35% of the plant's capacity. The uncommitted reserve capacity as of February, 2006 was 138,269 gallons per day. Due to this reserve capacity, operations now include aggressive septage receiving from septic tank pumping companies. Septage receiving does not preclude potential customers from buying additional uncommitted capacity, but does generate revenue for wastewater operations Water and sewer system capital improvement



#### 2.3 Condition of Existing Facilities

#### 2.3.1 Water

Presently, the West Main Street portion of the study area does not have a public water supply or distribution system. Water is provided to the residents through private wells that are maintained and financed by the individual property owners. There is an increasing concern for quality of the groundwater due to the failed or poorly functioning septic systems. There is also no fire protection for this portion of Town which is zoned as a growth center.

The Riverview Commons Mobile Home Park has a permitted public water system consisting of a bedrock well and disinfection treatment system. The system does not have fire protection. This system is 26 years old and in need of improvements.

#### 2.3.2 Sewer

Presently, wastewater generated within the west Main Street portion of the study area is treated in individual on-site septic systems. Due to lot sizes, individual water wells, poor soil conditions, and depth to groundwater, it is now difficult to provide sufficient wastewater treatment with on-site systems. The proper land area required for on-site disposal is simply not available for most homes within the study area limits. Most of the areas have a high ground water table, which is a limiting characteristic of the dominant soil type in the study area. In these areas, it would be necessary to construct mound type systems to comply with applicable health codes, which is not a feasible option due to financial and lot limitations. Concerns for future development in this area where both septic systems and water wells are utilized on each site are very high due to the potential for groundwater/drinking water contamination. On-site septic suitability is a major constraint to the development of this area as a growth center.

The Riverview Commons Mobile Home Park has a permitted Indirect Discharge wastewater treatment and disposal system. The system consists of gravity sewers, a large septic tank, dosing pump station and a large subsurface disposal system. The system is approximately 26 years old and in need of improvements. Some of the leachfields have clogged and failed in the past, requiring the fields to be replaced.

#### 2.4 Financial Status of Existing Facilities

#### 2.4.1 Income

Table 2 provides a summary of the Town's existing rate structure for water and sewer.

System	User	User
	Туре	Rate
Water	Commercial	\$381/Unit
		\$9.77/1,000 gal.
	Residential	\$130.64/Unit
		\$10.43/1,000 gal.
Sewer	Commercial	\$519.98/Unit
		\$13.00/1,000 gal.
	Residential	\$174.55/Unit
		\$14.13/1,000 gal.

Table 2Existing User Rate Structure

Based on the existing service area user types and flow usage, the Town currently receives approximately \$277,072 annually in water revenues and \$363,603 in sewer revenues per year for user fees. The Town also receives approximately \$1,500 annually in the sale of water from hydrants and \$500 per year in water tap fees. The Town also receives approximately \$181,576 annually in septage receiving fees. The Town's average daily water consumption for single family users is 190 gpd. The average yearly water fee for a typical single family home using 190 gpd is \$854 and the average sewer user fee is \$1,154. These user rates are high compared to averages in the State. If the Town were to be able to add more user base, these rates could come down.

# 2.4.2 Debt Repayments

Table 3 provides a summary of the existing debt repayments for the water system.

# Table 3Existing Water System Debt Repayments

Debt	Annual Payment	Interest Rate	Year Due
Brown's Court Waterline	\$15,000	2.32%	2018
New Water Tank Engineering	\$46,500	0%	2019
Planning Loan	\$12,081	0%	2024
Jericho Road Waterline	\$74,888	1.6%	2031
East Main Street Waterline Upgrades	\$48,000	0%	2036
New Water Tank Construction	\$47,000	0%	2046
Total	\$243,469		

Table 4 provides a summary of the existing debt repayments for the sewer system.

Debt	Annual Payment	Interest Rate	Year Due		
WWTF Phosphorous Upgrade	\$22,220	2%	2026		
Jericho Road Sewerline	\$23,400	1.6%	2031		
Collection System Rehab	\$13,951	2%	2032		
Total	\$57,571				

# Table 4Existing Sewer System Debt Repayments

# 2.4.3 Existing O&M Costs

Table 5 provides a summary of the existing O&M costs for the water system.

Existing water System O&M Costs				
Debt	Existing O&M			
	Cost			
Administration	\$12,300			
Engineering	\$2,000			
Capital Reserve	\$48,000			
Repairs/Maintenance	\$45,000			
Salaries/Benefits	\$60,000			
Supplies	\$10,200			
Utilities	\$11,300			
Total	\$188,800			

Table 5Existing Water System O&M Costs

Table 6 provides a summary of the existing sewer system O&M costs.

# Table 6Existing Sewer System O&M Costs

Debt	Existing O&M Cost
Administration	\$18,143
Engineering	\$500
Biosolids Disposal	\$65,000
Insurance	\$11,200
Repairs/Maintenance	\$32,000
Salaries/Benefits	\$138,126
Supplies	\$46,500
Utilities	\$79,100
Total	\$390,569

#### 3.0 NEED FOR PROJECT

#### 3.1 Health, Sanitation, and Security

The study area will benefit significantly from municipal water and wastewater infrastructure construction. On-site septic systems and the Riverview Commons Mobile Home Park septic system have failed in the past which has resulted in some surface and groundwater contamination. Further, private wells currently supply all of the residents' potable water. This could lead to further endangerment of the resident's water supply due to faulty or malfunctioning septic systems.

#### 3.2 Aging Infrastructure

The on-site septic systems have generally matured to the point that replacement on-site treatment will either become too costly or not possible to meet current rules. However, with the provision of public sewers, user fees cover the cost of operation for the public portion of the system. This assures the system is always in good working condition. The Riverview Commons Mobile Home Park water and septic systems are approximately 26 years old and nearing their useful life. The park has been rehabilitating septic trenches that have clogged over the years.

#### 3.3 Reasonable Growth

#### 3.3.1 Zoning

The Town's zoning regulations are established to preserve the look and feel of the Richmond area while accommodating reasonable development and growth in designated areas. As shown on Figure 2 (Zoning Map) in Appendix A, the study area is located within four (4) zoning districts including:

- a. Gateway Commercial District (G)
- b. Commercial (C)
- c. Mobile Home Park (MHP)
- d. Agricultural/Residential (AR)

The Gateway Commercial District is a designated growth center which is designated to allow for commercial uses in an area that has importance as a scenic entrance to the Town of Richmond. There are various allowed and conditional uses as specified in the zoning regulations. Currently water supply and wastewater disposal in the area are both served by on-site individual systems. The zoning regulations allow for 1/3 acres lots for properties served by municipal water and sewer and 1 acre lots for those not served by municipal water and sewer.

The Commercial District also allows for 1/3 acres lots for properties served by municipal water and sewer and 1 acre lots for those not served by municipal water and sewer.

For the Mobile Home Park District, a lot which is not a mobile home park (MHP) shall not be less than 1 acre. A lot which is used for a MHP shall contain not less than 10 acres and individual lots within the park shall not be less than <sup>1</sup>/<sub>4</sub> acre.

The Agricultural/Residential District also allows for 1 acre lots with no provision for smaller lots with community water and sewer.

Various uses are allowed in each district and reference is hereby made to the Richmond Zoning Regulations as well as the Subdivision regulations for a complete list of allowed and conditional uses.

# 3.3.2 Flows

An evaluation of the capacity of the Town of Richmond water system to supply water to the proposed study area was conducted. The following information is evaluated in this section:

- Water System Reserve Capacity
- Existing and Proposed Water System Demands
- Sewer System Reserve Capacity
- Existing and Proposed Sewer System Demands

#### Water System Reserve Capacity

The reserve capacity of the water system is calculated by present average daily flow and the committed allocations for water connections from the water system average daily flow capacity. The present average daily flow is 80,000 gpd. Table 7 summarizes the committed allocations for water services which have not yet been connected. This information was obtained from the Town officials.

Table		
Unconnected ( Water Allocated		
	Unconnected Committe Water Allocated Flow	
Applicant	(gpd)	3
Creamery (32 accts x 450 gpd)		14,400
Four Residences (4 accts x 450 gpd)		1,800
Total Unconnected	1	16,200
<b>Committed Water Allocations</b>		
Table 8 summarizes the water system capacity		



Estimated Water Capacity Analysis - 2014	Estimated Water Capacity Analysis - 2014				
Description	Capacity/Flow				
New Potable Water Reservoir Capacity	760,000 Gal				
- Present Average Daily Flow	80,000 gpd				
- Unconnected Committed Water Allocated Flows	16,200 gpd				
Net New Water Reservoir Capacity (including fire protection)	663,800				

Table 8

#### **Existing and Future Water System Demands**

Water flow projections were developed using the average flow numbers for the Richmond Village Area. Water flow demands for residential and apartment units were developed based on an average daily demand flow of 100 gpd per residential unit. For this study, it is assumed that each residence averages three (3) bedrooms. Water demand flow projections for businesses and other non-residential properties were developed using Table A2-1 of the Water Supply Rules. Table 9 (following page) provides a summary of the water system average demands for the existing Study Area properties.





Table 9Estimated Study Area Water and Wastewater Existing Flow Demand

Estimated Study Area water and wastewater Existing Flow Demand					
Phase/ Address	Use Description	User Type	Quantity	Flow* Basis	Ave. Daily Flow (gpd)
Phase 1	Description	cser rype	Quantity	Dusis	
840 W Main	Commercial	Reap Office Building/ Employees	42	15 gpd/staff	630
Subtotal Phase	e 1		1		630
Phase 2					
878 W Main	Residential	Single Family Home	1	100 gpd/Unit	100
920 W Main	Res./Commercial	Single Family Home/Tow Business	1	100 gpd/Unit	100
932 W Main	Residential	Single Family Home/Home Business	1	100 gpd/Unit	100
978 W Main	Residential	Single Family Home	1	100 gpd/Unit	100
1010-1014 W Main	Residential	Duplex	2	100 gpd/Unit	200
1008-1012 W Main	Residential	Duplex	2	210 gpd/Unit	200
1070 W Main	Commercial	Office Bldg/Employees	20	15 gpd/staff	300
1108 W Main	Commercial	Dog Day Care Employees Kennels Grooming Station	8 40	15 gpd/staff 25 gpd/kennel 400 gpd/station	120 1,000 400
1151 W Main	Res./Commercial	Residence Chiropractor Office	1 3 16	100 gpd/Unit 35 gpd/staff 10 gpd/patient	100 105 160
-	Vacant	Hay barn	-	-	-
-	Vacant	Field South Side	-	-	_
-	Vacant	Empty Lot	-	-	-
Subtotal Phase			1	L	2,985
Subtotal Phase	e 1 and 2				3,615
Phase 3					
1436 W Main	Commercial Gas Station	1 <sup>st</sup> Pump Set Additional Pump Sets Employees	1 3 6	500 gpd/Pump 300 gpd/Pump 15 gpd/staff	500 900 90
9 Gov. Peck	Commercial- Fuel	Employees	8	15 gpd/staff	120
116 River Rd	Commercial - Fuel	Employees	10	15 gpd/staff	150
Rte. 117	Mobile Home Park	Mobile Homes	148	142 gpd/MH	21,016
Subtotal Phase	e 3				22,626
	e 1, 2 and 3				26,241

\*Based on estimates, State "book flows" or existing State Permits except for Mobile Home Park which is metered



Future water system demands were estimated based on existing demand, together with projected development and build out. Table 10 provides a summary of the future estimated Study Area water system average demands.

#### Table 10

#### **Estimated Study Area Water System Future Flows**

Phase/ Address	Use Description	User Type	Quantity**	Flow* Basis	Average Daily Flow (gpd)
Phase 1					
840 W Main				Existing Flow	630
		New Office Building	51	15 gpd/employee	765
		Preschool/Day Care	30	15 gpd/staff & Child	450
		Barn Conversion	1	Estimated Set Aside	800
		·		Subtotal Phase 1	2,645
Phase 2					,
				Existing Flow	2985
878 W Main	Res./Commercial	Residential	2	100 gpd/Unit	200
		Commercial	2	300 gpd/Unit	600
920 W Main	Res./Commercial	Residential	2	100 gpd/Unit	200
		Commercial	2	300 gpd/Unit	600
932 W Main	Res./Commercial	Residential	3	100 gpd/Unit	300
		Commercial	3	300 gpd/Unit	900
978 W Main	Res./Commercial	Residential	2	100 gpd/Unit	200
		Commercial	2	300 gpd/Unit	600
1010-1014	Res./Commercial	Residential	2	100 gpd/Unit	200
W Main		Commercial	2	300 gpd/Unit	600
1008-1012	Res./Commercial	Residential	2	100 gpd/Unit	200
W Main		Commercial	2	300 gpd/Unit	600
1070 W Main	Res./Commercial	Residential	2	100gpd/Unit	200
		Commercial	2	300 gpd/Unit	600
1108 W Main	Res./Commercial	Residential	1	100 gpd/Unit	100
		Commercial	1	300 gpd/Unit	300
1151 W Main	Res./Commercial	Residential	1	100 gpd/Unit	100
		Commercial	1	300 gpd/Unit	300
-	Vacant- Residential	Hay barn- Residential	1	100 gpd/Unit	100
-	Vacant- Residential	Field South Side- Residential	1	100 gpd/Unit	100
-	Vacant- Comm/Res	Empty Lot			
		Residential	2	100 gpd/Unit	200
		Commercial	2	300 gpd/Unit	600
		·		Subtotal Phase 2	10,685
				Subtotal Phase 1 and 2	13,330
Phase 3					)
				Existing Flow	22,626
Rt 117	Mobile Home Park	Mobile Home	100	142 gpd/MH	14,200
	•			Subtotal Phase 3	36,826
			Su	btotal Phase 1, 2 and 3	50,156

\*Based on average Richmond Village flows for Residential and State of VT "book flows"

for 20 employees per commercial unit (15gpd x 20 = 300 gpd).

\*\* Approx. "Build out" based on allowable lots and Res./Commercial mix for each district.

Description	Existing**	Estimated Full Build- Out***
Available* Reservoir Capacity (including fire protection)	663,800	663,800
Phase 1 Flows	630	2,645
Remaining Capacity (including fire protection)	663,170	661,155
Phase 2 Flows	2,985	10,685
Remaining Capacity (including fire protection)	660,185	650,470
Phase 3 Flows	22,626	36,826
Remaining Capacity (including fire protection)	637,559	613,644
*See Table 2 **See Table 3 ***See	Table 4	

# Table 11Estimated Future Water Reservoir Capacity Analysis

# WWTF Uncommitted Reserve Capacity

The uncommitted reserve capacity of the Wastewater Treatment Facility (WWTF) is calculated by subtracting both the 12-month annual average daily flow and the committed allocations for sewer connections from the permitted capacity. The WWTF permitted capacity is 222,000 gallons per day (gpd). The 12- month annual average daily flow from August 2013 through July 2014 is 70,167 gpd as summarized in Table 12. This is calculated based on the monthly average flows as reported on the WWTF WR-43 monthly reports.



	Average
	<b>Daily Flow</b>
Month/Year	(gpd)
August 2013	65,000
September 2013	67,000
October 2013	61,000
November 2013	59,000
December 2013	61,000
January 2014	72,000
February 2014	61,000
March 2014	71,000
April 2014	97,000
May 2014	77,000
June 2014	78,000
July 2014	73,000
12-MonthAve.	70,167

Table 12 WWTF 12-Month Annual Average Daily Flow

Table 13 summarizes the committed allocations for sewer connections which have not yet been connected. This information was obtained from the Town officials.

Table 13Estimated Unconnected CommittedSewer Allocated Flows			
Applicant	Unconnected Committed Sewer Allocated Flows (gpd)		
Creamery (32 accts x 210 gpd)	6,720		
Four Residences (4 accts x 210 gpd)	840		

**Committed Sewer Allocations** 

**Total Unconnected** 

Table 14 summarizes the WWTF uncommitted sewer capacity allocation.

Estimated Sewer Uncommitted Reserve Capacity			
Description	Flow (gpd)		
WWTF Permitted Capacity	222,000		
80% of WWTF Permitted Capacity	176,000		
- 12-Month Annual Average Daily Flow	70,167		
- Unconnected Committed Sewer Allocated Flows	7,560		
= WWTF Uncommitted Reserve Capacity	98,273		

Table 14



7,560

#### **Existing and Future Wastewater Flows**

Wastewater flow projections were developed using the local average daily flows for the Richmond Village area and the State of Vermont, Environmental Protection Rules (EPR), Chapter 1, dated September 29, 2007. Flow demands for residential and apartment units were developed based on the number of living units. A living unit is defined as a single family home, apartment, or mobile home. A design flow of 100 gpd per living unit is used for wastewater without regard to the number of bedrooms. Wastewater flow projections for businesses and other non-residential properties were developed using Table 2 of the Rules. Sewer line infiltration was estimated for gravity sewer lines using 300 gal/in. pipe/dia/mile/day, as required by the rules. Infiltration is not accounted for in pressure pipes force mains and grinder low pressure sewers.

Table 15 outlines the available sewage treatment capacity in the existing WWTF.

Estimated Wastewater Capacity Analysis				
Description	Existing	Estimated Full Build-Out		
Available Capacity	98,273	98,273		
Phase 1 Flows	630	2,645		
Remaining Capacity	97,643	95,628		
% Remaining of Available Capacity	99%	97%		
Phase 2 Flows	3,975	13,875		
Remaining Capacity	93,668	81,753		
% Remaining of Available Capacity	95%	83%		
Phase 3 Flows	25,760	41,760		
Remaining Capacity	67,908	39,933		
% Remaining of Available Capacity	69%	41%		

Table 15Estimated Wastewater Capacity Analysis

#### 4.0 ALTERNATIVES CONSIDERED

#### 4.1 Description

#### 4.1.1 Water

Because the Town would like to provide fire flow to West Main Street and the mobile home park, there are few alternatives to consider besides the do nothing alternative. The Town prefers to use PVC pipe and not ductile iron pipe. Therefore, we did not evaluate the PVC vs. ductile iron. Because of the long runs between West Main Street and the mobile home park, plus wetlands and limited room in the right of way, HDPE pipe by directional boring was chosen as the alternative for the water extension. The size of the water extension is based on the water system hydraulic analysis provided in the next section.

#### 4.1.2 Sewer

# Alternatives for Connection into Existing Gravity Sewer System

It was determined that the logical place to connect into the Town's sewer system was the gravity sewer on Jericho Road at the Elementary/Middle school entrance road. Three (3) alternatives were considered for wastewater collection and transmission to the existing gravity sewer system.

One alternative evaluated was to pump the wastewater from West Main Street to the middle school wastewater pump station located in the northwestern corner of the school, which in turn pumps wastewater through an existing forcemain to the "B" line gravity sewer on Jericho Road. The middle school wastewater pump station consists of a 4 ft diameter wet well, and a steel dry well consisting of two (2) 500 gpm vertical centrifugal pumps and valves. The forcemain is a 4" cast iron and runs along the roadway on the northern side of the school. Although the pumps are adequate for the school and wastewater flow from the West Main Street sewer extension, the school's 4 ft diameter wet well is under sized for its current use. There is not enough storage capacity to meet the required 4 hours of storage in the event of a power outage. The wet well would need to be expanded to accommodate operating capacity and storage. This upgrade would result in

increased project costs, therefore, it was determined that connecting to the school's pump station is not viable.

A second alternative was a connection to the school's existing forcemain utilizing a valve structure and a solids handling pump station and forcemain from below, on West Main Street. This would save a significant amount of forcemain pipe in order to run to the Jericho Road gravity sewer. It was determined that this alternative would only be viable for an alternative that included a gravity collection system and pump station on West Main Street in order to maintain a minimum of 3 feet per second velocity in the forcemain. Utilization of grinder pumps from this location was not feasible because of the size of the pumps needed to maintain a minimum of 3 feet per second velocity in the forcemain.

The third alternative is for a 3" low pressure sewer running parallel to the school's forcemain and discharging separately into the main hole. This would allow to maintain a minimum of 3 feet per second velocity in the forcemain for a grinder pump system alternative without having the pump horsepower too high.

#### Sewer System Extension Alternatives

Three (3) sewerline extension alternatives were evaluated including:

- <u>Alternative No. 1: 3" force main and grinder pumping system from RCMHP</u> to #1151 West Main with 8" gravity sewer along Route 2 with a municipal pump station near the Reap property. The pump station would then pump the sewage through a 4" forcemain and connect into the middle School forcemain which connects to the gravity sewer on Jericho Road.
- <u>Alternative No. 2: A 3" grinder pump low pressure sewer along route 2 from</u> <u>RCMHP to Jericho Road</u>. The RCMHP and each building owner would be responsible for providing a grinder pump station and connection to the low pressure sewer main. The property owners would also be responsible for their own electrical costs. After evaluating the forcemain connection, it was determined that the grinder pump forcemain should not be connected to the school's 4" forcemain. A 3" forcemain is typically the largest diameter for grinder pump system without needing significant horsepower pumps in order to maintain

scouring velocities. Three alternatives for connection were evaluated including running a parallel forcemain to Jericho Road, upgrading the school's pump station with an expanded wet well and emergency storage, and upgrading the school's pump station with an expanded wet well and an emergency generator. The costs for each alternative are provided in Table 13. It is anticipated that 5hp pumps and single phase electrical service would be adequate for most connections but each proposed installation would need to be evaluated separately.

• <u>Alternative No. 3: A 2" STEP pump low pressure sewer along route 2 from</u> <u>RCMHP to Jericho Road</u>. The RCMHP and each building owner would be responsible for providing a Septic Tank Effluent Pumping Station and connection to the low pressure sewer main. The property owners would also be responsible for their own electrical costs. After evaluating the forcemain connection, it was determined that the STEP pump forcemain could be connected to the school's 4" forcemain. It is anticipated that 0.5hp pumps and single phase electrical service would be adequate for most connections but each proposed installation would need to be evaluated separately.

#### 4.2 Design Criteria

# 4.2.1 Water System Hydraulic Analysis

A hydraulic analysis of the Town of Richmond's water system was conducted using HydroCad® to evaluate the adequacy of the system including a water line extension for West Main Street. For the purpose of this report, a 7,900' extension with hydrants located at the Reap property, the high point of the line near the Crate Escape, the mobile home park entrance and the upper level of the mobile home park was analyzed. The analysis was performed to determine the system pressures for both average use and for different fire flow situations. Analysis was performed using the Town's new reservoir, which was placed in service in December of 2015.

Table 16 provides a summary of the water system hydraulic analysis. The State of Vermont, Water Supply Rules require a minimum pressure of 20 psi under all conditions of flow. The Town has a maximum pressure requirement of 100 psi before installing a pressure reducing valve. As shown in Table 6, the new 8" and 10" water lines meet the



pressure requirements. The new reservoir would need to be in operation before installing any hydrants west of the Reap property.

Summary of Water System Hydraulic Analysis						
	Pressure	Pressure	Pressure	Pressure		
	At	At	At	At		
	Reap	Crate Escape	RCMHP	Upper		
	Hydrant	Hydrant	Hydrant @	RCMHP		
Condition	(psi)	(psi)	Rte. 117	Hydrant		
			(psi)	(psi)		
	02.1	00.2	00.0	70.1		
50 yr. Max Day Demand	92.1	90.3	99.8	72.1		
1,500 gpm Fire Flow@ Reap	52.0	50.3	59.7	52.1		
1,000 gpm Fire Flow@Crate Escape	72.6	60.4	69.8	42.1		
1,000 gpm Fire Flow@ RCMHP/117	72.6	60.4	52.0	24.3		
500 gpm Fire Flow@Upper RCMHP	86.2	81.3	82.0	50.9		

Table 16	
rv of Water System Hydraulic Ai	nal

4.2.2 Existing Gravity Sewer System Capacity

The capacity of the Town of Richmond's gravity sewer from the manhole on Jericho Road along the "B" line sewer to the Wastewater Treatment Facility was evaluated for this project. The gravity sewer was evaluated manhole to manhole using the as-built drawings prepared by Webster-Martin, Inc. dated 1971. A program named FlowMaster® was used to evaluate the full flow capacity of the gravity sewers. The pipe diameter, pipe type, and slope were entered into the program for each segment of pipe. Based on the inputs, the program calculated the full flow capacity in millions of gallons per day. The program uses several factors to calculate full flow capacity including roughness of the pipe, geometric configuration (cross-section and length), and slope. The Continuity Equation and the Manning Equation for steady-state flow are used by the program to calculate the flow in a sewer pipe:

Continuity Equation:  $Q = V \ge A$ 

Q = peak flow, cubic feet per second (cfs).

V = velocity, feet per second (fps).

A = cross-sectional area of pipe, square feet (sf).

Manning Equation: V = (1.486 x R2/3 x S1/2)/n

V = velocity, fps.

n = Manning's coefficient of friction.



R = hydraulic radius (area divided by wetted perimeter), feet.

S = slope of pipe, feet per foot.

Table 17 provides a summary of the full flow capacity of the existing gravity sewer lines. As shown on Table 17, the gravity sewer lines have significant capacity available above the treatment plant capacity.

Existin	Existing Gravity Sewer System Capacity						
				Segment			
				<b>Full Flow</b>			
Pipeline	Diameter		Slope	Capacity			
Segment	(in.)	Туре	(ft/ft)	(MGD)			
32A - 32	8	AC	0.0040	0.584			
32 - 31	8	AC	0.0040	0.584			
31 - 30	8	AC	0.0563	2.190			
30 - 29	8	AC	0.0043	0.605			
29 - 28	8	AC	0.0040	0.584			
28 - 27	8	AC	0.0040	0.584			
27 - 26	8	AC	0.0103	0.937			
26 - 25	8	AC	0.0040	0.584			
25 - 24	8	AC	0.0152	1.138			
24 - 23	8	AC	0.1551	2,744			
23 - 22	8	AC	0.0040	0.584			
22 - 21	8	AC	0.2308	4.434			
21 - 20	8	AC	0.0580	2.223			
20 – 19A	8	AC	0.0040	0.584			
19A - 19	8	AC	0.0040	0.584			
19 - 18	8	AC	0.0040	0.584			
18 - 17	8	AC	0.0040	0.584			
17 - 16	8	AC	0.0040	0.584			
16 - 15	8	AC	0.0124	1.028			
15 - 13	8	AC	0.0277	1.536			
13 - 12	10	AC	0.0021	0.767			
12 - 11	10	AC	0.0028	0.886			
11 - 10	10	AC	0.0280	2.800			
10 - 9	10	AC	0.0097	1.648			
9 - 8	10	AC	0.0239	1.420			
8 - 7	10	AC	0.0072	1.420			
7 - 2	10	AC	0.0022	0.785			
2 - 1	12	AC	0.0022	1.276			

Table	17
Lanc	1/

System aait  $\mathbf{\alpha}$ 

MGD= Million Gallons per Day

#### 4.3 Map

Maps of the alternatives are provided in Appendix A.

#### 4.4 Environmental Impacts

The potential environmental impacts for this project are to wetlands, stream crossings and important farmland during construction. These impacts will be mitigated by using HDPE pipe and directional boring rather than open cut construction. Impacts will also be mitigated by using proper erosion and sedimentation controls. The ground will be restored to its original condition which will not impact the future use for important farmland soils.

#### 4.5 Land Requirements

An easement would be needed from the Reap property for the cross country portion of the project from the school to Route 2. The remainder of the project is within the right of way of The Vermont Agency of Transportation. A Vtrans permit will be required for work within these areas. For the gravity sewer alternative approximately <sup>1</sup>/<sub>4</sub> acre would need to be purchased.

#### 4.6 Potential Construction Problems

The major construction problems with the gravity sewer system alternative is the depth of sewer within a very tight right of way. The low pressure sewer alternatives provide better access within the right of way through directional drilling.

# 4.7 Sustainability Considerations

# 4.7.1 Water and Energy Efficiencies

The use of HDPE pipe helps promote water efficiency as there are fewer joints which promotes fewer leaks.

The grinder pump and STEP low pressure sewer alternatives provides significant energy efficiencies over large municipal solids handling pump stations both at the mobile home park and at the bottom of Route 2.

# 4.7.2 Green Infrastructure

There are no green infrastructure consideration in this project.

#### 4.8 Cost Estimates

#### 4.8.1 Opinion of Probable Construction Costs

Opinions of probable construction costs were developed for the water and sewer extension alternatives. Prior to development of the construction cost estimates, quantity take-offs were completed to establish unit quantities for projected project unit price bid items. Construction costs were generated using unit price bids on recent construction projects in the area. The construction costs are based on the assumption that work will be performed by an independent general contractor. The construction costs also include a 10% contingency.

Detailed opinion of probable construction costs for each project item is provided in Appendix C. Because it is not known when each of these projects will occur, current and future projected construction cost estimates were developed using the Engineering News Record (ENR) Construction Cost Index (CCI). Current 2016 construction cost estimates (ENR 9750) were developed by adjusting the unit price items from similar jobs to today's dollars using a ratio of ENR values. Estimates for future ENR values were developed by graphing the last ten (10) years of ENR values and projecting a best fit line into the future and estimating the future ENR values. Construction cost estimates were then projected out for to 2017.

Table 18 (following page) provides a summary of the opinion of probable construction costs for the years 2016 (ENR 10000), and 2017 (ENR 10200). The Town has decided that the cost for utilities on private property will be borne by the property owners. The typical cost for a gravity connection (alternative No. 1) is approximately \$3,000 (39,000 for 13 systems). The typical cost for a grinder pumping system is approximately \$4,000 (\$52,000 for 13 systems). The typical cost for A STEP system is approximately \$5,000 (\$65,000 for 13 systems).

# Table 18Opinion of Probable Construction Cost

	Opinion of Construc	
Project	ENR 10000	ENR 10200
	2016	2017
8" Waterline Extension		
	¢220.000	¢224.000
PH1- School to West Main Street (Reap Property) PH2- Reap Property to Chiropractor Office	\$229,000 \$296,000	\$234,000 \$302,000
Subtotal	\$525,000	\$536,000
PH3- Chiropractor Office to Mobile Home Park	\$727,000	\$742,000
Total	\$1,252,000	\$1,278,000
Sewer Extension Alternatives		
Alterative No. 1		
Gravity Sewer/Pump Station/Forcemain		
PH1- Sewer Pump Station & 4" Forcemain Reap Property to School	\$389,000	\$396,000
PH2- 8" Gravity Sewer- Reap Property to Chiropractor Office	\$200,000	\$204,000
Subtotal	\$589,000	\$600,000
PH3- Chiropractor Office to Mobile Home Park (Grinder System)	\$537,000	<u>\$548,000</u>
Total	\$1,126,000	\$1,148,000
Alternative No. 2	K	
3" Low Pressure Sewer Grinder Pump Forcemain	A151000	¢1 <b>7</b> 0.000
PH1- Reap Property to School	\$174,000	\$178,000
PH2- Reap Property to Chiropractor Office Subtotal	<u>\$143,000</u>	<u>\$146,000</u>
	<b>\$317,000</b> <u>\$537,000</u>	\$324,000
PH3- Chiropractor Office to Mobile Home Park Total	\$854,000	<u>\$548,000</u> <b>\$872,000</b>
Iotal	\$0 <b>54,</b> 000	\$672,000
Alternative No. 3		
2" Low Pressure Sewer STEP Pump Forcemain		
PH1- Reap Property to School	\$168,000	\$171,000
PH2- Reap Property to Chiropractor Office	\$139,000	\$142,000
Subtotal	\$307,000	\$313,000
PH3- Chiropractor Office to Mobile Home Park	\$527,000	\$538,000
Total	\$834,000	\$851,000
	•	

# 4.8.2 Operation and Maintenance Costs

Table 19 provide a summary of the estimated Operation and Maintenance cost for each of the sewer extension alternatives.

# Table 19

# O&M Costs

Debt	Alternative 1 Gravity Sewer	Alternative 2 LPS Grinder	Alternative 3 LPS STEP
Administration	\$2,000	\$2,000	\$2,000
Engineering	\$500	\$500	\$500
Biosolids Disposal	\$13,000	\$13,000	\$13,000
Insurance	\$2,000	\$0	\$0
Repairs/Maintenance	\$10,000	\$5,000	\$5,000
Salaries/Benefits	\$0	\$0	\$0
Supplies	\$10,000	\$7,500	\$7,500
Utilities	\$18,000	\$11,900	\$11,900
Septage Pumping	\$0	\$0	\$1,500
Total	\$55,500	\$39,926	\$41,400

#### 5.0 SELECTION OF AN ALTERNATIVE

The economic analysis for the various alternatives is displayed in terms of the present worth (life cycle) of each alternative over the required 20-year planning period. Present worth calculations were performed using the federal discount rate (1.20%) from Appendix C of OMB Circular A-94 for establishing the present worth of the uniform series of O, M & R values (in today's dollars).

# 5.1 Life Cycle Cost Analysis

Life cycle cost estimates were not performed for the waterline alternatives because HDPE pipe was selected due to installation reasons.

Table 20 summarizes the cost-effective analysis of the collection system alternatives. Based on this analysis, it is concluded that the grinder pump low pressure sewer alternate is the preferred choice.

Debt	Alternative 1	Alternative 2	Alternative 3
	Gravity Sewer	LPS Grinder	LPS STEP
Construction (Public)	\$1,126,000	\$854,000	\$834,000
Construction (Private)	\$39,000	\$52,000	\$65,000
O&M Cost	\$55,500	\$39,926	\$41,400
Present Wort Analysis 20 Years @ 1.2%			
Construction (Public)	\$1,126,000	\$854,000	\$834,000
Construction (Private)	\$39,000	\$52,000	\$65,000
O&M Cost	\$943,500	\$678,800	\$703,800
Total	\$2,108,500	\$1,584,800	\$1,602,800

 Table 20

 Present Worth Analysis of Sewer Collection System Alternatives

#### 5.2 Non-Monetary Factors

#### 5.2.1 Advantages and Disadvantages of Gravity Sewers

#### Advantages

- Conventional gravity sewers are standard technology and gave been used for many years with procedures for their design well established.
- Gravity sewers can handle grit and solids.
- Additional service or lateral connections can be made easily at any time in the future, as the need arises.



- Gravity sewers are less expensive to connect to in the future than low pressure sewers.
- Frequent manholes provide ready access for regular inspection and maintenance of the sewers
- Gravity Sewers maintain a minimum velocity which reduces the production of hydrogen sulfide and methane which reduces odors, blockages, pipe and concrete corrosion, and the potential for hazardous or explosive gases.

#### Disadvantages

- Gravity sewers must be laid at a constant slope or grade and can become excessively deep, thereby, requiring a pump station.
- Gravity sewers require accounting for infiltration in the design flows unlike a low pressure sewer.
- The cost maintenance of operating a large pump station is substantially higher than individual low pressure sewer pumps.
- 5.2.2 Advantages and Disadvantages of Grinder Pump Low Pressure Sewers

#### Advantages

- Low pressure sewers have smaller diameter pipes which is easier to work with.
- Low pressure sewers do not need to be laid to line and grade. They are typically buried six (6) foot deep and follow the lay of the land.
- Shallower installations are easier to maintain if problems occur.
- Gravity sewers are less expensive to connect to in the future than low pressure sewers.
- Solids are transported to the WWTF through the low pressure sewer pipe which creates less maintenance than STEP system which need septic tanks inspected and pumped out at regular intervals.
- Electrical costs are borne by the individual users.
- Grinder pump use significantly less electricity with smaller horsepower motors than a municipal solids handling pump station.

# Disadvantages

- Grinder pumps use more electricity than STEP pumps.
- Individual wastewater pumps are required at each property.
- Grinder pump typically require more maintenance than STEP pumps, especially with items getting stuck in the cutters.
- Grinder pump system are significantly more expensive for future connections than gravity systems.



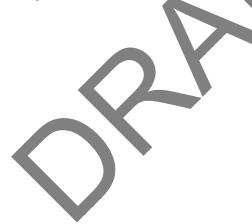
#### 5.2.3 Advantages and Disadvantages of STEP Pump Low Pressure Sewers

#### Advantages

- Low pressure sewers have smaller diameter pipes which is easier to work with.
- Low pressure sewers do not need to be laid to line and grade. They are typically buried six (6) foot deep and follow the lay of the land.
- Shallower installations are easier to maintain if problems occur.
- Gravity sewers are less expensive to connect to in the future than low pressure sewers.
- STEP pumps have use less electricity and have smaller horsepower motors than grinder pumps.
- Electrical costs are borne by the individual users.
- STEP pumps last longer and have fewer maintenance problems than grinder pumps.

#### Disadvantages

- STEP systems require regular inspection of the grease and sludge in the septic tanks.
- STEP systems require hiring a septage hauler or individual Town owned equipment to pump out the septic tanks.
- Individual wastewater pumps are required at each property.
- STEP systems are significantly more expensive for future connections than gravity and grinder systems.



#### 6.0 PROPOSED PROJECT

#### 6.1 Preliminary Project Design

This project entails extending the existing water and sewer from the Richmond Elementary School cross country to West Main Street (Route 2) to service the zoned "Gateway" growth area along West Main Street and then West to the River View Commons Mobile Home Park. The project will include 9,000 l.f. of new 8" HDPE waterline by directional drilling and 8,000 l.f. of new 3" HDPE grinder pump low pressure sewer with services to ROW and other appurtenances.

#### 6.2 Project Schedule

To complete a project, a large number of separate actions must occur. Many of those steps can occur simultaneously. It is expected that throughout the process public and private sector parties will be working closely to expedite the project. At this point in the Town's efforts to meet their goal of constructing expanding the water and sewer system to West Main Street and the mobile home park, the following steps together with their approximate time schedule are presented in Table 21.

Task	Date
Submit 90% Preliminary Engineering Report & Environmental Report to RD	12/23/16
RD Review	12/23/16 - 1/13/17
Engineer's Responses to RD's PER & ER Comments	1/20/17
Approval of PER and ER by RD	1/27/17
No Findings of Significant Impact (FONSI) Issuance	1/27/17
Obtain RD Obligation of Funds Letter	1/27/17
Start Final Design, Permitting and Easements	1/3/17
Finish Final Design, Permitting and Easements	6/30/17
Advertise for Bids	7/10/17
Pre-Bid Meeting	7/20/17
Bid Opening	8/10/17
Start Construction	8/24/17
Complete Construction	7/1/18

# Table 21Project Schedule



2017

The information presented in this report shows that the proposed improvements seem financially feasible. This is predicated on the basis that the Town is successful in being awarded a USDA-RD loan funding package.

## 6.3 Sustainability Considerations

6.3.1 Water and Energy Efficiencies

The use of HDPE pipe helps promote water efficiency as there are fewer joints which promotes fewer leaks.

The grinder pump low pressure sewer alternative provides significant energy efficiencies over large municipal solids handling pump stations both at the mobile home park and at the bottom of Route 2.

6.3.2 Green Infrastructure

There are no green infrastructure initiatives as part of this project.

6.4 Total Project Cost Estimate

Total project costs include construction, final design, and construction engineering costs. Table 22 provides a summary of the total project cost estimates for the 2016 (ENR 10000) and 2017 (ENR 10200). Final design and construction engineering service cost estimates are based on the State of Vermont, Facility Engineering Division, Engineering Services Curve formulas. These costs do not include land acquisition, advertisement or legal fees.



Opinion of Probable Total Project Cost Summary- Water           Total Project Cost					
imate					
ENR10200	ENR10000	Project			
2017	2016				
		Waterline Extension			
		Ph1: School to 840 West Main Street (Reap Property)			
	\$10,000	Preliminary Engineering			
\$234,000	\$229,000	Construction			
	\$16,000	Final Design			
\$29,000	\$29,000	Construction Engineering			
\$1,000	\$1,000	Admin.			
	\$1,000	Easements			
	\$3,000	Legal & Fiscal			
	<u>\$14,200</u>	Short Term Interest			
\$308,600	\$303,200	Phase 1 Subtotal			
		Ph2: 840 West Main (Reap Property)			
\$302,000	\$296,000	to 920 West Main - Chiropractor Office			
	\$23,000	Construction			
	\$41,000	Final Design			
. ,	\$1,000	Construction Engineering			
	\$1,000	Admin.			
	\$3,000	Easements			
	\$14,800	Legal & Fiscal Short Term Interest			
	\$379,000	Short Term Interest Phase 2 Subtotal			
	\$682,200	Phase 2 Subotar Phase 1 and 2 Total			
		Ph3: 920 West Main - Chiropractor Office to Mobile Home Park			
\$742,000	\$727,000	Construction			
\$50,000	\$49,000	Final Design			
\$92,000	\$91,000	Construction Engineering			
	\$3,000	Admin.			
	\$3,000	Easements			
	\$9,000	Legal & Fiscal			
	<u>\$36,400</u>	Short Term Interest			
<u>\$936,500</u>	<u>\$918,400</u>	Phase 3 Subtotal			
\$1,631,300	\$1,600,600	Phase 1, 2 and 3 Total			
) )	<u>\$36,400</u> <b>\$918,400</b>	Short Term Interest Phase 3 Subtotal			

# Table 22A Opinion of Probable Total Project Cost Summary- Water

Project Sewer Extension Alternatives Alternative No. 2: 3" Sewer Grinder Pump Force main	Esti ENR10000 2016	mate ENR10200
Sewer Extension Alternatives		ENR10200
	2016	
	2010	2017
Alternative No. 2, 27 Server Crinder Burn Force main		
Anernauve No. 2: 5° Sewer Grinder Pump Force main		
Ph1: School to 840 West Main Street (Reap Property)		
Preliminary Engineering	\$10,000	\$10,000
Construction	\$174,000	\$178,000
Final Design	\$14,000	\$14,000
Construction Engineering	\$25,000	\$26,000
Admin.	\$1,000	\$1,000
Easements	\$1,000	\$1,000
Legal & Fiscal	\$2,500	\$2,600
Short Term Interest	\$9,000	\$9,200
Phase 1 Subtotal	\$236,500	\$241,800
T hase T Subtotal		
Ph2: 840 West Main (Reap Property)		
to 920 West Main - Chiropractor Office		
Construction	\$143,000	\$146,000
Final Design	\$12,000	\$12,000
Construction Engineering	\$22,000	\$22,000
Admin.	\$1,000	\$1,000
Easements	\$1,000	\$1,000
Legal & Fiscal	\$2,000	\$2,000
Short Term Interest	\$7,200	\$7,400
	#100 <b>•</b> 00	\$191.400
Phase 2 Subtotal Phase 1 and 2 Total	A 43 4 500	\$433,200
Phase 1 and 2 1 otal	¢ ;,, o o	¢,
Ph3: 920 West Main - Chiropractor Office		
to Mobile Home Park		
Construction	\$537,000	\$548,000
Final Design	\$39,000	\$40,000
Construction Engineering	\$72,000	\$73,000
Admin.	\$3,000	\$3,100
Admin. Easements	\$3,000	\$3,100
Legal & Fiscal	\$5,000	\$5,100
Short Term Interest	\$27,000	\$27,500
Short Term Interest Ph 3 Subtotal	\$ (9( 000	\$699,800
Ph 5 Subtotal		
Phase 1, 2 and 3 Total	\$1,110,700	\$1,133,000
	1	1

# Table 22B Opinion of Probable Total Project Cost Summary- Sewer

Table 22C			
<b>Opinion of Probable Total Project Cost Summary- Entire Project</b>			

Project	Total Project Cost Estimate ENR10200 2017
Preliminary Engineering	\$20,000
Construction	\$1,950,000
Construction Contingency (10%)	\$195,000
Final Design	\$155,000
Construction Engineering	\$283,000
Admin.	\$10,200
Easements	\$10,200
Legal & Fiscal	\$25,100
Short Term Interest	<u>\$110,800</u>
<b>Total</b>	<b>\$2,759,300</b>







### 6.5 Annual Operating Budget

### 6.5.1 Income

Table 23 provides a summary of the Town's existing rate structure for water and sewer.

System	User	User
	Туре	Rate
Water	Commercial	\$381/Unit
		\$9.77/1,000 gal.
	Residential	\$130.64/Unit
		\$10.43/1,000 gal.
Sewer	Commercial	\$519.98/Unit
		\$13.00/1,000 gal.
	Residential	\$174.55/Unit
		\$14.13/1,000 gal.

Table 23Existing User Rate Structure

Based on the existing service area user types and flow usage, the Town currently receives approximately \$277,072 annually in water revenues and \$363,603 in sewer revenues per year for user fees. The Town also receives approximately \$1,500 annually in the sale of water from hydrants and \$500 per year in water tap fees. The Town also receives approximately \$181,576 annually in septage receiving fees.

The Town has decided to charge a connection fee to the properties in the study area. Table 24 and 25 provides a summary of the hook on fees for water and sewer respectively.



 Table 24

 Estimated Study Area Water System Hook-On Fees

Phase/ Address	Use Description	User Type	Quantity	Flow For Fee Basis*	Ave. Daily Flow (gpd)	Hook- On Fee*
Phase 1	<b>F</b>			_ 000_00		
840 W Main	Commercial	Reap Office Building/ Employees	42	15 gpd/staff	630	\$1,341
Subtotal Ph	nase 1	Linployees			630	\$1,341
Phase 2						+-9
878 W Main	Residential	Single Family Home	1	450 gpd/Unit	450	\$1,001
920 W Main	Res./Commercial	Single Family Home/Tow Business	1	450 gpd/Unit	450	\$1,001
932 W Main	Res./Commercial	Single Family Home/ Home Business	1	450 gpd/Unit	450	\$1,001
978 W Main	Residential	Single Family Home	1	450 gpd/Unit	450	\$1,001
1010-1014 W Main	Residential	Duplex	2	450 gpd/Unit	900	\$1,851
1008-1012 W Main	Residential	Duplex	2	450 gpd/Unit	900	\$1,851
1070 W Main	Commercial	Office Building/Employees	20	15 gpd/staff	300	\$717
1108 W Main	Commercial	Dog Day Care Employees Kennels Grooming Station	8 40 1	<ul><li>15 gpd/staff</li><li>25 gpd/kennel</li><li>400 gpd/station</li></ul>	120 1,000 400	\$3,023
920 W Main	Res./Commercial	Residence Chiropractor Office	1 3 16	450 gpd/Unit 35 gpd/staff 10 gpd/patient	450 105 160	\$1,501
-	Vacant	Hay barn	-	-	-	
-	Vacant	Field South Side	-	-	-	
-	Vacant	Empty Lot	-	-	-	
Subtotal Ph	nase 2				6,135	\$12,947
Subtotal Ph	ase 1 and 2				6,765	\$14,288
Phase 3	1	V	T	1	1	
1436 W Main	Commercial - Gas Station	1 <sup>st</sup> Pump Set Additional Pump Sets Employees	1 3 6	500 gpd/Pump 300 gpd/Pump 15 gpd/staff	500 900 90	\$2,966
9 Gov. Peck	Commercial -Fuel	Employees	8	15 gpd/staff	120	\$377
116 River Rd	Commercial- Fuel	Employees	10	15 gpd/staff	150	\$433
Rte. 117	Mobile Home Park	Mobile Home	148	250 gpd/MH	37,000	\$70,080
Subtotal Ph					38,760	\$73,856
Subtotal Ph	ase 1, 2 and 3				45,525	\$88,144
		ok flows" or existing State	D	d = 1.90/Cal/Day		

\*Based on estimates State "book flows" or existing State Permits\*\*gpd x 1.89/Gal/Day + \$150 Inspection Fee

Table 25Estimated Study Area Wastewater Hook-On Fees

Phase/	Use			Flow For Fee	Average Daily Flow	Hook-On
Address	Description	User Type	Quantity	Basis*	(gpd)	Fee*
Phase 1						
840 W Main	Commercial	Reap Office Building/ Employees	42	15 gpd/staff	630	\$2,928
Subtotal Pha		630	\$2,928			
Phase 2						
878 W Main	Residential	Single Family Home	1	210 gpd/Unit	210	\$1,076
920 W Main	Res./Commercial	Single Family Home/Tow Business	1	210 gpd/Unit	210	\$1,076
932 W Main	Residential	Single Family Home/Home Business	1	210 gpd/Unit	210	\$1,076
978 W Main	Residential	Single Family Home	1	210 gpd/Unit	210	\$1,076
1010-1014	Residential	Duplex	2	210 gpd/Unit	420	\$2,002
W Main						
1008-1012	Residential	Duplex	2	210 gpd/Unit	420	\$2,002
W Main						
1070 W Main	Commercial	Office Bldg/Employees	20	15 gpd/staff	300	\$1,473
1108 W	Commercial	Dog Day Care				\$6,853
Main		Employees	8	15 gpd/staff	120	
		Kennels	40	25 gpd/kennel	1,000	
		Grooming Station	1	400gpd/station	400	
1151 W	Res./Commercial	Residence	1	210 gpd/Unit	210	\$2,245
Main		Chiropractor Office	3	35 gpd/staff	105	
			16	10 gpd/patient	160	
-	Vacant	Hay barn	-	-	-	
-	Vacant	Field South Side	-	-	-	
-	Vacant	Empty Lot	-	-	-	
Subtotal Phas					3,975	\$18,879
Subtotal Phas	se 1 and 2				4,605	\$21,807
Phase 3	ſ		1	Γ	Γ	l .
1436 W	Commercial	1 <sup>st</sup> Pump Set	1	500 gpd/Pump	500	\$6,721
Main	Gas Station	Add'l Pump Sets	3	300 gpd/Pump	900	
<u> </u>		Employees	6	15 gpd/staff	90	
9 Gov. Peck	Commercial-Fuel	Employees	8	15 gpd/staff	120	\$679
116River Rd	Commercial -Fuel	Employees	10	15 gpd/staff	150	\$812
Rte. 117Mobile Home ParkMobile Homes148210 gpd/MH					31,080	\$137,213
Subtotal Phase 3					32,840	\$145,425
Subtotal Phase 1, 2 and 3					37,445	\$167,233

\*Based on estimates, State "book flows" or existing State Permits \*\*gpd x 4.41/Gal/Day + \$150 Inspection Fee

# Table 26Estimated Future Water and Sewer Income

Table 26 provides a summary of the Town's existing and proposed water and sewer income based on the existing and proposed user base, the above rate structure and other income sources.

Income Type	Existing Water System	Proposed Water System	Existing Sewer System	Proposed Sewer System
User Fees	\$277,072	\$400,565	\$363,603	\$530,227
Sale of Water	\$1,500	\$1,500	-	-
(Hydrant)				
Tap Fees	\$500	\$500	\$1,000	\$1,000
Septage Fees	-	-	\$181,576	\$181,576
Total	\$279,072	\$402,565	\$546,179	\$712,803

#### 6.5.2 Annual O&M Costs

Table 27 provides a summary of the existing and proposed (with proposed project) O&M costs for the water system.

Table 27 Existing & Proposed Water System O&M Costs Debt Existing Proposed **0&M 0&M** Cost Cost Administration \$12,300 \$12,300 Engineering \$2,000 \$2,000 Capital Reserve \$48,000 \$52,000 Repairs/Maintenance \$45,000 \$50,000 Salaries/Benefits \$60,000 \$60,000 **Supplies** \$10,200 \$13,000 Utilities \$11,300 \$14,400 \$203,700 Total \$188,800

Table 28 provides a summary of the existing and proposed (with proposed project) O&M costs for the sewer system.



Debt	Existing	Proposed
	O&M	O&M
	Cost	Cost
Administration	\$18,143	\$20,000
Engineering	\$500	\$1,000
Biosolids Disposal	\$65,000	\$78,000
Insurance	\$11,200	\$11,200
Repairs/Maintenance	\$32,000	\$37,000
Salaries/Benefits	\$138,126	\$138,126
Supplies	\$46,500	\$54,000
Utilities	\$79,100	\$91,000
Total	\$390,569	\$430,326

Table 28Existing & Proposed Sewer System O&M Costs

#### 6.5.3 Debt Repayments

The Town has set a policy for this project that the new users within the extension will pay for 100% of the debt service for the project.

The Town has been working with USDA Rural Development (RD) for grants and loans for the project. RD has offered a loan only package with an interest rate of 1.875%. Term will be for 30 years for sewer and 40 years for water. The annual payment the 1.875%, 40 year water extension loan is \$35.55/\$1,000 borrowed. The annual payment the 1.875%, 30 year sewer extension loan is \$43.61/\$1,000 borrowed.

The Town has an approved bond vote for \$2,500,000. The water portion of that bond is \$1,475,000. The sewer portion of the bond is \$1,025,000. The difference between the bond amount and the current total project cost of \$2,764,300 (\$1,631,300 for water and \$1,133,000 for sewer) is due to the increase in construction costs between when the total project cost was initially estimated in 2015 and now. The Town will pay the difference with its own funds using connection fee funds.

The annual debt payment on the water portion of the loan is  $1,475,000 \times 35.55/$ ,000 borrowed which equals 52,436/year. The annual debt payment on the sewer portion of the loan is  $1,025,000 \times 41.63/$ ,000 borrowed which equals 42,671/year.

Because the Town's policy is to have the connected users within the project area pay for the debt costs, it is logical to spread that costs though out the users based on an equivalent user basis. Table 29 provides a summary of the number of equivalent users (EU) within the project area and their associated annual costs for both water and sewer debt service. The annual water debt service cost will be \$309/EU and the annual sewer debt service cost will be \$251/EU.

Phase/	Use		Equivalent	Annual Water Debt	Annual Sewer Debt
Address	Description	User Type	Users	Service	Service
840 W Main	Commercial	Reap Office Building/ Employees	2	\$618	\$502
878 W Main	Residential	Single Family Home	1	\$309	\$251
920 W Main	Res./Commercial	Single Family Home/Tow Business	1	\$309	\$251
932 W Main	Res./Commercial	Single Family Home/ Home Business	1	\$309	\$251
978 W Main	Residential	Single Family Home	1	\$309	\$251
1010-1014 W Main	Residential	Duplex	2	\$618	\$502
1008-1012 W Main	Residential	Duplex	2	\$618	\$502
1070 W Main	Commercial	Office Building/Employees	1	\$309	\$251
1108 W Main	Commercial	Dog Day Care	3	\$927	\$753
920 W Main	Res./Commercial	Residence Chiropractor Office	2	\$618	\$502
1436 W Main	Commercial - Gas Station	1 <sup>st</sup> Pump Set Additional Pump Sets Employees	4	\$1,236	\$2,008
9 Gov. Peck	Commercial - Fuel	Employees	1	\$309	\$251
116 River Rd	Commercial- Fuel	Employees	1	\$309	\$251
Rte. 117	Mobile Home Park	Mobile Homes	148	\$45,732	\$37,148
		Totals	170	\$52,530	\$43,674

 Table 29

 Annual User Water and Sewer Debt Service Payments



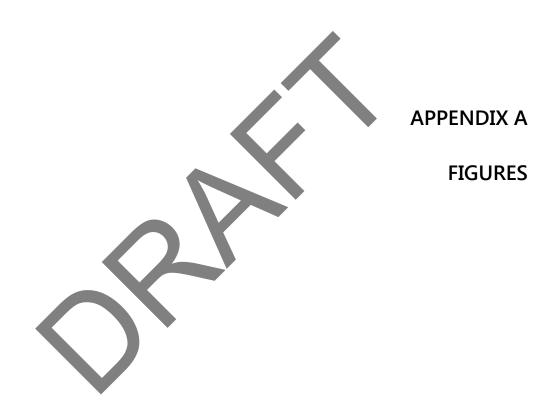
#### 6.5.4 User Rates

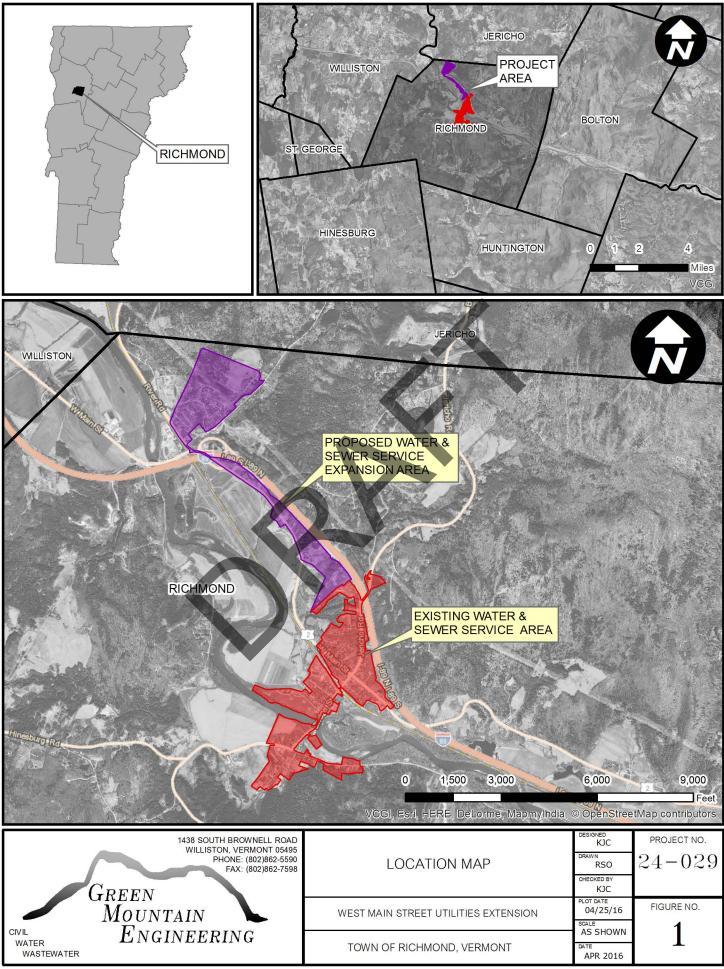
It is anticipated that adding the additional 170 users to the water and sewer system will reduce the overall rates throughout the system. For the first year, the Town intends to maintain the same user rates until it gets a firm handle on the total water and sewer income with adding the new users.

As stated in section 2.4.1, for average single family home using 190 gpd, the average yearly water fee for a typical single family home using 210 gpd is approximately \$854 and the average sewer user fee is approximately \$1,154. As shown in Table 24, the annual debt service for a single family home will be \$309 for water and \$251. With adding the debt service on to the expansion area users, the average user costs within the expanded area for a typical single family home will be approximately \$1,163 for water and \$1,405 for sewer.

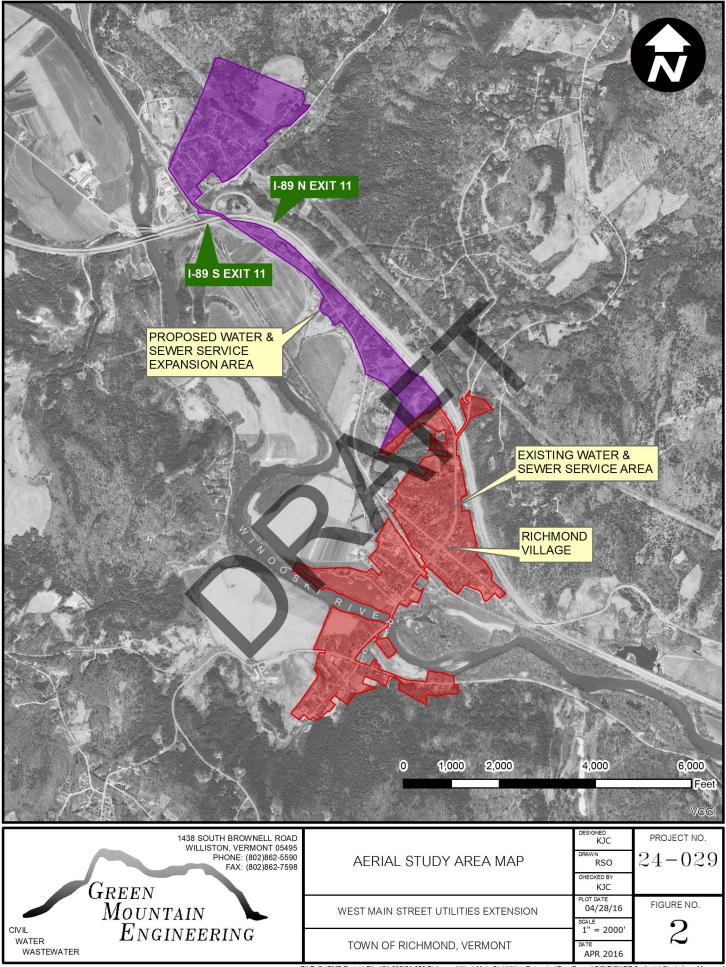




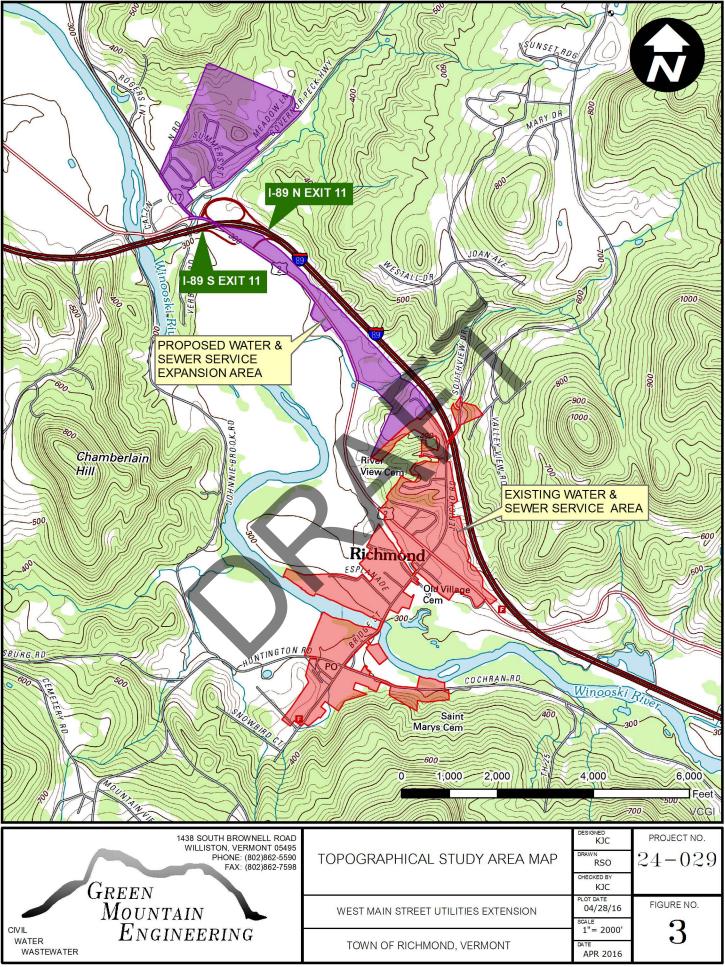


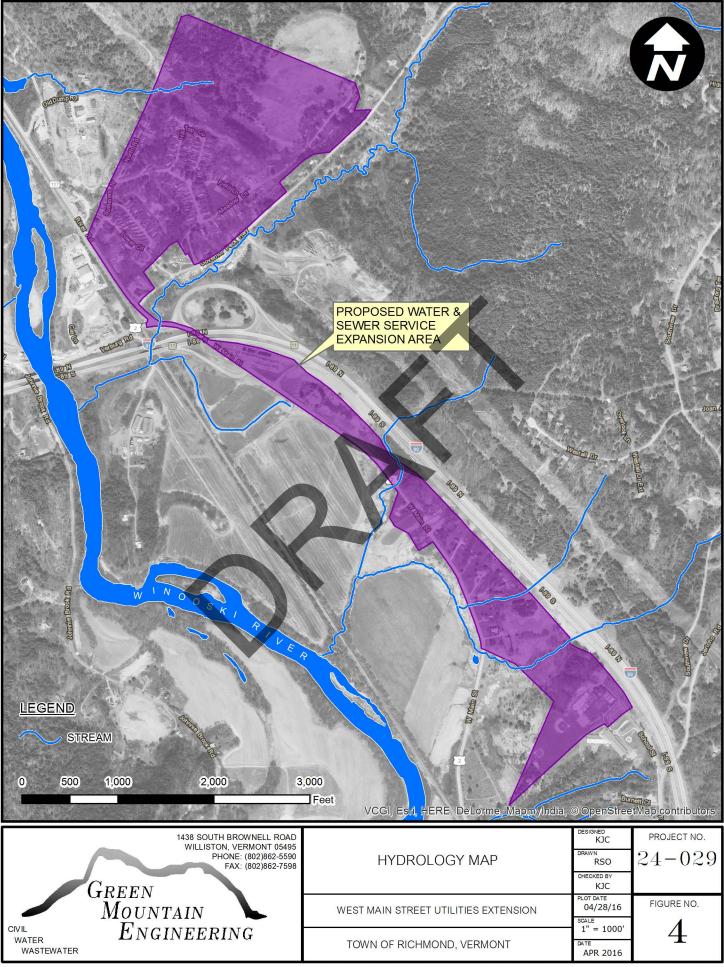


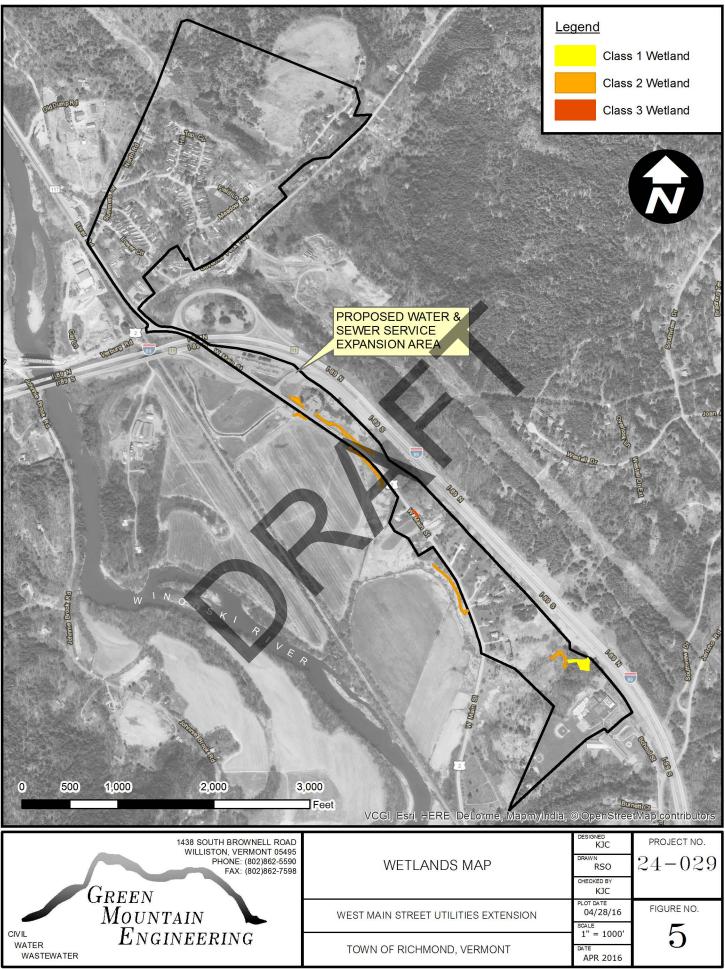
FILE:S:\GME Project Files\24-000\24-029 Richmond West Main St. Utilities Extension\Env. Report\GIS\FIGURE 1 - Location Map.mxd



FILE: S:\GME Project Files\24-000\24-029 Richmond West Main St. Utilities Extension\Env. Report\GIS\FIGURE 2 - Aerial Study Area Map.mxd



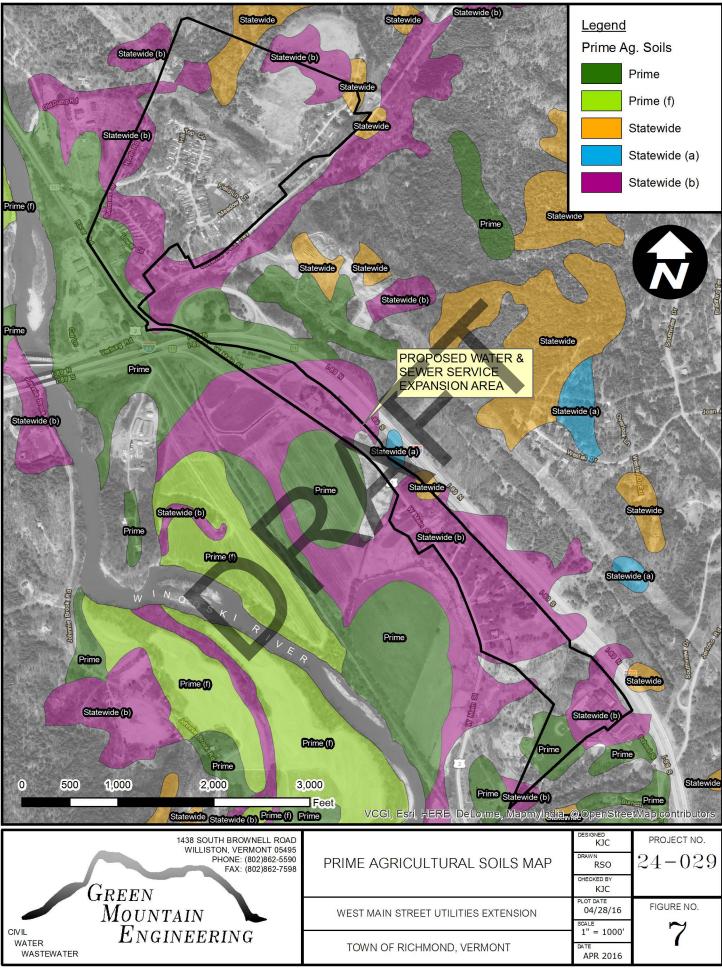




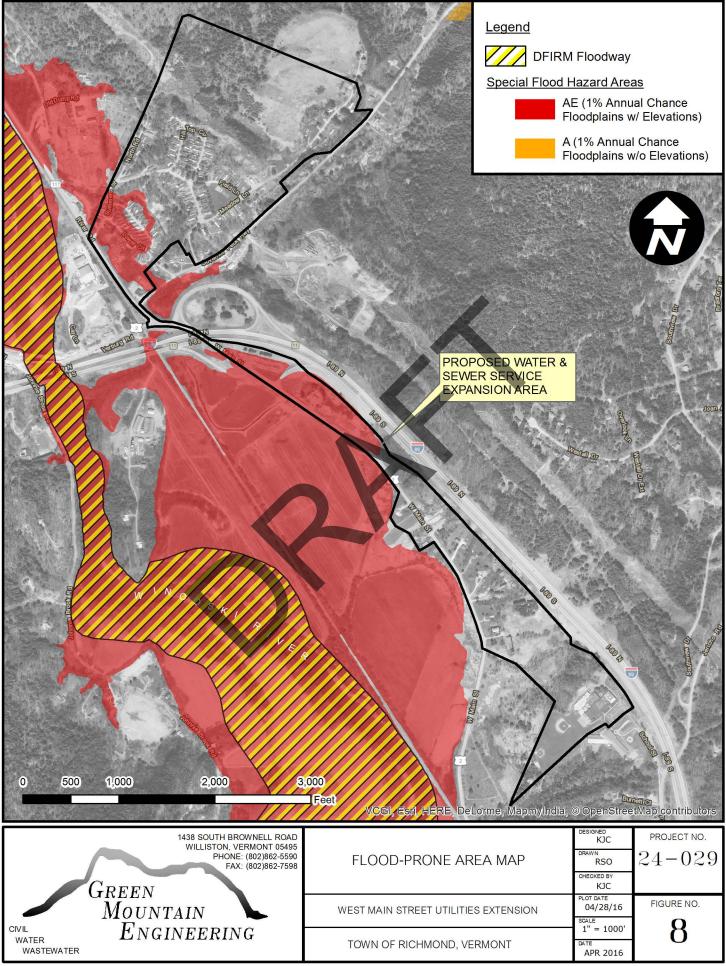
FILE: S:\GME Project Files\24-000\24-029 Richmond West Main St. Utilities Extension\Env. Report\GIS\FIGURE 5 - Wetlands Map.mxd

		SU DE LO DE		
AdA	Adams and Windsor loamy sands, 0 to 5 percent slopes	AdB TeE		
AdB	Adams and Windsor loamy sands, 5 to 12 percent slopes	BB (%)		
AdE	Adams and Windsor loamy sands, 30 to 60 percent slopes			
AgA	Agaw amfine sandy loam, 0 to 5 percent slopes Agaw amfine sandy loam, 30 to 60 percent slopes			
AgE An	Agaw annihe sandy loan, so to oo percent slopes	MyB	The states the states	
Au	Au Gres fine sandy loam			
BIB	Belgrade and Edridge soils, 3 to 8 percent slopes			
Br	Borrow pits			
DdA	Duane and Deerfield soils, 0 to 5 percent slopes		100	
DdC	Duane and Deerfield soils, 12 to 20 percent slopes			
Hf	Hadley very fine sandy loam		Мув	
HID	Hartland very fine sandy loam, 12 to 25 percent slopes		60	
HIE	Hartland very fine sandy loam, 25 to 60 percent slopes			
Le	Limerick silt loam	E C	TEE	
Lf	Limerick silt loam, very w et		MyB H	
LyD	Lyman-Marlow very rocky loams, 5 to 30 percent slopes	B C C C C C C C C C C C C C C C C C C C		
MuD	Munson and Belgrade silt loams, 12 to 25 percent slopes		AgA	
MyB	Munson and Raynham silt loams, 2 to 6 percent slopes	2		
MyC	Munson and Raynham silt loams, 6 to 12 percent slopes		MuD	
Sd	Scarboro loam		MyB Burnett of	
TeE	Terrace escarpments, silty and clayey	VCGI, Esri, HERE, DeLorme, MapmyIndi	the second s	
Wo	Winooski very fine sandy loam	VOGI, ESII, RERE, BELOITIE, Mapmyindi		
	1438 SOUTH BROWNELL ROAD WILLISTON, VERMONT 05495 PHONE: (802)862-5590 FAX: (802)862-7598 GREEN MOUNTAIN	SOIL SURVEY MAP	$\begin{array}{c c} \begin{array}{c} \begin{array}{c} \text{Designed} \\ \text{KJC} \end{array} \end{array} & \begin{array}{c} \text{PROJECT NO.} \\ \end{array} \\ \hline \\ \begin{array}{c} \text{Drawn} \\ \text{RSO} \end{array} \\ \hline \\ \begin{array}{c} \text{OHECKED BY} \\ \text{KJC} \end{array} \end{array} & \begin{array}{c} 24-029 \\ \end{array} \\ \hline \end{array} \\ \hline \end{array}$	
CIVIL	Mountain Engineering	WEST MAIN STREET UTILITIES EXTENSION	04/28/16 SCALE 1" = 1000'	
WATER WASTE	WATER TOWN OF RICHMOND, VERMONT DATE APR 2016			

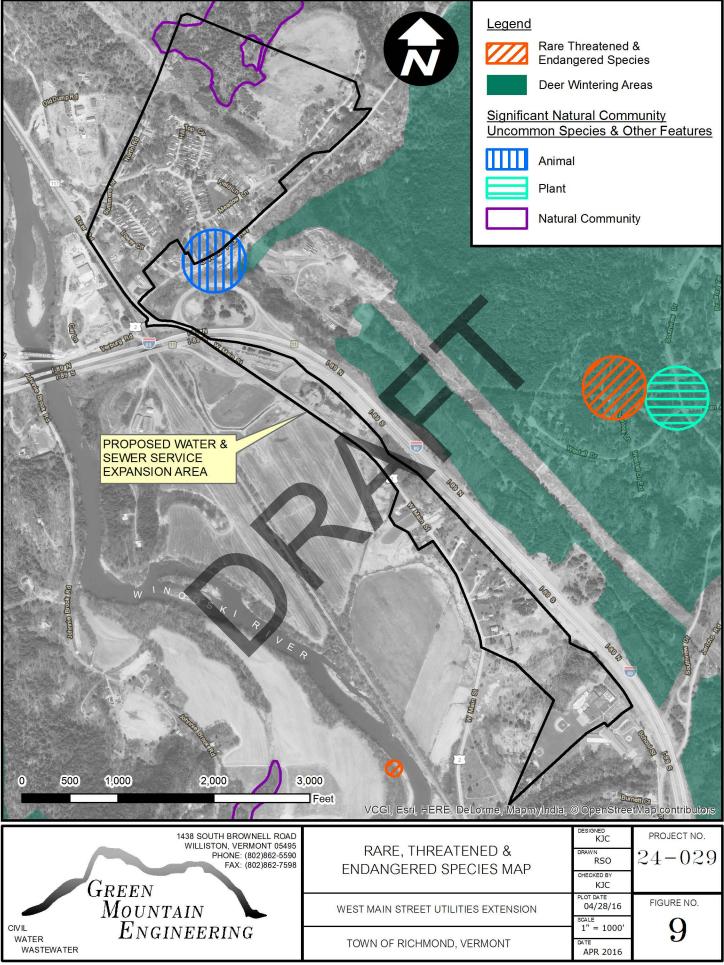
S:\GME Project Files\24-000\24-029 Richmond West Main St. Utilities Extension\Env. Report\GIS\FIGURE 6 - Soil Survey Map.mxd

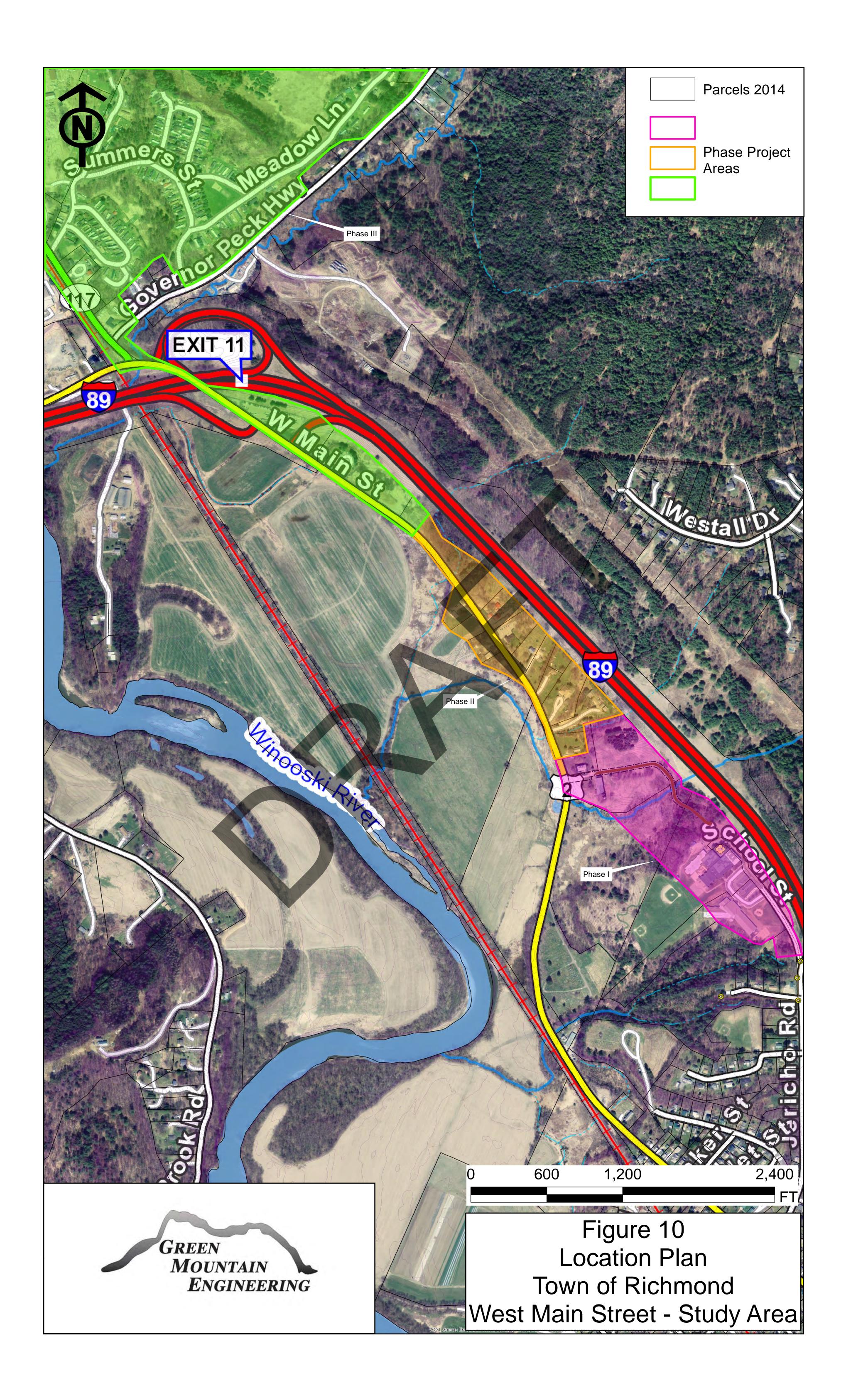


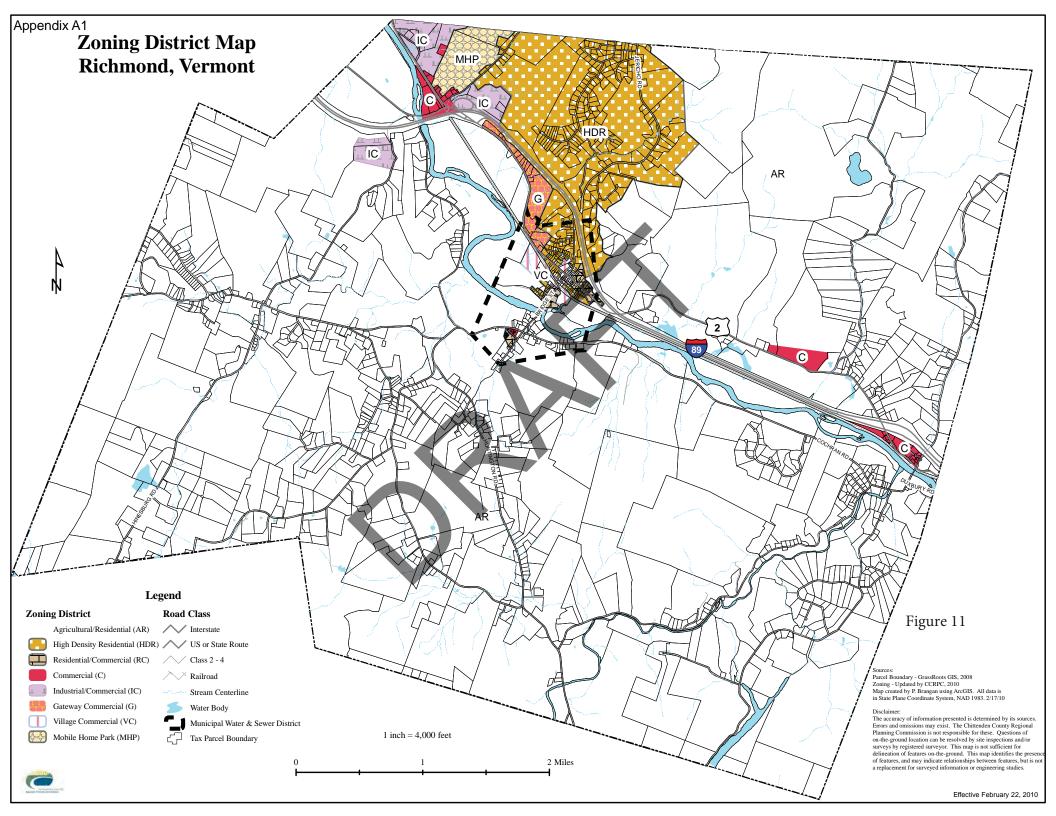
FILE: S:\GME Project Files\24-000\24-029 Richmond West Main St. Utilities Extension\Env. Report\GIS\FIGURE 7 - Prime Agricultural Soils Map.mxd

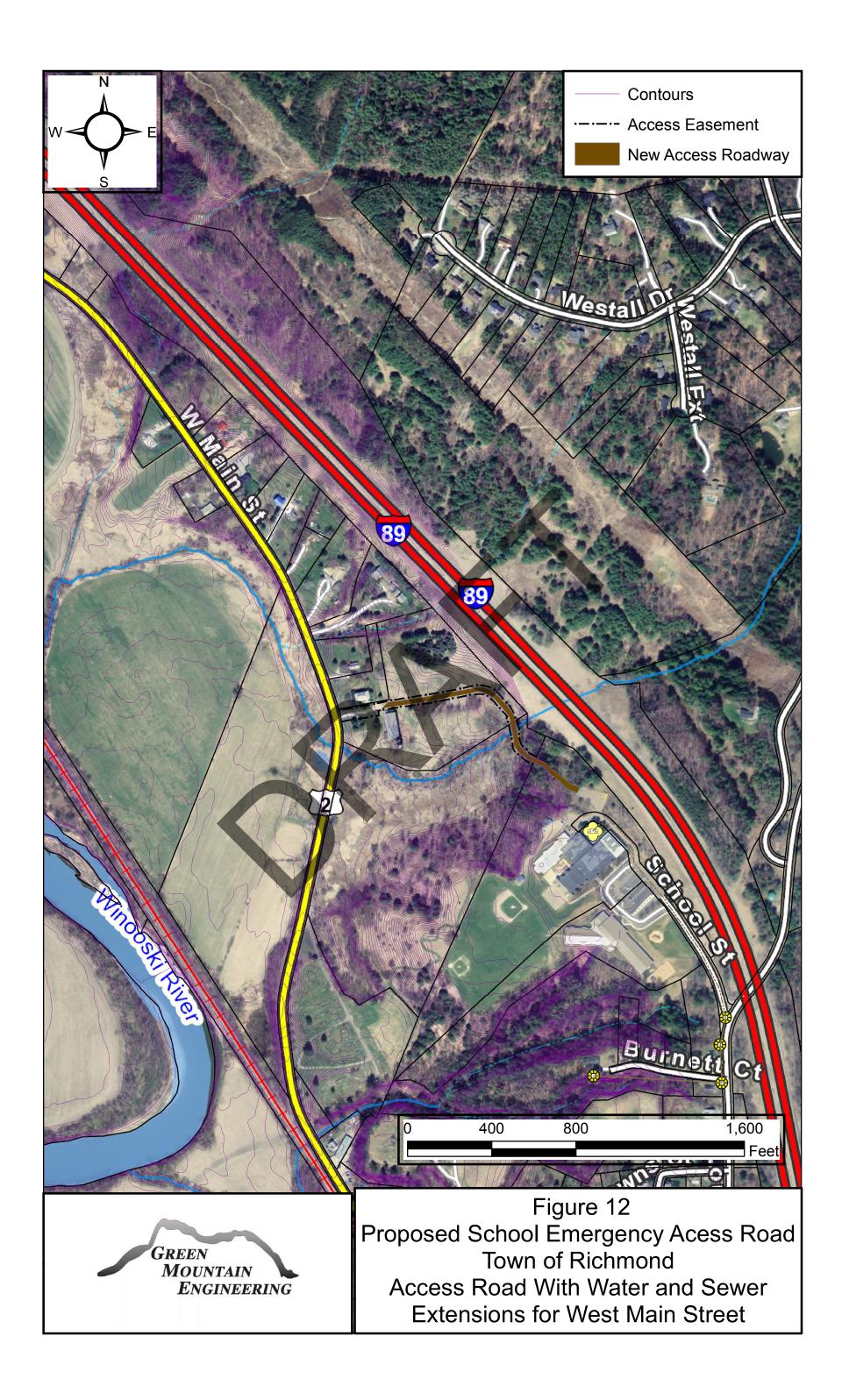


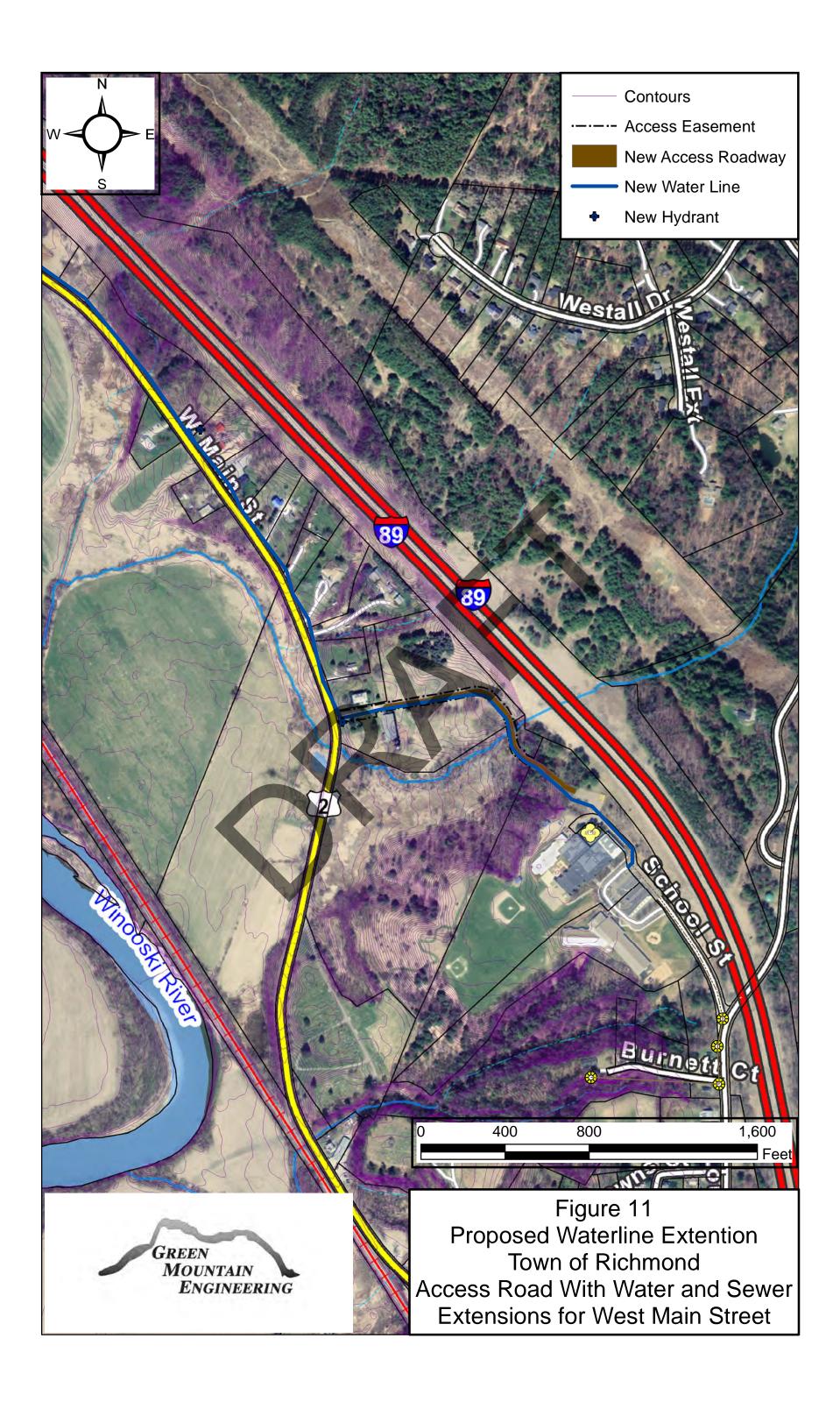
FILE: S:\GME Project Files\24-000\24-029	Richmond West Main St. Utilities Extension	Env. Report\GIS\FIGURE 8 - Floo	d-Prone Area Map.mxd

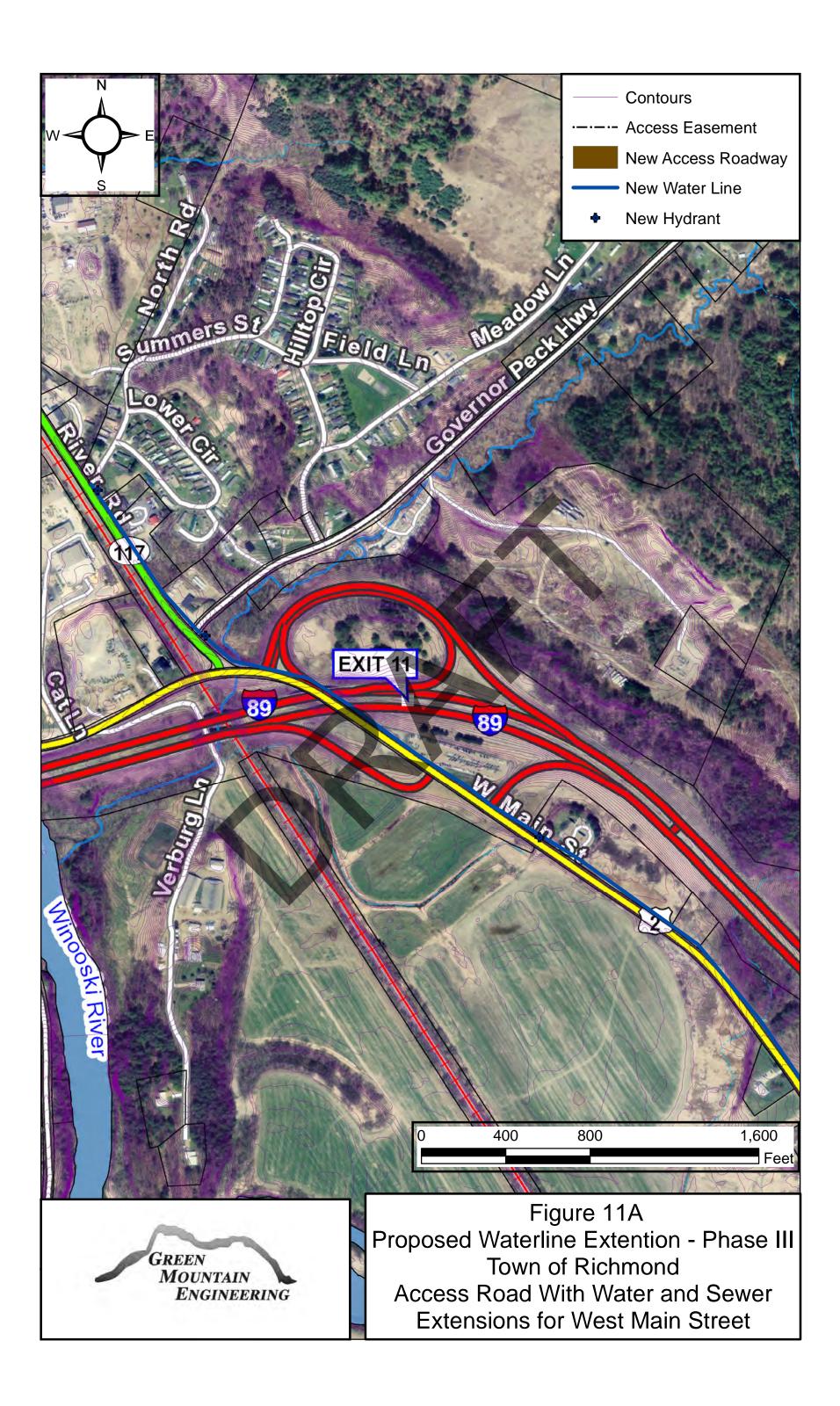


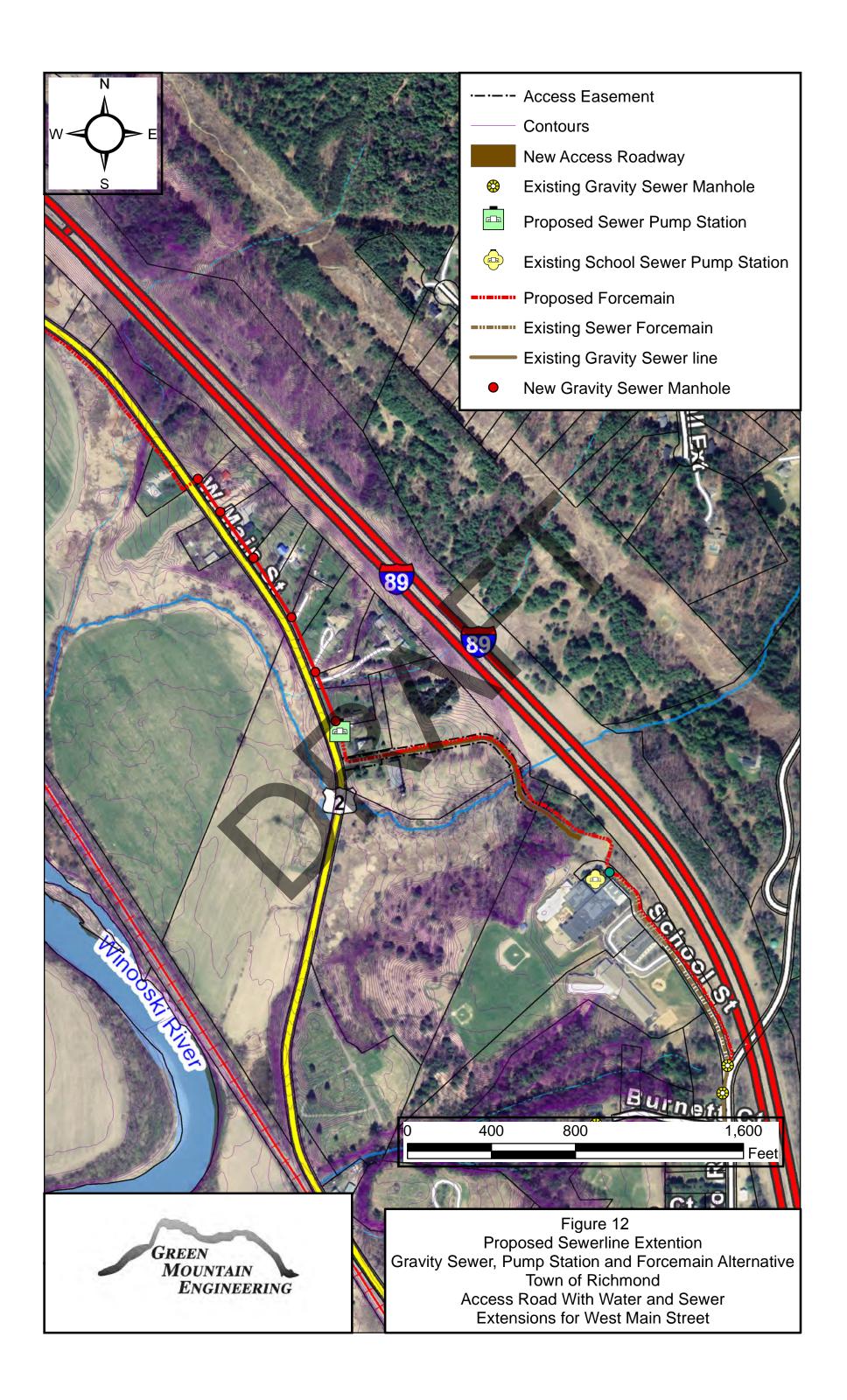


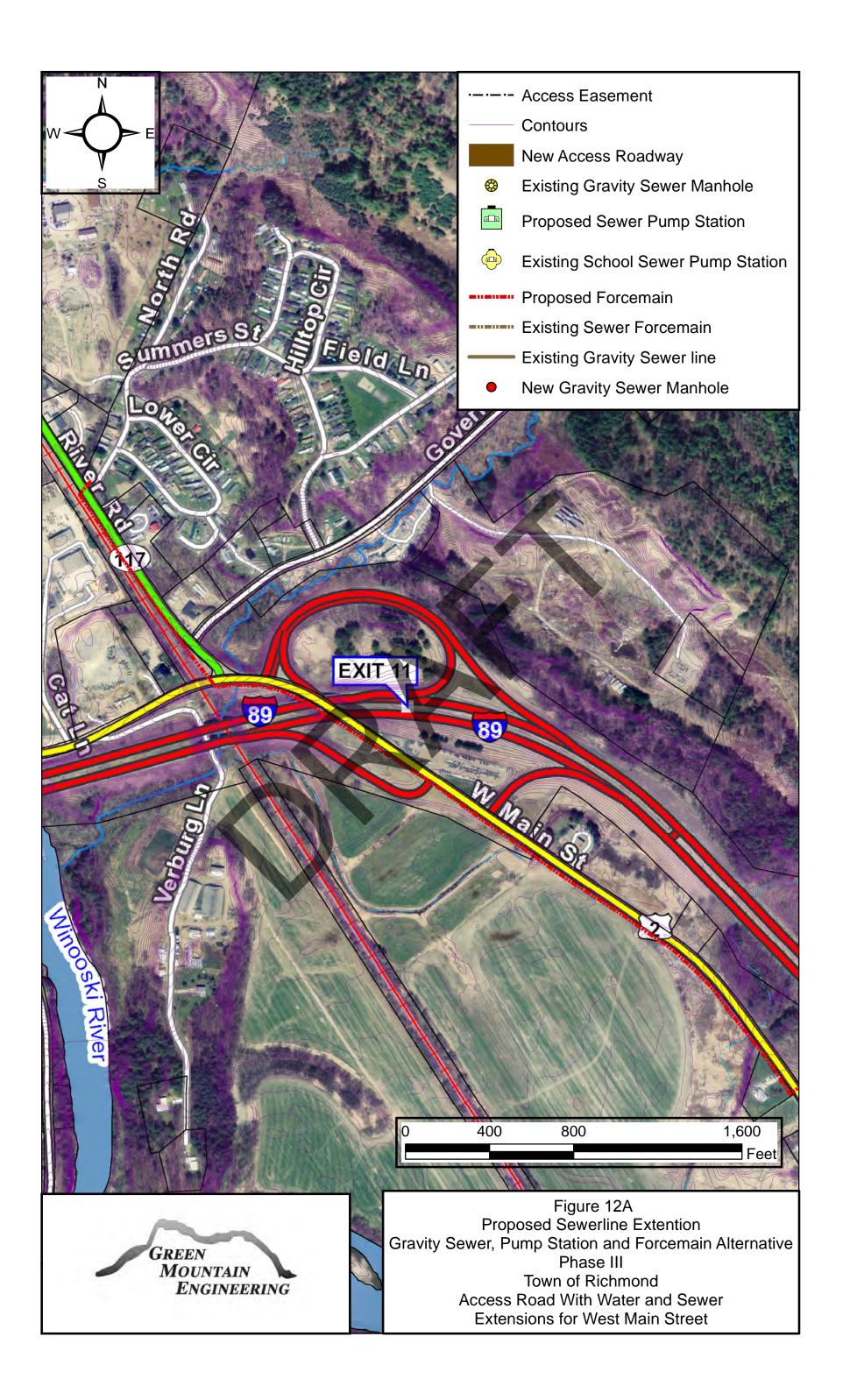




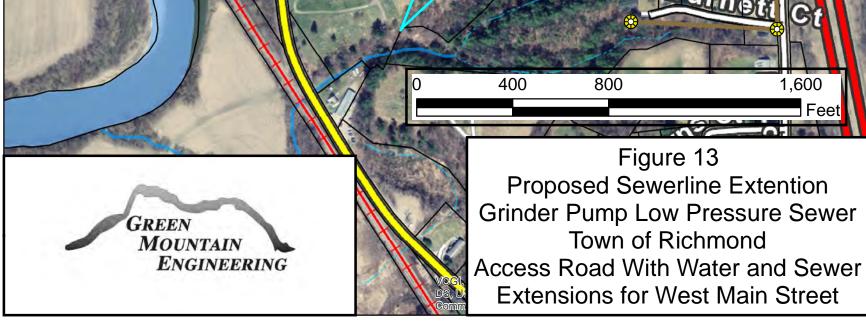


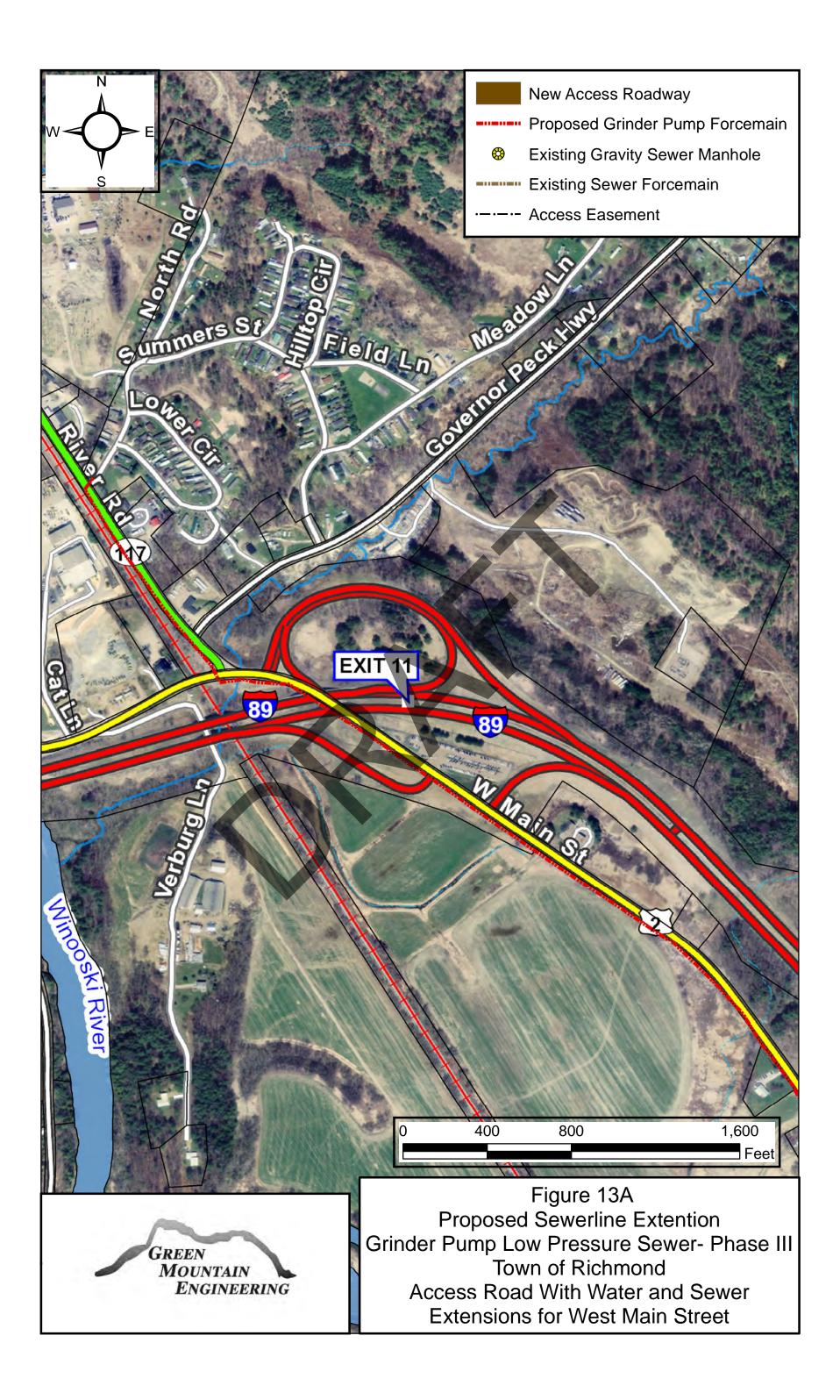














#### Richmond Water Resources Department PO Box 285, Richmond, VT 05477

Service Area Expansion Survey

- 1. (OPTIONAL)
  - Name: <u>Robert & Joy Reap</u> Address: <u>840 West Main St.</u> (mail PO Box 442) <u>Richmond, VT 05477</u>
- 2. <u>Type of User</u> (Check One)

	Single Family Residential (Seasonal	_ or Year Round	_)
-	Multi-Family Residential (Indicate number of units)		
V	Community I		

- X. Commercial
- \_\_\_\_ Agricultural \_\_\_\_ Other (Specify

3. What are your future plans for this property? (Check one)
\_\_\_\_\_ Single Family Residential (Seasonal \_\_\_\_\_ or Year Round \_\_\_\_\_

- \_\_\_\_ Multi-Family Residential (Indicate number of units \_\_\_\_
- <u>X</u> Commercial
- \_\_\_\_ Agricultural
- \_\_\_\_ Other (Specify
- \_\_\_\_ Unsure
- \_\_\_\_ None, it will stay as it is
- 3. Location

Place an X on the attached map to indicate your approximate location. This information will be used to determine where expansion is feasible. (If you do not know where to put the X, make sure your Richmond street address is above).

- 4. What is your present source of water? (Check all that apply)
  - X Drilled Well
  - Shallow Dug Well
  - \_\_\_\_ Cistern
  - \_\_\_\_ Bottled Water
  - \_\_\_\_ Other (explain) \_

\_\_\_\_ No water used at present (vacant lot for example)

- 5. What is your present form of wastewater disposal? (Check all that apply)
  - Leachfield
  - \_\_\_\_ Mound system
  - \_\_\_\_ Other (explain)
  - \_\_\_\_ No wastewater used at present (vacant lot for example)

6. Does your current wastewater disposal system limit your development potential? If so, how?

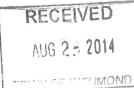
Yes, greany su. 7. Would you be willing to connect to the system by paying the required connection and allocation fees and becoming a paying permanent member of the system? X Yes No (Check One) If this questionnaire does not address your present or future needs, please explain, or 8. use this space to ask questions.

#### Richmond Water Resources Department PO Box 285, Richmond, VT 05477

#### Service Area Expansion Survey

1.	(OPTIONAL)
	Name: 1202 West Phone: 434-5751
	Address: 878 + 920 W. Main
2.	Type of User (Check One) 878
7. X	$\sqrt{\frac{1}{20}}$ Single Family Residential (Seasonal or Year Round $\sqrt{\frac{1}{20}}$ x $\frac{1}{20}$
V	Multi-Family Residential (Indicate number of units
	Commercial
	Agricultural Other (Specify)
3.	What are your future plans for this property? (Check one)
	Single Family Residential (Seasonal or Year Round)
	Multi-Family Residential (Indicate number of units) Commercial
	Agricultural
	Other (Specify)
	Unsure but lean toward commercial
	None, it will stay as it is
3.	Location
	2
	Place an <b>X</b> on the attached map to indicate your approximate location. This
	information will be used to determine where expansion is feasible. (If you do not know
	where to put the X, make sure your Richmond street address is above).
4.	What is your present source of water? (Check all that apply)
	Drilled Well Shallow Dug Well
	Cistern
	Bottled Water
	Other (explain)
	No water used at present (vacant lot for example)
5.	What is your present form of wastewater disposal? (Check all that apply)
	Leachfield RECE

Mound system Other (explain) No wastewater used at present (vacant lot for example)



6. Does your current wastewater disposal system limit your development potential? If so, how?

system 1 0 920 Groom 878 CANENTIANO mEnous 0 7. Would you be willing to connect to the system by paying the required connection and allocation fees and becoming a paying permanent member of the system? (Check One) \_\_\_\_\_ Yes \_\_\_\_\_ No If this questionnaire does not address your present or future needs, please explain, or 8. use this space to ask questions.

## Richmond Water Resources Department PO Box 285, Richmond, VT 05477

Service Area Expansion Survey

1. (OPTIONAL)

Name: Address:

tor Muntbrd

Phone: 434-2239

2. <u>Type of User</u> (Check One)

K Single Family Residential (Seasonal	or Year Round)
Multi-Family Residential (Indicate nur	nber of units

- $\chi$  Commercial
- \_\_\_\_ Agricultural
- \_\_\_\_ Other (Specify \_\_\_\_\_

3. What are your future plans for this property? (Check one) \_\_\_\_\_ Single Family Residential (Seasonal \_\_\_\_\_ or Year Round

- $\ltimes$  Multi-Family Residential (Indicate number of units 6
- $\underline{\dot{\mathcal{K}}}$  Commercial
- \_\_\_\_ Agricultural
- \_\_\_\_ Other (Specify
- \_\_\_\_ Unsure
- \_\_\_\_\_ None, it will stay as it is
- 3. Location

Place an X on the attached map to indicate your approximate location. This information will be used to determine where expansion is feasible. (If you do not know where to put the X, make sure your Richmond street address is above).

4. What is your present source of water? (Check all that apply)

932 W. MAIN S-

Drilled Well

- \_\_\_\_\_ Shallow Dug Well
- \_\_\_\_ Cistern
- \_\_\_\_ Bottled Water
- \_\_\_\_ Other (explain) \_

\_\_\_\_\_ No water used at present (vacant lot for example)

- 5. What is your present form of wastewater disposal? (Check all that apply)
  - \_\_\_\_ Leachfield
  - \_\_\_\_ Mound system
  - \_\_\_\_ Other (explain)
  - \_\_\_\_\_ No wastewater used at present (vacant lot for example)

Does your current wastewater disposal system limit your development potential? If so, 6. how?

there is not there is not 05

- Would you be willing to connect to the system by paying the required connection and 7. allocation fees and becoming a paying permanent member of the system? (Check One) Yes No
- If this questionnaire does not address your present or future needs, please explain, or 8. use this space to ask questions.

## Service Area Expansion Survey

1.	(OPTIONAL)
	Name: Address: Address: Address: Richmend 14 Phone: BOD 999 8185 Phone: BOD 999 8185
2.	<u>Type of User</u> (Check One)
	Single Family Residential (Seasonal or Year Round) // Multi-Family Residential (Indicate number of units) Commercial Agricultural Other (Specify
3.	What are your future plans for this property? (Check one) Single Family Residential (Seasonal or Year Round) _/Multi-Family Residential (Indicate number of units) _/_Commercial Agricultural Other (Specify) Unsure None, it will stay as it is
3.	Location Place an X on the attached map to indicate your approximate location. This information will be used to determine where expansion is feasible. (If you do not know where to put the X, make sure your Richmond street address is above).
4.	What is your present source of water? (Check all that apply)
	Shallow Dug Well

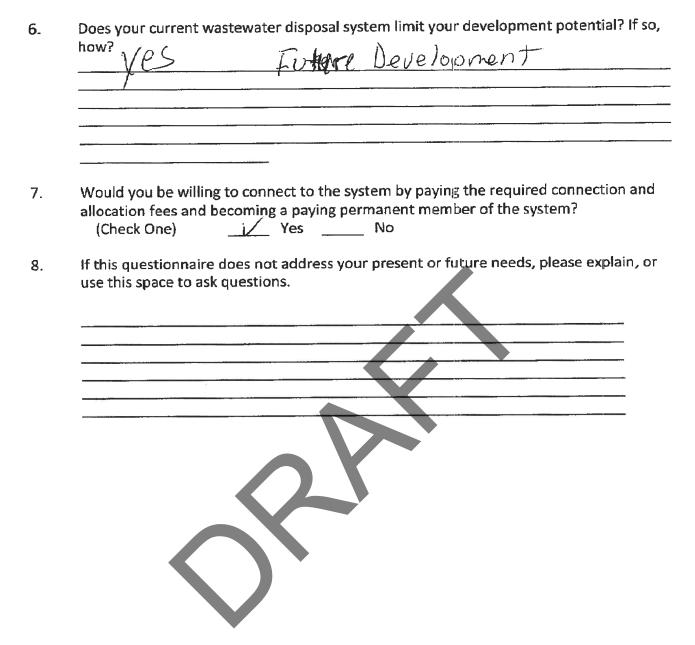
Cistern

- \_\_\_\_\_ Bottled Water
- \_\_\_\_\_ Other (explain) \_\_\_\_\_\_

\_\_\_\_\_ No water used at present (vacant lot for example)

5. What is your present form of wastewater disposal? (Check all that apply)

- \_\_\_\_ Mound system
- \_\_\_\_ Other (explain)
- \_\_\_\_\_ No wastewater used at present (vacant lot for example)



	ond Water Resources Department x 285, Richmond, VT 05477	RECEIVED
Servic	e Area Expansion Survey	
		TOWN OF RICHMOND
1.	(OPTIONAL) Chris Perron P1012 W Maih also shares the same interti His # is 999-8185	ms_
م بنام ا	HIS # is 999-8185	
	Name: Mike Stromme Phone: 802 Address: 1014 W Main St	-999-8502
2.	<u>Type of User</u> (Check One)	
	Single Family Residential (Seasonal or Year Round) Multi-Family Residential (Indicate number of units) Commercial Agricultural Other (Specify)	
3.	What are your future plans for this property? (Check one) Single Family Residential (Seasonal or Year Round) Multi-Family Residential (Indicate number of units) 3 , 3 Commercial Agricultural Other (Specify) Unsure None, it will stay as it is	-unit buildings
3.	Location Place an X on the attached map to indicate your approximate location. The information will be used to determine where expansion is feasible. (If your where to put the X, make sure your Richmond street address is above).	
4.	What is your present source of water? (Check all that apply)	
	<ul> <li>Drilled Well</li> <li>Shallow Dug Well</li> <li>Cistern</li> <li>Bottled Water</li> <li>Other (explain)</li> </ul>	
	No water used at present (vacant lot for example)	
5.	What is your present form of wastewater disposal? (Check all that apply)	
	_X Leachfield Mound system Other (explain)	

Other (explain)
No wastewater used at present (vacant lot for example)

6. Does your current wastewater disposal system limit your development potential? If so, how?

allows Yes Zaning ty per lacre with tor for I building per 1/3 50 town wastewate allows e . Would you be willing to connect to the system by paying the required connection and allocation fees and becoming a paying permanent member of the system? (Check One) X Yes No

8. If this questionnaire does not address your present or future needs, please explain, or use this space to ask questions.

7.

of limited World like to add housing Vesidential nookro with correct water Sewer

## Richmond Water Resources Department PO Box 285, Richmond, VT 05477

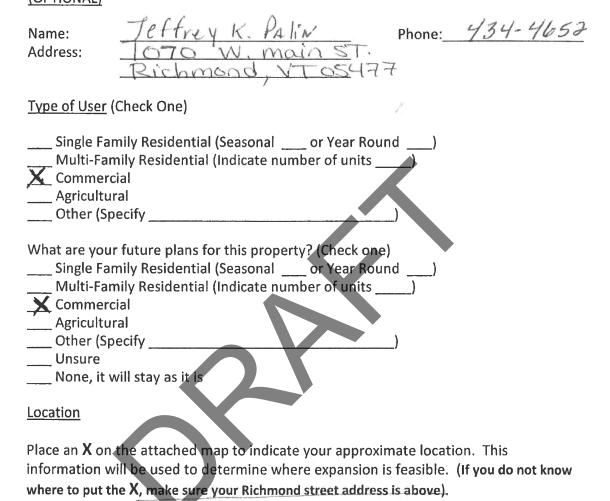
Service Area Expansion Survey

1. (OPTIONAL)

2.

3.

3.



- 4. What is your present source of water? (Check all that apply)
  - X Drilled Well
  - \_\_\_\_ Shallow Dug Well
  - \_\_\_\_ Cistern
  - \_\_\_\_ Bottled Water
  - \_\_\_\_ Other (explain) \_\_

\_\_\_\_ No water used at present (vacant lot for example)

5. What is your present form of wastewater disposal? (Check all that apply)

X	Lea	chf	Fiel	d

- \_\_\_\_ Mound system
- \_\_\_\_ Other (explain)
- \_\_\_\_\_ No wastewater used at present (vacant lot for example)

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TOWN OF RICHMOND

6. Does your current wastewater disposal system limit your development potential? If so, how?

SURC. C

- Would you be willing to connect to the system by paying the required connection and allocation fees and becoming a paying permanent member of the system?
   (Check One) Yes No
- 8. If this questionnaire does not address your present or future needs, please explain, or use this space to ask questions.

We see an asse community mo

## Richmond Water Resources Department PO Box 285, Richmond, VT 05477

Service Area Expansion Survey

- 1. (OPTIONAL) AMMY SCHEY Phone: 802 373-6546 Name: Address: 2. Type of User (Check One) \_ Single Family Residential (Seasonal \_\_\_\_ or Year Round \_ Multi-Family Residential (Indicate number of units \_ Commercial Agricultural Other (Specify 3. What are your future plans for this property? (Check one) \_\_\_\_ Single Family Residential (Seasonal \_\_\_\_\_ or Year Round \_\_\_\_\_ Multi-Family Residential (Indicate number of units \_ \_\_\_\_ Agricultural Other (Specify Unsure \_\_\_\_ None, it will stay as it is 3. Location Place an X on the attached map to indicate your approximate location. This information will be used to determine where expansion is feasible. (If you do not know where to put the X, make sure your Richmond street address is above). What is your present source of water? (Check all that apply) 4. Drilled Well Shallow Dug Well Cistern **Bottled Water** shared well on adjacent property \_\_\_\_ Other (explain) \_\_\_\_ No water used at present (vacant lot for example)
- 5. What is your present form of wastewater disposal? (Check all that apply)
  - Leachfield Mound system Other (explain) No wastewater used at present (vacant lot for example)

6. Does your current wastewater disposal system limit your development potential? If so, how? //

- Would you be willing to connect to the system by paying the required connection and allocation fees and becoming a paying permanent member of the system?
   (Check One)
   Yes \_\_\_\_\_ No
- 8. If this questionnaire does not address your present or future needs, please explain, or use this space to ask questions.



APPENDIX C DETAILED CONSTRUCTION COST ESTIMATES

		TA	BLE C-	1									
	OPINI	ON	OF PRO	<b>)BABL</b>	E								
CONSTRUCTION COST													
Town of Richmond													
West Main Street- Access Road with Water & Sewer Extensions													
	Scoping Study												
		Emerg	ency Access Ro	ad									
			ENR 9,750	ENR 9,750	ENR 9,800	ENR 10,000	ENR 10,200						
		Sale C	2014	2014	2015	2016	2017						
Description	Quantity	Units	Unit Price	Total Cost	Total Cost	Total Cost	Total Cost						
EMERGENCY ACCESS ROAD				<b></b>	<b>*</b> • • • • • •								
Clearing and Grubbing	1	Acre	\$10,000	\$10,000	\$10,051	\$10,256							
Common Fill Subgrade	27,900	CY	\$28	\$781,200	\$785,206		\$817,255						
90" CMP Culvert	150	EA.	\$350	\$52,500	\$52,769		\$54,923						
15" CMP Culvert	40	L.F.	\$80	\$3,200	\$3,216		\$3,348						
12" Gravel Subbase	400	C.Y.	\$28	\$11,200			\$11,717						
6" Fine Crushed Gravel Surface	200	C.Y.	\$30	\$6,000	\$6,031	\$6,154	\$6,277						
Geotextile Fabric	1,200	S.Y.	\$2	\$2,400	\$2,412	\$2,462	\$2,511						
Topsoil	800	C.Y.	\$25	\$20,000	\$20,103	\$20,513	\$20,923						
Seeding, Fertilizer and Liming	1.5	Acre	\$1,000	\$1,500	\$1,508	\$1,538	\$1,569						
Mulching	1.5	Acre	\$1,000	\$1,500	\$1,508	\$1,538	\$1,569						
Silt Fence	800	L.F.	\$4	\$2,800	\$2,814	\$2,872	\$2,929						
Rock Check Dams	6	EA.	\$175	\$1,050	\$1,055	\$1,077	\$1,098						
Site Prep and Miscellaneous (8%)	1	L.S.	\$71,468	\$71,468	\$71,835	\$73,301	\$74,767						
Contingency (10%)	1	L.S.	\$96,482	\$96,482	\$96,977	\$98,956	\$100,935						
Contractor's Bonds (2%)	1	L.S.	\$21,226	\$21,226	\$21,335	\$21,770							
		S	ubtotal	\$1,082,526	\$1,088,077	\$1,110,283	\$1,132,489						
			USE	\$1,083,000	\$1,089,000	\$1,111,000	\$1,133,000						

	TABLE C-2													
	CONS	TRI	JCTION	I COST	-									
			n of Richmond		3 4 5 E 8 5									
West	Main Street		load with Wate	r & Sowar Fyt	ansions									
Treat.	Main Street- 2		oping Study	I de Sewei Ext	clisiolis									
Pha	Phase I: Water Line Extension- School to West Main Street													
ENR 9,750   ENR 9,800   ENR 10,000   ENR 10,200														
	ENR 9,750 ENR 9,750 ENR 9,800 ENR 10,000 ENR 10,2 2014 2014 2015 2016 2017													
Description	Quantity	Units	Unit Price	Total Cost	Total Cost	<b>Total Cost</b>	Total Cost							
WATER SYSTEM														
8" Diameter PVC Water Main	1,800	L.F.	\$80	\$144,000	\$144,738	\$147,692	\$150,646							
8" x 8" Tapping Sleeve & Valve	1	EA.	\$3,000	\$3,000	\$3,015	\$3,077	\$3,138							
3/4" Diameter HDPE House Service	44	L.F.	\$42	\$1,848	\$1,857	\$1,895	\$1,933							
1" Diameter HDPE House Service	21	L.F.	\$50	\$1,050	\$1,055	\$1,077	\$1,098							
Existing Waterline Connections	1	EA.	\$2,500	\$2,500	\$2,513	\$2,564	\$2,615							
8" Gate Valve	1	EA.	\$1,300	\$1,300	\$1,307	\$1,333	\$1,360							
3/4" Curb Stops and Boxes	1	EA.	\$200	\$200	\$201	\$205	\$209							
3/4" Corporation Stops	1	EA.	\$300	\$300	\$302	\$308	\$314							
1" Curb Stops and Boxes	2	EA.	\$350	\$700	\$704	\$718	\$732							
1" Corporation Stops	2	EA.	\$350	\$700	\$704	\$718	\$732							
Fire Hydrant Branch Connection	1	EA.	\$4,250	\$4,250	\$4,272	\$4,359	\$4,446							
Rigid Trench Insulation	200	S.F.	\$2	\$400	\$402	\$410	\$418							
Class "B" Concrete	10	C.Y.	\$250	\$2,500	\$2,513	\$2,564	\$2,615							
Miscellaneous Extra Excavation	50	C.Y.	\$24	\$1,200	\$1,206	\$1,231	\$1,255							
Below-Grade Rock Removal (pipelines)	50	C.Y.	\$125	\$6,250	\$6,282	\$6,410	\$6,538							
Replacement of Unsuitable Material	50	C.Y.	\$32	\$1,600	\$1,608	\$1,641	\$1,674							
Boulder Excavation	10	C.Y.	\$50	\$500	\$503	\$513	\$523							
Erosion Control	1	L.S.	\$10,000	\$10,000	\$10,051	\$10,256	\$10,462							
Dust Control	2	Ton	\$500	\$1,000	\$1,005	\$1,026	\$1,046							
Permanent Trench Pavement Repair	10	S.Y.	\$60	\$600	\$603	\$615	\$628							
Traffic Control	0	L.Ś.	\$0	\$0	\$0									
Site Prep and Miscellaneous (8%)		L.S.	\$14,712	\$14,712	\$14,787		· · ·							
Contingency (10%)	1	L.S.	\$19,861	\$19,861	\$19,963									
Contractor's Bonds (2%)	1	L.S.	\$4,369	\$4,369	\$4,392		\$4,571							
			Subtotal	\$222,840	\$223,983	\$228,554	\$233,125							
			USE	\$223,000	\$224,000	\$229,000	\$234,000							

		TA	BLE C-3	3									
0	PINIC	DN (	<b>OF PRO</b>	BABL	E								
	CONS	TRI	UCTION	I COST									
	0110		n of Richmond										
West M	lain Street.		Road with Wate	r & Sewer Fyt	ensions								
Scoping Study													
Phase II Water Line Extension- Reap Property to Gateway District Border													
ENR 9,750   ENR 9,750   ENR 9,800   ENR 10,000   ENR 10,200													
			2014	2014	2015	2016	2017						
Description	Quantity	Units	Unit Price	Total Cost	<b>Total Cost</b>	Total Cost	<b>Total Cost</b>						
WATER SYSTEM													
8" Diameter PVC Water Main	1,600	L.F.	\$80	\$128,000	\$128,656	\$131,282	\$133,908						
20" Jack & Bore Steel Sleeve	50	L.F.	\$800	\$40,000	\$40,205	\$41,026	\$41,846						
3/4" Diameter HDPE House Service Boring	308	L.F.	\$42	\$12,936	\$13,002	\$13,268	\$13,533						
1" Diameter HDPE House Service Boring	220	L.F.	\$50	\$11,000	\$11,056	\$11,282	\$11,508						
Existing Waterline Connections	0	EA.	\$2,500	\$0	\$0	\$0	\$0						
8" Gate Valve	1	EA.	\$1,300	\$1,300	\$1,307	\$1,333	\$1,360						
3/4" Curb Stops and Boxes	7	EA.	\$200	\$1,400	\$1,407	\$1,436	\$1,465						
3/4" Corporation Stops	7	EA.	\$300	\$2,100	\$2,111	\$2,154	\$2,197						
1" Curb Stops and Boxes	5	EA.	\$350	\$1,750	\$1,759	\$1,795	\$1,831						
1" Corporation Stops	5	EA.	\$350	\$1,750	\$1,759	\$1,795	\$1,831						
Fire Hydrant Branch Connection	1	EA.	\$4,250	\$4,250	\$4,272	\$4,359	\$4,446						
Rigid Trench Insulation	200	S.F.	\$2	\$400	\$402	\$410	\$418						
Class "B" Concrete	10	C.Y.	\$250	\$2,500	\$2,513	\$2,564	\$2,615						
Miscellaneous Extra Excavation	50	C.Y. <sup>1</sup>	\$24	\$1,200	\$1,206	\$1,231	\$1,255						
Below-Grade Rock Removal (pipelines)	50	C.Y.	\$125	\$6,250	\$6,282	\$6,410	\$6,538						
Replacement of Unsuitable Material	50	C.Y.	\$32	\$1,600	\$1,608	\$1,641	\$1,674						
Boulder Excavation	10	C.Y.	\$50	\$500	\$503	\$513	\$523						
Erosion Control	1	L.S.	\$10,000	\$10,000	\$10,051	\$10,256	· ·						
Dust Control	2	Ton_	\$500	\$1,000	\$1,005	\$1,026	\$1,046						
Permanent Trench Pavement Repair	0	S.Y.	\$60	\$0	\$0								
Traffic Control		L.S.	\$10,000	\$10,000	\$10,051								
Site Prep and Miscellaneous (8%)		L.S.	\$19,035	\$19,035	\$19,132								
Contingency (10%)		L.S.	\$25,697	\$25,697	\$25,829	\$26,356							
Contractor's Bonds (2%)		L.S.	\$5,653	\$5,653	\$5,682	\$5,798							
			Subtotal	\$288,321	\$289,800	\$295,714	\$301,628						
Notes:			USE	\$289,000	\$290,000	\$296,000	\$302,000						

		TAI	BLE C-4	4								
0	PINIC	DN (	<b>DF PRO</b>	BABLI	E							
	CONG	TDI	JCTION	J COST	-							
	LOND			1 CO21								
			of Richmond									
West N	lain Street-		load with Wate	r & Sewer Ext	ensions							
Dhose III Wete	n I in e Ersten		oping Study	Dandan ta Mal	de Henre Desi							
Phase III Water Line Extension- Gateway District Border to Mobile Home Park ENR 9,750   ENR 9,750   ENR 9,800   ENR 10,000   ENR 10,200												
ENR 9,750 ENR 9,750 ENR 9,800 ENR 10,000 ENR 10,200 2014 2014 2015 2016 2017												
Description	Quantity	Units	Unit Price	Total Cost	Total Cost	Total Cost	Total Cost					
WATER SYSTEM	Quantity	CHIts	Unit Frite	I Utal COSt	Total Cost	Total Cost	I otal Cost					
8" Diameter PVC Water Main	900	L.F.	\$80	\$72,000	\$72,369	\$73,846	\$75,323					
10" Diameter PVC Water Main	3,600	L.F.	\$90	\$324,000	\$325,662	\$332,308	\$338,954					
20" Jack & Bore Steel Sleeve	150	L.F.	\$800	\$120,000	\$120,615	\$123,077	\$125,538					
3/4" Diameter HDPE House Service Boring	20	L.F.	\$42	\$840	\$844	\$862	\$879					
1" Diameter HDPE House Service Boring	10	L.F.	\$50	\$500	\$503	\$513	\$523					
Existing Waterline Connections	1	EA.	\$2,500	\$2,500	\$2,513	\$2,564	\$2,615					
8" Gate Valve	3	EA.	\$1,300	\$3,900	\$3,920	\$4,000	\$4,080					
10" Gate Valve	1	EA.	\$2,000	\$2,000	\$2,010	\$2,051	\$2,092					
3/4" Curb Stops and Boxes	2	EA.	\$200	\$400	\$402	\$410	\$418					
3/4" Corporation Stops	2	EA.	\$300	\$600	\$603	\$615	\$628					
1" Curb Stops and Boxes	1	EA.	\$350	\$350	\$352	\$359	\$36					
1" Corporation Stops	1	EA.	\$350	\$350	\$352	\$359	\$36					
Meter Pit	1	EA.	\$10,000	\$10,000	\$10,051	\$10,256	\$10,462					
Fire Hydrant Branch Connection	3	EA.	\$4,250	\$12,750	\$12,815	\$13,077	\$13,33					
Rigid Trench Insulation	800	S.F.	\$2	\$1,600	\$1,608	\$1,641	\$1,674					
Class "B" Concrete	10	C.Y.	\$250	\$2,500	\$2,513							
Miscellaneous Extra Excavation	50	C.Y.	\$24	\$1,200	\$1,206	\$1,231	\$1,25					
Below-Grade Rock Removal (pipelines)	50	Ċ.Y.	\$125	\$6,250	\$6,282	\$6,410						
Replacement of Unsuitable Material	50	C.Y.	\$32	\$1,600	\$1,608	\$1,641						
Boulder Excavation	10	C.Y.	\$50	\$500	\$503	\$513	\$52					
Erosion Control		L.S.	\$10,000	\$10,000	\$10,051	\$10,256						
Dust Control Permanent Trench Pavement Repair	2	Ton S.Y.	\$500 \$60	\$1,000 \$0	\$1,005 \$0	\$1,026						
Traffic Control		L.S.	\$60 \$10.000	\$0 \$10,000	\$0 \$10,051	\$0 \$10,256						
Site Prep and Miscellaneous (8%)	1	L.S. L.S.	\$10,000 \$46,787	\$10,000 \$46,787	\$10,051							
Contingency (10%)	1	L.S. L.S.	\$63,163	\$63,163	\$47,027 \$63,487	\$64,782						
Contractor's Bonds (2%)	1	L.S. L.S.	\$13,896	\$13,896	\$13,967	\$14,252	\$14,53					
			ubtotal	\$708,686	\$712,320	\$726,857	\$741,394					
			USE	\$709,000	\$713,000	\$727,000	\$742,000					

		TA	BLE C-:	5			
(	<b>PINIC</b>	DN (	OF PRO	BABL	E		
	CONS	TRI	UCTION	I COST	-		
	COID		n of Richmond	10051			
West	Main Street-		Road with Wate	r & Sewer Ext	ensions		
			oping Study				
Sewe	r Pump Statio	on & 4"	Forcemain- Sch	ool to Reap Pr	operty		
			ENR 9,750	ENR 9,750	ENR 9,800	ENR 10,000	ENR 10,200
Description	Ourseliter	Tinthe	2014 Unit Price	2014	2015	2016	2017
Description WASTEWATER SYSTEM	Quantity	Units	Unit Price	Total Cost	Total Cost	Total Cost	Total Cost
4" Diameter HDPE Forcemain	1,500	L.F.	\$50	\$75,000	\$75,385	\$76,923	\$78,462
5' Diameter Air Release Manhole	1,500	EA.	\$8,000	\$8,000	\$8,041	\$8,205	\$8,369
5' Diameter Valve Manhole	1	EA.	\$10,000	\$10,000	\$10,051		
Rigid Trench Insulation	200	S.F.	\$2	\$400	\$402		
Class "B" Concrete	10	C.Y.	\$250	\$2,500	\$2,513	\$2,564	\$2,615
Miscellaneous Extra Excavation	50	C.Y.	\$24	\$1,200	\$1,206		\$1,255
Below-Grade Rock Removal (pipelines)	50	C.Y.	\$125	\$6,250	\$6,282	\$6,410	\$6,538
Replacement of Unsuitable Material	50	C.Y.	\$32	\$1,600	\$1,608	\$1,641	\$1,674
Boulder Excavation	10	C.Y.	\$50	\$500	\$503	\$513	\$523
Erosion Control	1	L.S.	\$10,000	\$10,000	\$10,051	\$10,256	\$10,462
Dust Control	2	Ton	\$500	\$1,000	\$1,005	\$1,026	\$1,046
Permanent Trench Pavement Repair	10	S.Y.	\$60	\$600	\$603	\$615	\$628
Traffic Control	1	L.S.	\$10,000	\$10,000	\$10,051	\$10,256	\$10,462
Pump Station	1	L.S.	\$200,000	\$200,000	\$201,026		
Site Prep and Miscellaneous (8%)	1	L.S.	\$10,164	\$10,164	\$10,216		\$10,633
Contingency (10%)	1	L.S.	\$33,721	\$33,721	\$33,894		\$35,278
Contractor's Bonds (2%)	1	L.S.	\$7,419	\$7,419	\$7,457		\$7,761
			Subtotal	\$378,354	\$380,294	\$388,055	\$395,817
			USE	\$379,000	\$381,000	\$389,000	\$396,000

		TAI	BLE C-	6									
(	<b>PINIO</b>	DN C	)F PRO	BABLI	E								
	CONS	TRI	JCTION	I COST	7								
			of Richmond				19 19 3 1 1						
West	Main Street-	Access R	oad with Wate	r & Sewer Ext	ensions								
			oping Study										
8" Gravity Sewer- Reap Property to Gateway District Boarder													
			ENR 9,750	ENR 9,750	ENR 9,800	ENR 10,000	ENR 10,200						
Description	0	FT. **	2014	2014	2015	2016	2017						
Description WASTEWATER SYSTEM	Quantity	Units	Unit Price	Total Cost	Total Cost	Total Cost	Total Cost						
8" PVC Gravity Sewer	1,600	L.F.	\$60	\$96,000	\$96,492	\$98,462	\$100,431						
4' Diameter Manholes	6	EA.	\$3,500	\$21,000	\$21,108	\$21,538	\$21,969						
8" x 4" Service Wye	7	EA.	\$100	\$700	\$704		\$732						
8" x 6" Sewervice Wye	7	EA.	\$150	\$1,050	\$1,055		\$1,098						
4" Sewer Service	70	L.F.	\$50	\$3,500	\$3,518								
6" Sewer Service	70	L.F.	\$53	\$3,710	\$3,729		\$3,881						
Class "B" Concrete	10	C.Y.	\$250	\$2,500	\$2,513		\$2,615						
Miscellaneous Extra Excavation	50	C.Y.	\$24	\$1,200	\$1,206		\$1,255						
Below-Grade Rock Removal (pipelines)	50	C.Y.	\$125	\$6,250	\$6,282								
Replacement of Unsuitable Material	50	C.Y.	\$32	\$1,600	\$1,608	\$1,641	\$1,674						
Boulder Excavation	10	C.Y.	\$50	\$500	\$503	\$513	\$523						
Erosion Control	1	L.S.	\$10,000	\$10,000	\$10,051	\$10,256	\$10,462						
Dust Control	2	Ton	\$500	\$1,000	\$1,005	\$1,026	\$1,046						
Permanent Trench Pavement Repair	20	S.Y.	\$60	\$1,200	\$1,206	\$1,231	\$1,255						
Traffic Control	1	L.S.	\$10,000	\$10,000	\$10,051	\$10,256	\$10,462						
Site Prep and Miscellaneous (8%)	1	L.S.	\$12,817	\$12,817	\$12,883	\$13,145	\$13,408						
Contingency (10%)	1	L.S.	\$17,303	\$17,303	\$17,391	\$17,746	\$18,101						
Contractor's Bonds (2%)	1	L.S.	\$3,807	\$3,807	\$3,826	\$3,904	\$3,982						
		S	ubtotal	\$194,136	\$195,132	\$199,114	\$203,096						
			USE	\$195,000	\$196,000	\$200,000	\$204,000						

Notes:
1. The construction cost estimates are based on preliminary phase estimates only. More detailed costs shall be developed during Final Design Phase Engineering based on actual design quantities.
2. ENR= Engineering News Record Construction Cost Index.

		TA	BLE C-'	7			
(	<b>PINIC</b>	DN	OF PRO	BABL	E		an an Tar
	CONS	TR	UCTION	I COST	7		
			of Richmond				
West	Main Street-	Access	Road with Wate	r & Sewer Ext	ensions		
		S	coping Study				
Phase I: 3" I	low Pressure	Grinde	r Pump Forcema	ain- School to l	<b>Reap Property</b>		
			ENR 9,750	ENR 9,750	ENR 9,800	ENR 10,000	ENR 10,200
			2014	2014	2015	2016	2017
Description	Quantity	Units	Unit Price	Total Cost	Total Cost	Total Cost	Total Cost
WASTEWATER SYSTEM			<b>6</b> 4 <b>8</b>				
3" HDPE LPS	2,600	L.F.	\$42	\$109,200	\$109,760		
5' Diameter Air Release Manhole	1	EA.	\$8,000	\$8,000	\$8,041		
1 1/2" LPS Service	63	L.F.	\$35	\$2,205	\$2,216		
1 1/2" Curb Stops and Boxes	3	EA.	\$250	\$750	\$754		
Core Existing Manhole	1	L.S.	\$1,500	\$1,500	\$1,508		
Class "B" Concrete	10	C.Y.	\$250	\$2,500	\$2,513		
Miscellaneous Extra Excavation	50	C.Y. C.Y.	\$24	\$1,200 \$6,250	\$1,206		
Below-Grade Rock Removal (pipelines) Replacement of Unsuitable Material	50 50	C.Y.	\$125 \$32	\$6,250 \$1,600	\$6,282 \$1,608		
Boulder Excavation	30 10	C.Y.	\$52	\$1,000	\$1,808		
Erosion Control	10	L.S.	\$2,500	\$2,500	\$2,513		
Dust Control	1	Ton	\$500	\$2,500 \$250	\$2,515	\$2,504.10	
Permanent Trench Pavement Repair	20	S.Y.	\$60	\$1,200	\$1,206	+	+
Traffic Control	1	L.S.	\$2,000	\$2,000	\$2,010		
Site Prep and Miscellaneous (8%)	1	L.S.	\$11,172	\$11,172	\$11,230		
Contingency (10%)	1	L.S.	\$15,083	\$15,083	\$15,160		
Contractor's Bonds (2%)	1	L.S.	\$3,318	\$3,318	\$3,335		
			Subtotal	\$169,228	\$170,096	\$173,568	\$177,039
			USE	\$170,000	\$171,000	\$174,000	\$178,000

		TA	BLE C-	8			
(	<b>PINIC</b>	DN	OF PRO	BABLI	E		
	CONS	TR	UCTION	I COST	T		
	COND		vn of Richmond				
West	Main Street-		Road with Wate	r & Sewer Ext	ensions		
			coping Study				
Phase II: 3" Low Press	ure Grinder P			Property to Ga	teway District	Boundary	
			ENR 9,750	ENR 9,750	ENR 9,800	ENR 10,000	ENR 10,200
			2014	2014	2015	2016	2017
Description	Quantity	Units	Unit Price	Total Cost	Total Cost	Total Cost	Total Cost
WASTEWATER SYSTEM							
3" HDPE LPS	1,600	L.F.	\$42	\$67,200	\$67,545	\$68,923.08	\$70,301.54
5' Diameter Air Release Manhole	1	EA.	\$8,000	\$8,000	\$8,041	\$8,205.13	\$8,369.23
5' Diameter Cleanout Manhole	1	EA.	\$8,000	\$8,000	\$8,041	\$8,205.13	\$8,369.23
1 1/2" LPS Service	110	L.F.	\$35	\$3,850	\$3,870	\$3,948.72	\$4,027.69
1 1/2" Curb Stops and Boxes	11	EA.	\$250	\$2,750	\$2,764	\$2,820.51	\$2,876.92
Class "B" Concrete	10	C.Y.	\$250	\$2,500	\$2,513	\$2,564.10	
Miscellaneous Extra Excavation	50	C.Y.	\$24	\$1,200	\$1,206		\$1,255.38
Below-Grade Rock Removal (pipelines)	50	C.Y.	\$125	\$6,250	\$6,282	\$6,410.26	\$6,538.40
Replacement of Unsuitable Material	50	C.Y.		\$1,600	\$1,608	\$1,641.03	
Boulder Excavation	10	C.Y.	\$50	\$500	\$503	\$512.82	
Erosion Control	1	L.S.	\$2,500	\$2,500	\$2,513		\$2,615.38
Dust Control	1	Ton	\$500	\$250	\$251	\$256.41	\$261.54
Permanent Trench Pavement Repair	0	S.Y.	\$60	\$0	\$0	\$0.00	\$0.00
Traffic Control	1	L.S.	\$10,000	\$10,000	\$10,051	\$10,256.41	· ·
Site Prep and Miscellaneous (8%)	1	L.S.	\$9,168	\$9,168	\$9,215	\$9,403.08	\$9,591.14
Contingency (10%)	1	L.S.	\$12,377	\$12,377	\$12,440		\$12,948.04
Contractor's Bonds (2%)	1	L.S.	\$2,723	\$2,723	\$2,737		\$2,848.5
			Subtotal	\$138,868	\$139,580	\$142,428	\$145,277
			USE	\$139,000	\$140,000	\$143,000	\$146,000

TABLE C-9									
	OPINI	ON C	OF PRO	BABLE					
CONSTRUCTION COST									
Town of Richmond									
W	est Main Stree	t- Access R	oad with Water	& Sewer Exten	isions				
			oping Study						
Phase 3- 3" Gateway District Boundary to River View Commons Mobile Home Park									
			ENR 9,750 2014	ENR 9,750 2014	ENR 9,800 2015	ENR 10,000 2016	ENR 10,200 2017		
Description	Ouantity	Units	Unit Price	Total Cost	Total Cost	Total Cost	Total Cost		
WASTEWATER SYSTEM	Quantity	Units	Outerine	I Utal COSt	Total Cost	Total Cost	Total Cost		
3" HDPE LPS	4,150	L.F.	\$42	\$174,300	\$175,194	\$178,769	\$182,345		
16" Jack & Bore Steel Sleeves	350	L.F.	\$500	\$175,000	\$175,897		\$183,077		
5' Diameter Air Release Manhole	2	EA.	\$8,000	\$16,000	\$16,082	\$16,410	\$16,738		
5' Diameter Cleanout Manhole	2	EA.	\$8,000	\$16,000	\$16,082	\$16,410	\$16,738		
3" LPS Service	30	L.F.	\$42	\$1,260	\$1,266	\$1,292	\$1,318		
3" Curb Stops and Boxes	1	EA.	\$500	\$500	\$503	\$513	\$523		
1 1/2" LPS Service	30	L.F.	\$35	\$1,050	\$1,055	\$1,077	\$1,098		
1 1/2" Curb Stops and Boxes	3	EA.	\$250	\$750	\$754	\$769	\$785		
Class "B" Concrete	10	C.Y.	\$250	\$2,500	\$2,513	\$2,564	\$2,615		
Miscellaneous Extra Excavation	50	C.Y.	\$24	\$1,200	\$1,206	\$1,231	\$1,255		
Below-Grade Rock Removal (pipelines)	50	C.Y.	\$125	\$6,250	\$6,282	\$6,410	\$6,538		
Replacement of Unsuitable Material	50	C.Y.	\$32	\$1,600	\$1,608	\$1,641	\$1,674		
Boulder Excavation	10	C.Y.	\$50	\$500	\$503	\$513	\$523		
Erosion Control	1	L.S.	\$2,500	\$2,500	\$2,513		\$2,615		
Dust Control	5	Ton	\$500	\$2,500	\$2,513		\$2,615		
Permanent Trench Pavement Repair	0	S.Y.	\$60	\$0	\$0		\$0		
Traffic Control	1	L.S.	\$30,000	- ·	\$30,154				
Site Prep and Miscellaneous (8%)	1	L.S.	\$34,553	\$34,553	\$34,730				
Contingency (10%)	1	Ĺ.S.	\$46,646	\$46,646	\$46,885	\$47,842	\$48,799		
Contractor's Bonds (2%)	1	L.S.	\$10,262	\$10,262	\$10,315	· · · · · · · · · · · · · · · · · · ·	\$10,736		
			ibtotal	\$523,371	\$526,055	\$536,791	\$547,527		
			USE	\$524,000	\$527,000	\$537,000	\$548,000		

TABLE C-10										
<b>OPINION OF PROBABLE</b>										
CONSTRUCTION COST										
Town of Richmond										
West Main Street- Access Road with Water & Sewer Extensions										
Scoping Study										
Upgraded Middle School Pump Station (Wet Well and Emergency Storage)										
			ENR 9,750	ENR 9,750	ENR 9,800	ENR 10,000	ENR 10,200			
Description	Ouantity	Units	2014 Unit Price	2014 Total Cost	2015 Total Cost	2016 Total Cost	2017 Total Cost			
EMERGENCY ACCESS ROAD	Quantity	Units	Cuntifice	10tal Cost	Total Cost	1 Otal Cost	I Utal COSt			
Demo Existing 4' Diameter Wetwell	1	LS	\$4,000	\$4,000	\$4,021	\$4,103	\$4,185			
New 8' Diameter Wet Well	1	EA.	\$15,000	\$15,000		\$15,385				
New 6,000 Gallon Emergency Storage Tank	1	EA.	\$15,000	\$15,000	\$15,077	\$15,385				
New Electrical Service	1	L.S.	\$15,000	\$15,000	\$15,077	\$15,385	\$15,692			
8" PVC Sewerline	40	L.F.	\$80	\$3,200	\$3,216	\$3,282	\$3,348			
Topsoil	30	C.Y.	\$25	\$750	\$754	\$769	\$785			
Seeding, Fertilizer and Liming	0.5	Acre	\$1,000	\$500	\$503	\$513	\$523			
Mulching	0.5	Acre	\$1,000	\$500	\$503	\$513	\$523			
Silt Fence	100	L.F.	\$4	\$350	\$352	\$359	\$366			
Site Prep and Miscellaneous (8%)	1	L.S.	\$4,344	\$4,344	\$4,366	\$4,455	\$4,544			
Contingency (10%)	1	L.S.	\$5,864	\$5,864	\$5,894	\$6,015	\$6,135			
Contractor's Bonds (2%)	1	L.S.	\$1,290	\$1,290	\$1,297	\$1,323	\$1,350			
		S	ubtotal	\$65,799	\$66,136	\$67,486	\$68,835			
			USE	\$66,000	\$67,000	\$68,000	\$69,000			

TABLE C-11										
OPINION OF PROBABLE										
CONSTRUCTION COST										
Town of Richmond West Main Street- Access Road with Water & Sewer Extensions										
Scoping Study										
Upgrade	d Middle Schoo			ell and Emerg	ency Storage)					
			ENR 9,750	ENR 9,750	ENR 9,800	ENR 10,000	ENR 10,200			
			2014	2014	2015	2016	2017			
Description	Quantity	Units	Unit Price	Total Cost	Total Cost	Total Cost	Total Cost			
EMERGENCY ACCESS ROAD										
Demo Existing 4' Diameter Wetwell	1	LS	\$4,000	\$4,000	\$4,021	\$4,103	\$4,185			
New 8' Diameter Wet Well	1	EA.	\$15,000	\$15,000	\$15,077	\$15,385	\$15,692			
New Emergency Generator	1	EA.	\$30,000	\$30,000	\$30,154	\$30,769	\$31,385			
New Electrical Service	1	L.S.	\$12,000	\$12,000	\$12,062	\$12,308	\$12,554			
8" PVC Sewerline	30	L.F.	\$80	\$2,400	\$2,412	\$2,462	\$2,511			
Topsoil	20	C.Y.	\$25	\$500	\$503	\$513	\$523			
Seeding, Fertilizer and Liming	0.5	Acre	\$1,000	\$500	\$503	\$513	\$523			
Mulching	0.5	Acre	\$1,000	\$500	\$503	\$513	\$523			
Silt Fence	100	L.F.	\$4	\$350	\$352	\$359	\$366			
Site Prep and Miscellaneous (8%)	1	L.S.	\$5,220	\$5,220		\$5,354	\$5,461			
Contingency (10%)	1	L.S.	\$7,047	\$7,047		\$7,228				
Contractor's Bonds (2%)	1	L.S.	\$1,550			\$1,590				
		S	ubtotal	\$79,067	\$79,473	\$81,095	\$82,717			
	USE				\$80,000	\$82,000	\$83,000			

TABLE C-12									
<b>OPINION OF PROBABLE</b>									
CONSTRUCTION COST									
Town of Richmond									
West Main Street- Access Road with Water & Sewer Extensions Scoping Study									
3" Low	Pressure Grind		Forcemain Pa	rallel School's	Forcemain				
			ENR 9,750	ENR 9,750	ENR 9,800	ENR 10,000	ENR 10,200		
			2014	2014	2015	2016	2017		
Description	Quantity	Units	Unit Price	Total Cost	Total Cost	Total Cost	Total Cost		
WASTEWATER SYSTEM									
3" HDPE LPS	1,200	L.F.	\$42	\$50,400	\$50,658	\$51,692.31	\$52,726.15		
Core Manhole	1	L.S.	\$1,500	\$1,500	\$1,508	\$1,538.46	\$1,569.23		
Permanent Trench Pavement Repair	6	S.Y.	\$60	\$360	\$362	\$369.23	\$376.62		
Traffic Control	1	L.S.	\$1,500	\$1,500	\$1,508	\$1,538.46	\$1,569.23		
Site Prep and Miscellaneous (8%)	1	L.S.	\$4,301	\$4,301	\$4,323	\$4,411.08	\$4,499.30		
Contingency (10%)	1	L.S.	\$5,806		\$5,836				
Contractor's Bonds (2%)	1	L.S.	\$1,277	\$1,277	\$1,284	\$1,310.09	\$1,336.29		
		S	ubtotal	\$65,144	\$65,478	\$66,815	\$68,151		
			USE	\$66,000	\$66,000	\$67,000	\$69,000		

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